

13.10 ORDINARY METHOD OF SLICES

AC is an arc of a circle

The soil above the trial failure surface is divided into several vertical slices. The width of each slice need not be the same. Considering a unit length perpendicular to the cross section shown, the forces that act on a typical slice (nth slice) are shown in Figure 13.20b.

W_n - weight of the slice.

N_r and T_r - normal and tangential components of the reaction, R , respectively.

P_n and P_{n+1} - normal forces that act on the sides of the slice

T_n and T_{n+1} - shearing forces that act on the sides of the slice

For equilibrium consideration, $N_r = W_n \cos \alpha_n$

The resisting shear force can be expressed as

$$T_r = \tau_r(\Delta L_n) = \frac{\tau_r(\Delta L_n)}{F_s} = \frac{1}{F_s} [c + \sigma \tan \phi] \Delta L_n \quad (13.55)$$

The normal stress, σ , in Eq. (13.55) is equal to $\frac{N_r}{\Delta L_n} = \frac{W_n \cos \alpha_n}{\Delta L_n}$

For equilibrium of the trial wedge ABC, the moment of the driving force about O equals the moment of the resisting force about O,

$$\text{or } \sum_{n=1}^{n=p} W_n r \sin \alpha_n = \sum_{n=1}^{n=p} \frac{1}{F_s} \left(c + \frac{W_n \cos \alpha_n}{\Delta L_n} \tan \phi \right) (\Delta L_n) (r)$$

$$\text{or } F_s = \frac{\sum_{n=1}^{n=p} (c \Delta L_n + W_n \cos \alpha_n \tan \phi)}{\sum_{n=1}^{n=p} W_n \sin \alpha_n} \quad (13.56)$$

assumption

pore water pressure is zero.

the resultants of P_n and T_n are equal in magnitude to the resultants of P_{n+1} and T_{n+1} , and that their lines of action coincide.

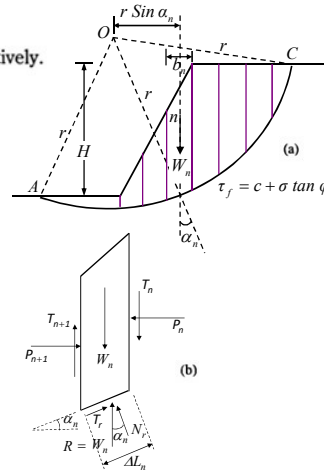
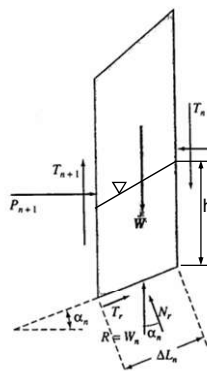


FIGURE 13.20 Stability analysis by ordinary method of slices: (a) trial failure surface; (b) forces acting on nth slice

The ordinary method of slices can also be used for drained conditions/ effective stress analysis



$$N_r = W_n \cos \alpha_n - u_n \Delta L_n$$

$$\sigma' = \frac{W_n \cos \alpha_n - u_n \Delta L_n}{\Delta L_n}$$

$$T_r = \frac{1}{F_s} [c' + \sigma' \tan \phi'] \Delta L_n = \frac{1}{F_s} \left[c' + \frac{W_n \cos \alpha_n - u_n \Delta L_n}{\Delta L_n} \tan \phi' \right] \Delta L_n$$

the moment of the driving force about O $M_d = \sum_{n=1}^{n=p} W_n r \sin \alpha_n$

the moment of the resisting force about O,

$$M_r = \frac{1}{F_s} \left[c' + \frac{W_n \cos \alpha_n - u_n \Delta L_n}{\Delta L_n} \tan \phi' \right] \Delta L_n r$$

For equilibrium of the trial wedge ABC, the moment of the driving force about O equals the moment of the resisting force about O, or

$$\sum_{n=1}^{n=p} W_n r \sin \alpha_n = \sum_{n=1}^{n=p} \frac{1}{F_s} \left[c' + \frac{W_n \cos \alpha_n - u_n \Delta L_n}{\Delta L_n} \tan \phi' \right] \Delta L_n r \Rightarrow F_s = \frac{c' \Delta L_n + (W_n \cos \alpha_n - u_n \Delta L_n) \tan \phi'}{\sum_{n=1}^{n=p} W_n \sin \alpha_n}$$

[Note: ΔL_n in Eq. (13.56) is approximately equal to $(b_n)/(\cos \alpha_n)$, where b_n = the width of the nth slice.]

α_n may be either positive or negative. The value of α_n is positive when the slope of the arc is in the same quadrant as the ground slope.

To find the minimum factor of safety — that is, the factor of safety for the critical circle — one must make several trials by changing the center of the trial circle.

Compare with no seepage case

For equilibrium consideration,

$$N_r = W_n \cos \alpha_n$$

The resisting shear force can be expressed as

$$T_r = \tau_r(\Delta L_n) = \frac{\tau_r(\Delta L_n)}{F_s} = \frac{1}{F_s} [c + \sigma \tan \phi] \Delta L_n \quad (13.55)$$

Compare with the expression for no seepage/water pressure

$$F_s = \frac{\sum_{n=1}^{n=p} (c \Delta L_n + W_n \cos \alpha_n \tan \phi)}{\sum_{n=1}^{n=p} W_n \sin \alpha_n} \quad (13.56)$$

▼ EXAMPLE 13.9

For the slope shown in Figure 13.22, find the factor of safety against sliding for the trial slip surface AC. Use the ordinary method of slices.

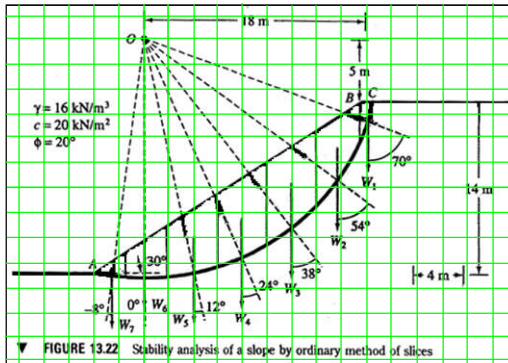
Solution The sliding wedge is divided into seven slices.

$$F_s = \frac{\sum_{n=1}^{n=7} (cb_n + W_n \tan \phi) \frac{1}{m_{\alpha(n)}}}{\sum_{n=1}^{n=7} W_n \sin \alpha_n} \quad (13.63)$$

where $m_{\alpha(n)} = \cos \alpha_n + \frac{\tan \phi \sin \alpha_n}{F_s}$ (13.62)

$\gamma, \text{ kN/m}^3 =$	16
$\phi, \text{ deg} =$	20
$c, \text{ kN/m}^2 =$	20

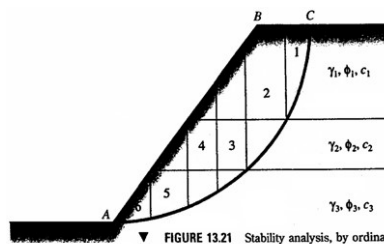
$F_s = 1.58$



▼ FIGURE 13.22 Stability analysis of a slope by ordinary method of slices

Slice No. (1)	Width of slice $b_n, \text{ m}$ (2)	Avg. ht. of slice, $h_n, \text{ m}$, (3)	$W_n, \text{ kN/m}$ (4)	$\alpha_n \text{ (deg)}$, (5)	$m_{\alpha(n)}$, (6)	$W_n/m_{\alpha(n)}$, (7)	$W_n \sin \alpha_n$ (8)
1	1	1.4	22.4	70	0.56	40.11	21.0
2	4	4.6	294.4	54	0.77	380.29	238.2
3	4	6.8	435.2	38	0.93	468.04	267.9
4	4	6.8	435.2	24	1.01	432.07	177.0
5	4	6.1	390.4	12	1.03	380.49	81.2
6	4	4.2	268.8	0	1.00	268.80	0.0
7	3.2	1.3	66.6	-8	0.96	69.46	-9.3
Σ	24.2					2039.26	776.1

$F_s = 1.58$



▼ FIGURE 13.21 Stability analysis, by ordinary method of slices, for slope in layered soils

Method of slices can applied to layered soil
 c, ϕ, γ will not be same for all slices; γ may vary within a slice