

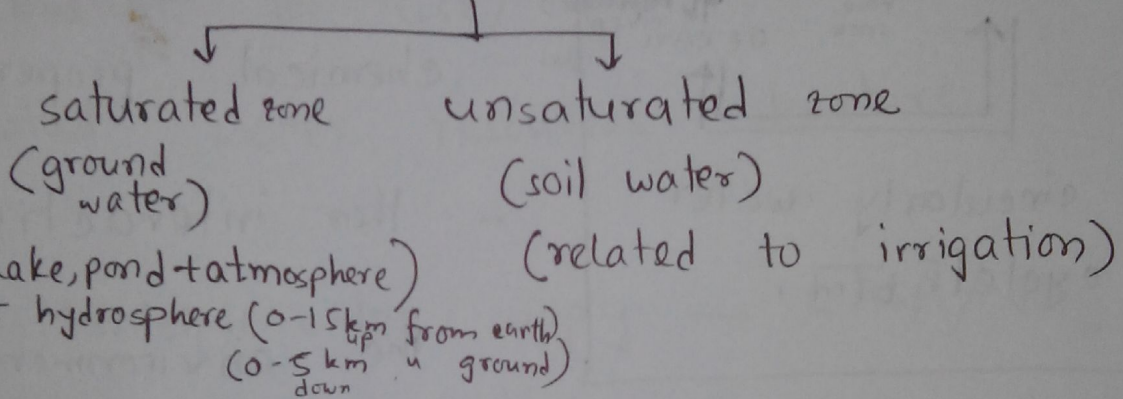
Hydrology

Minjahan sir

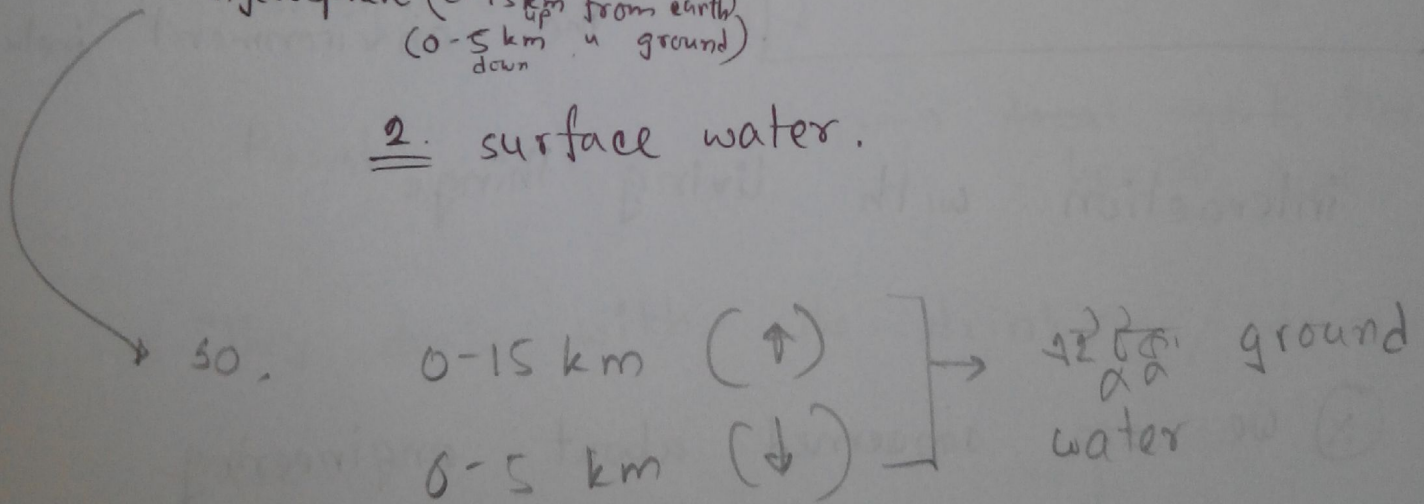
Sat + Sun
1st half

* Hydrology is science of water.

* 1. sub surface water



2. surface water.

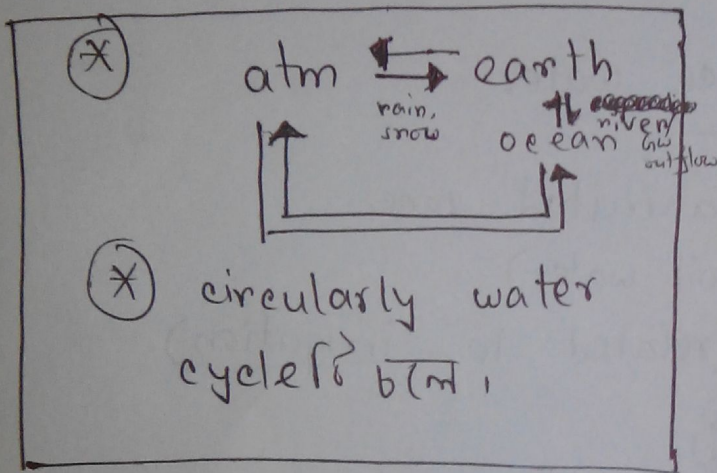


It mainly deals with hydrology.

* climatology / meteorology → atmosphere related

(*) Hydrology :

⇒ Its a branch of earth science which deals with waters of the earth; their - their occurrence, circulation and distribution.



- their physical and chemical properties.

- their interaction with the environment including

interaction with living things.

(*) we are concerned about engineering hydrology.

(*) Hydrology is a multi-disciplinary subject.

(*) Engineering hydrology :

⇒ deals with planning, design and operation of engineering project.

(*) structure for use/control the water is called water resources structure.

(*) Municipal water supply → WASA / city corp.

Rural " " → ~~local~~ ^{govt.} Public health dept

→ They deals with the drinking / domestic use.

→ Hydrologic design is the first step, for this hydrologic analysis is needed.

→ BRWTN controls the navigation.

→ Hydropower generation.

→ Recreation/ Esthetic value.

→ Water control : ⇒ Dam construction
→ (dry season व गति बन्ने)
→ (Flood control)

⇒ Embankment construct

⇒ coastal embankment

regulator : 2 future flow

sluice gate : 1 " " (country side
↓
river side)

→ sedimentation control

Purpose:

⊗ hydrologic process

⊗ hydrologic measurement

→ GW flow
(dead body, river etc)

→ evaporation/Transpiration (living body (एरो)
water → vapour

→ precipitation

→ infiltration

→ River water flow

⊗ sun एरो energy

⊗ → latent heat (body temp)

→ sensible heat (air एरो)

⊗ Data analysis

⊗ Estimation of runoff rainfall.

⊗ Design.

Content :

- Hydrologic cycle.

- Weather and hydrology. (V.T. Chow)
↓
climatology

- Rainfall - runoff relations.

- Hydrograph and unit hydrograph.

- statistical methods in hydrology.

Ref. book : 1. Engineering hydrology

- K.S. Subramanya.

2. Hydrology for engineers - R.K. Linsley, M.A. Kohler
and J.H.L. Paulhus.

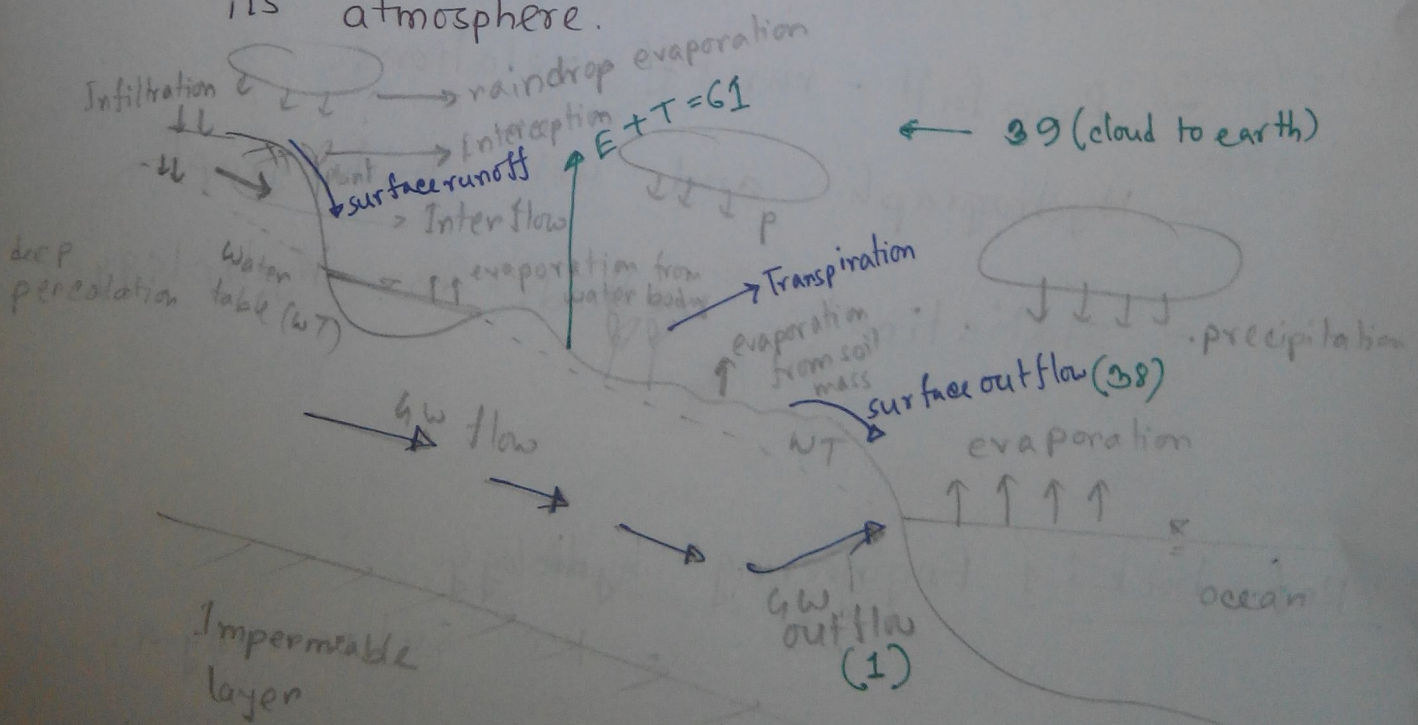
3. Applied Hydrology - V.T. Chow, D.R. Maidment
& L.W. Mays.

Chapter 1: Introduction

Hydrologic cycle

- continuous cycle
- circulation of water

It's a cycle representing endless circulation of water betⁿ the earth and its atmosphere.



starts from ocean (by evaporation)

↓
condensation → cloud form

↓
precipitation

↓
raindrop evaporation

↓
Interception

↓
Infiltration

↓
Interflow (→ →)

and deep percolation (↓↓)

and GW flow (→ →)

and surface runoff

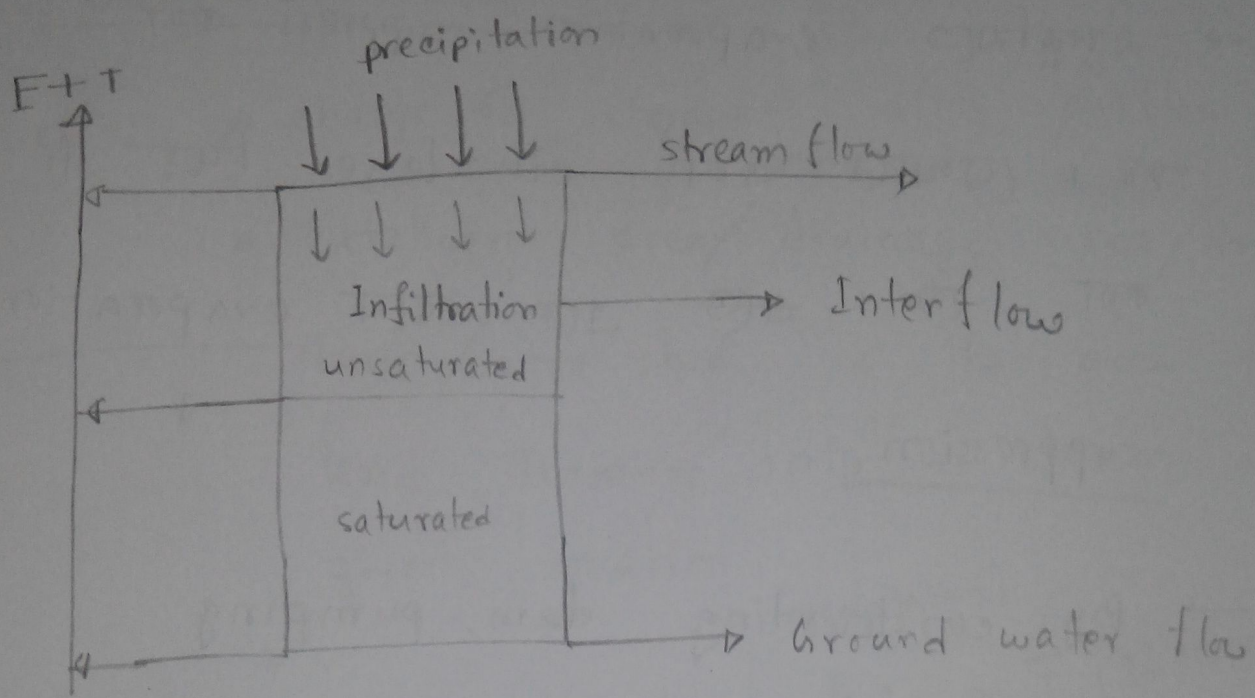
↓
~~evaporation~~ evaporation from water body

u

u

↓
Transpiration.

✗ Transportation component এর inter dependence:



✗ Human interference:

→ স্থানি স্ববায়ু ক্ষয় condensation + dust particle এর দরকার। জনৈক ক্ষয় স্থানি না হলে chemical / salt দিতে দিতে পারি না। dust provide করে। এতে স্থানি হয়।

It's called artificial rain.

→ ଅନୁକୂଳିତ evaporation suppress କର

→ ଏ. । ସେଧାନ ଏକାଏ oil layer ଦିଅଁ ଏକ

କର ଏ. । ଏକ ଏକ evaporation

suppression.

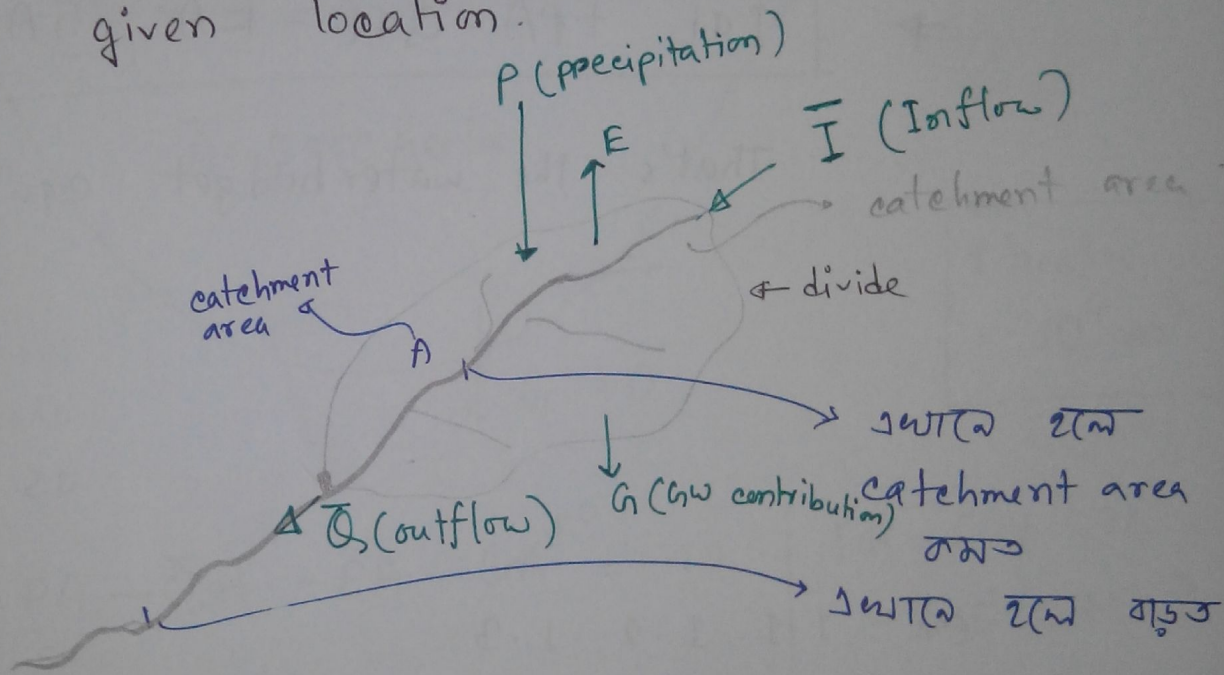
→ By constructing dam, pumping
(surface water କୁ ଅନ୍ୟ divert କର)

→ waste water / solid waste dump କର,

→ clearing forest for agriculture.
(Infiltration + run off diff amount)

X] Water budget equation:

- change of storage = inflow - outflow.
- catchment area / drainage area / drainage basin / water shed is the area of land draining into a stream at a given location.



- ridge line କୁହାଯାଏ କ୍ଷେତ୍ର catchment area କୁହାଯାଏ । from the elevation map we can do that.

• In an interval of time, Δt

$$\text{mass inflow} - \text{mass outflow} = \text{change in mass storage}$$

$\left(\frac{\text{vol}^m}{\text{density}}\right)$

If density is const., then mass = vol^m.

$$\therefore V_i - V_o = \Delta V$$

$$\Rightarrow \boxed{\bar{I} \Delta t + PA - \bar{Q} \Delta t - EA - GA = \Delta S}$$

$\begin{matrix} -GA \\ (+/-) \end{matrix}$

That's the water budget eqnⁿ.

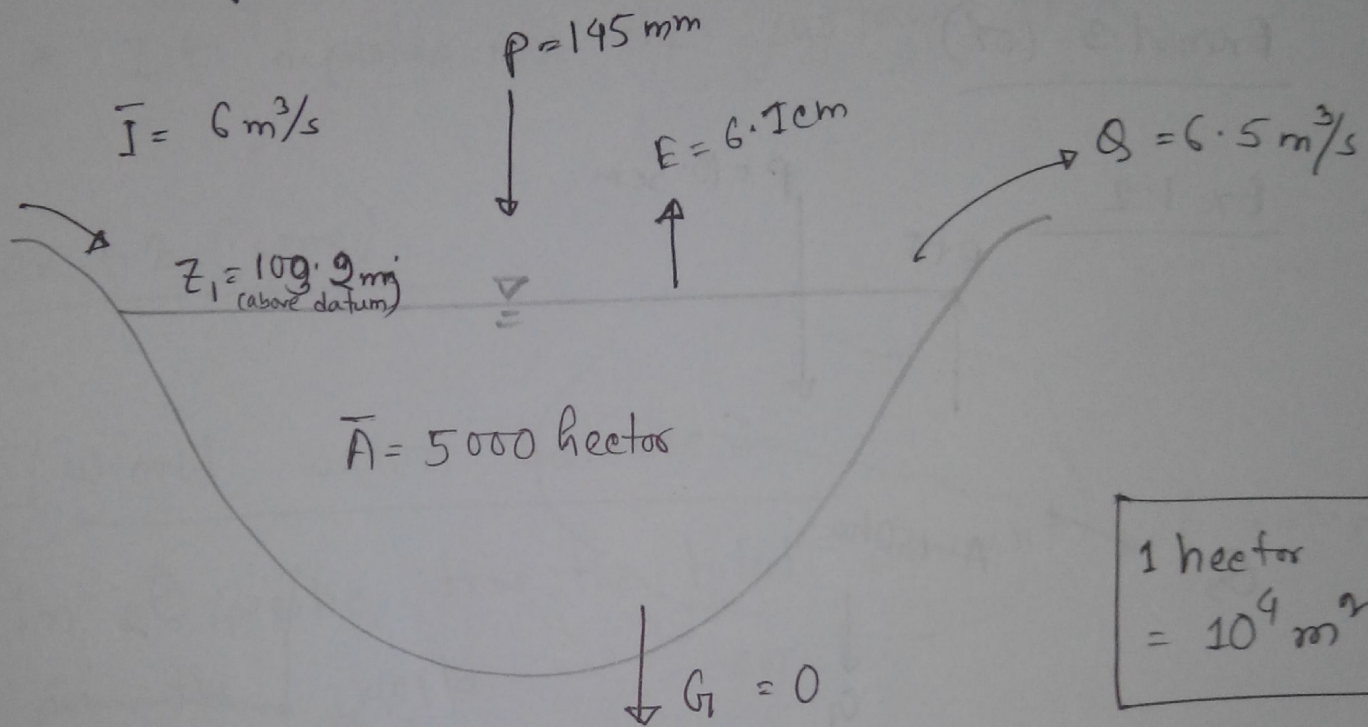
surface area \times
change in depth
 $\Delta S = \Delta z A$

ex: 1.1, 1.2, 1.3

$$I \Delta t - Q \Delta t + PA - EA - GA = \Delta S$$

Ex 1.1 Problem Type 1: Water budget eqn

$$\Delta t = 1 \text{ month (30 days)} = \frac{30 \times 24 \times 60 \times 60}{10^6} = 2.592 \text{ Ms.}$$



1 hectare
= 10^4 m^2

$$\bar{I}\Delta t + PA - \bar{Q}\Delta t - EA - GA = \Delta S$$

$$\Rightarrow \Delta S = 15.552 + 7.25 - 16.848 - 3.05$$

$$= 2.904 \text{ Mm}^3$$

(WL ↑)

$$\Delta S = \Delta z A$$

$$\Rightarrow \Delta z = \frac{\Delta S}{A} = \frac{2.904 \times 10^6}{5000 \times 10^4} = 0.05808 \text{ m rise}$$

$$\begin{aligned} \bar{I}\Delta t &= 6 \times 2.592 \\ &= 15.552 \text{ Mm}^3 \end{aligned}$$

$$\begin{aligned} PA &= \frac{145 \times 10^{-3} \times 5000 \times 10^4}{10^6} \\ &= 7.25 \text{ Mm}^3 \end{aligned}$$

$$\begin{aligned} \bar{Q}\Delta t &= 6.5 \times 2.592 \\ &= 16.848 \text{ Mm}^3 \end{aligned}$$

$$\begin{aligned} EA &= \frac{6.1 \times 10^{-2} \times 5000 \times 10^4}{10^6} \\ &= 3.05 \text{ Mm}^3 \end{aligned}$$

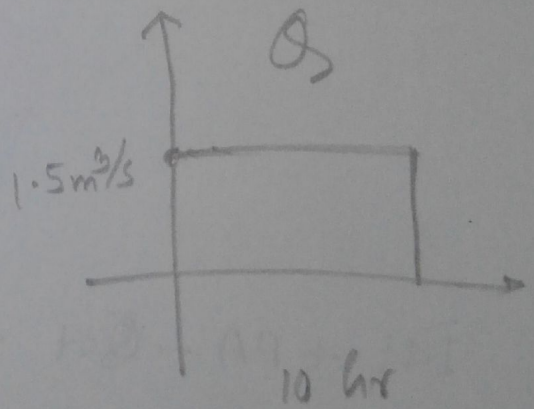
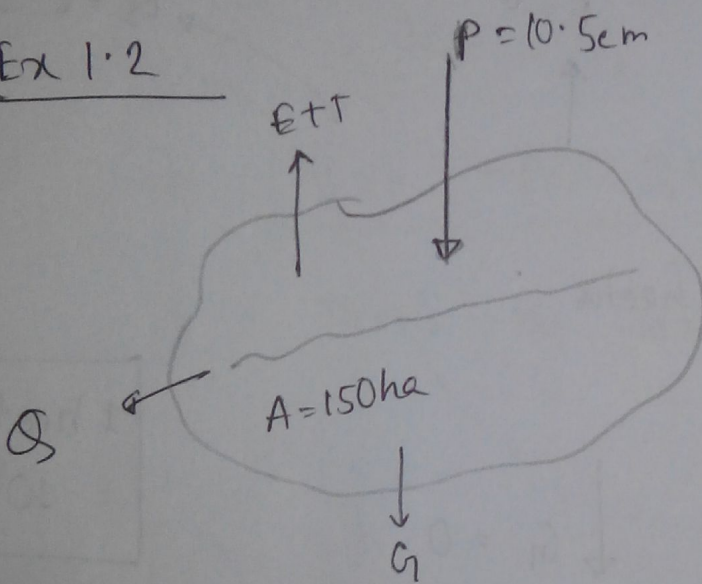
$$z_2 = 103.2 + 0.058$$

= 103.258 m above datum.

(Am)

Exercise 1.3 (02)

Ex 1.2

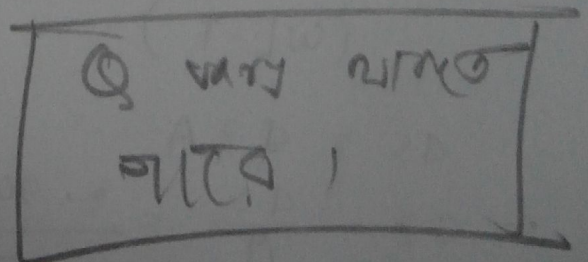


dry ($\Delta S = 0$)

inflow = P

outflow = 10 hr \times Q
+ (E+T) (h)

Q emit



* run-off co-efficient = $\frac{\text{run-off vol}^m}{\text{rain vol}^m (P)}$

* only loss is infiltration.

* It depends on the surface area.

ex 1.3 (same)

World water balance :

→ In a specific time, the total amount of water in the world is const.

→ Total water = 1386 M km^3 .

→ maximum water is saline.

→ fresh water = 2.57% (2.57% Polar ice = 1.77%)

∴ rest (usable fresh water = 0.8%)

Ground water = 0.76% [total usable freshwater → 95%]

Lakes and streams = 0.008%

others = 0.032%

[with 57% surface water]

* ~~Residence time~~ ~~of~~ ~~the~~ ~~system~~ ~~indicates~~ ~~the~~ ~~dynamic~~ ~~of~~ ~~the~~ ~~system~~

* Atmospheric water is the most dynamic
Surface & Ground water medium (laminar)

So, & Ground water model possible AT,

* Residence time, T_p = the average duration of a particle of water to pass through a phase of hydrologic cycle.

$$T_p = \frac{V_0 \text{ m of water in the phase (Table 1.1)}}{\text{Average flow rate (outflow rate) in the phase}}$$

(Table 1.2)

* Global river water, $T_r = \frac{0.00212 \text{ Mkm}^3}{44700 \text{ km}^3/\text{yr}}$

$$= \frac{0.00212 \text{ Mkm}^3 \times 365}{44700 \text{ km}^3/\text{days}}$$

$$= 17.8 \text{ days.}$$

Type 2: Residence time

* Problem 1.6 (a), (b)

~~precipitation~~ ^{outflow} precipitation → Both ~~precipitation~~ ^{+ ocean} add ~~precipitation~~

Ans = 8.2 days.

GW → 28,500 ~~precipitation~~

05/03/17

lec. 4

Weather and hydrology (Sheet 2/10)

only related to hydrology part

* [

climate → long term atmospheric condition

weather → short u " "

* weather parameters :

→ rainfall / precipitation

→ temperature

→ Humidity / vapour pressure

→ Wind speed

↓ application part
as river bank erosion or
storm water etc. stream flow,
erosion etc.

* Wind speed measure using
process part.

Hydrologic cycle energy source :

- ① Sun/solar radiation
- ② Wind/Atmosphere

ATMOSPHERIC
WATER

Of the many meteorological processes occurring continuously within the atmosphere, the processes of precipitation and evaporation, in which the atmosphere interacts with surface water, are the most important for hydrology. Much of the water precipitated on the land surface is derived from moisture evaporated from the oceans and transported long distances by atmospheric circulation. The two basic driving forces of atmospheric circulation result from the rotation of the earth and the transfer of heat energy between the equator and the poles.

3.1 ATMOSPHERIC CIRCULATION

The earth constantly receives heat from the sun through solar radiation and emits heat through re-radiation, or *back radiation* into space. These processes are in balance at an average rate of approximately 210 W/m^2 . The heating of the earth is uneven; near the equator, the incoming radiation is almost perpendicular to the land surface and averages about 270 W/m^2 , while near the poles, it strikes the earth at a more oblique angle at a rate of about 90 W/m^2 . Because the rate of radiation is proportional to the absolute temperature at the earth's surface, which does not vary greatly between the equator and the poles, the earth's emitted radiation is more uniform than the incoming radiation. In response to this imbalance, the atmosphere functions as a vast heat engine, transferring energy from the equator toward the poles at an average rate of about $4 \times 10^9 \text{ MW}$.

If the earth were a nonrotating sphere, atmospheric circulation would appear as in Fig. 3.1.1. Air would rise near the equator and travel in the upper atmosphere toward the poles, then cool, descend into the lower atmosphere, and return toward the equator. This is called Hadley circulation.

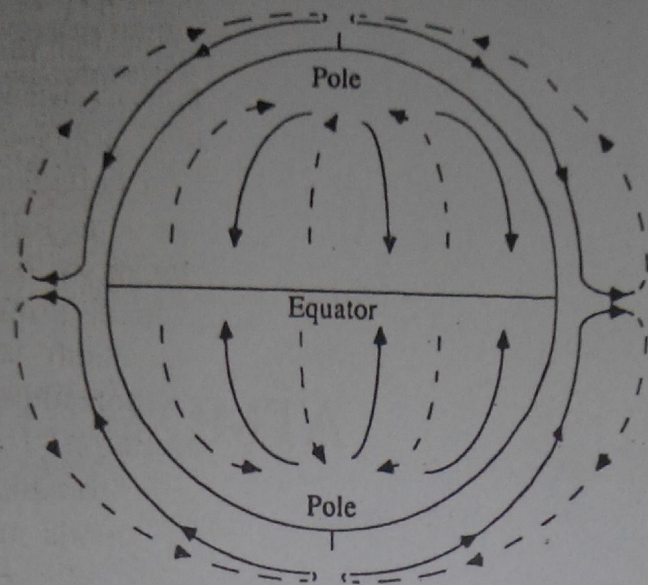


FIGURE 3.1.1

One-cell atmospheric circulation pattern for a nonrotating planet.

The rotation of the earth from west to east changes the circulation pattern. As a ring of air about the earth's axis moves toward the poles, its radius decreases. In order to maintain angular momentum, the velocity of air increases with respect to the land surface, thus producing a westerly air flow. The converse is true for a ring of air moving toward the equator—it forms an easterly air flow. The effect producing these changes in wind direction and velocity is known as the *Coriolis force*.

The actual pattern of atmospheric circulation has three cells in each hemisphere, as shown in Fig. 3.1.2. In the *tropical cell*, heated air ascends at the equator, proceeds toward the poles at upper levels, loses heat and descends toward the ground at latitude 30° . Near the ground, it branches, one branch moving toward the equator and the other toward the pole. In the *polar cell*, air rises at 60° and flows toward the poles at upper levels, then cools and flows back to 60° near the earth's surface. The *middle cell* is driven frictionally by the other two; its surface air flows toward the pole, producing prevailing westerly air flow in the mid-latitudes.

The uneven distribution of ocean and land on the earth's surface, coupled with their different thermal properties, creates additional spatial variation in atmospheric circulation. The annual shifting of the thermal equator due to the earth's revolution around the sun causes a corresponding oscillation of the three-cell circulation pattern. With a larger oscillation, exchanges of air between adjacent cells can be more frequent and complete, possibly resulting in many flood years. Also, monsoons may advance deeper into such countries as India and Australia. With a smaller oscillation, intense high pressure may build up around 30° latitude, thus creating extended dry periods. Since the atmospheric circulation is very complicated, only the general pattern can be identified.

The atmosphere is divided vertically into various zones. The atmospheric circulation described above occurs in the *troposphere*, which ranges in height from about 8 km at the poles to 16 km at the equator. The temperature in the troposphere decreases with altitude at a rate varying with the moisture content of

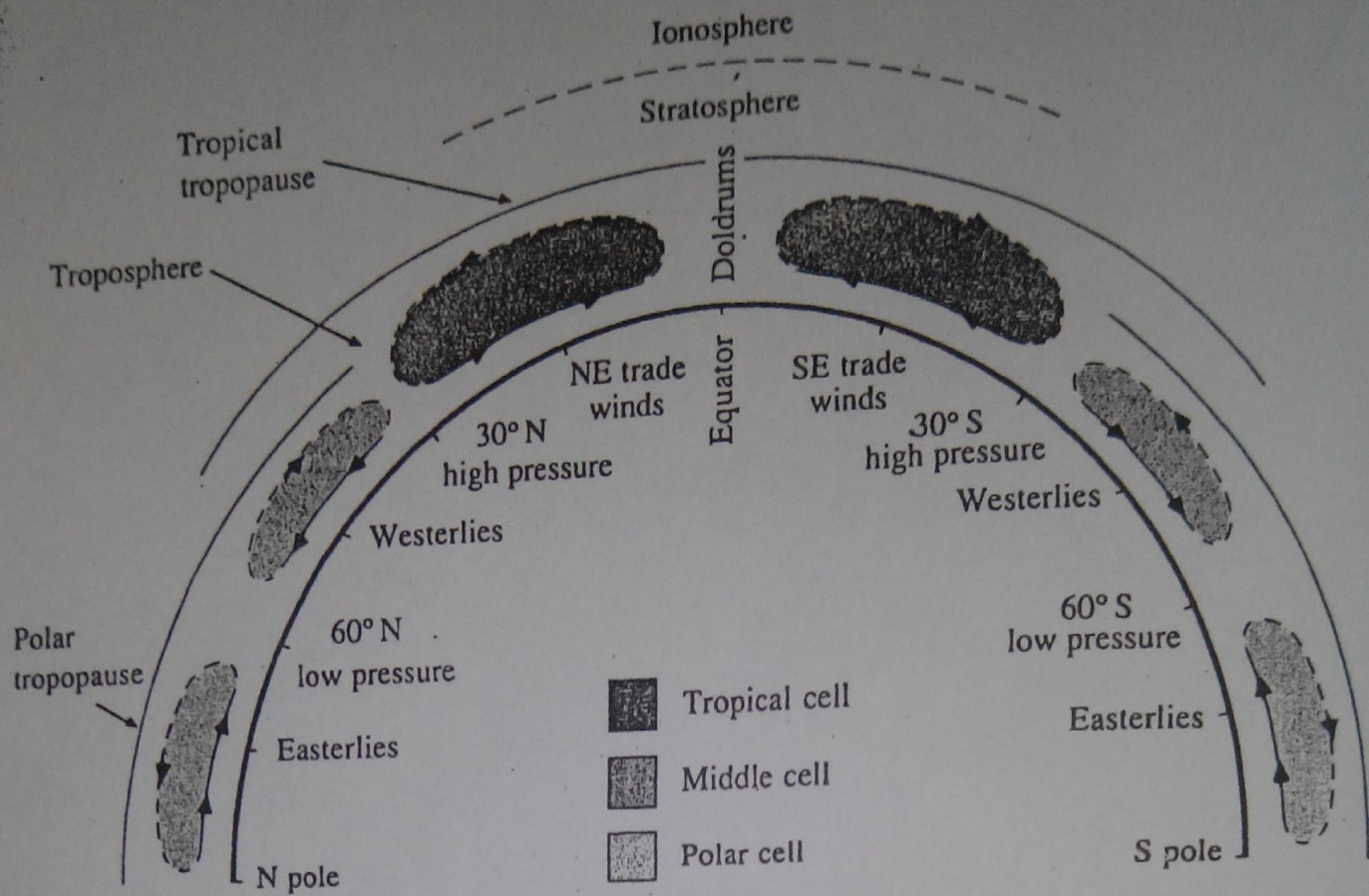


FIGURE 3.1.2
 Latitudinal cross section of the general atmospheric circulation.

the atmosphere. For dry air the rate of decrease is called the dry adiabatic lapse rate and is approximately $9.8^{\circ}\text{C}/\text{km}$ (Brutsaert, 1982). The saturated adiabatic lapse rate is less, about $6.5^{\circ}\text{C}/\text{km}$, because some of the vapor in the air condenses as it rises and cools, releasing heat into the surrounding air. These are average figures for lapse rates that can vary considerably with altitude. The tropopause separates the troposphere from the stratosphere above. Near the tropopause, sharp changes in temperature and pressure produce strong narrow air currents known as jet streams with speeds ranging from 15 to 50 m/s (30 to 100 mi/h). They flow for thousands of kilometers and have an important influence on air mass movement.

An air mass in the general circulation is a large body of air that is fairly uniform horizontally in properties such as temperature and moisture content. When an air mass moves slowly over land or sea areas, its characteristics reflect those of the underlying surface. The region where an air mass acquires its characteristics is its source region; the tropics and the poles are two source regions. Where a warm air mass meets a cold air mass, instead of their simply mixing, a definite surface of discontinuity appears between them, called a front. Cold air, being heavier, underlies warm air. If the cold air is advancing toward the warm air, the leading edge of the cold air mass is a cold front and is nearly vertical in slope. If the warm air is advancing toward the cold air, the leading edge is a warm front, which has a very flat slope, the warm air flowing up and over the cold air.

✓ A cyclone is a region of low pressure around which air flows in a counterclockwise direction in the north

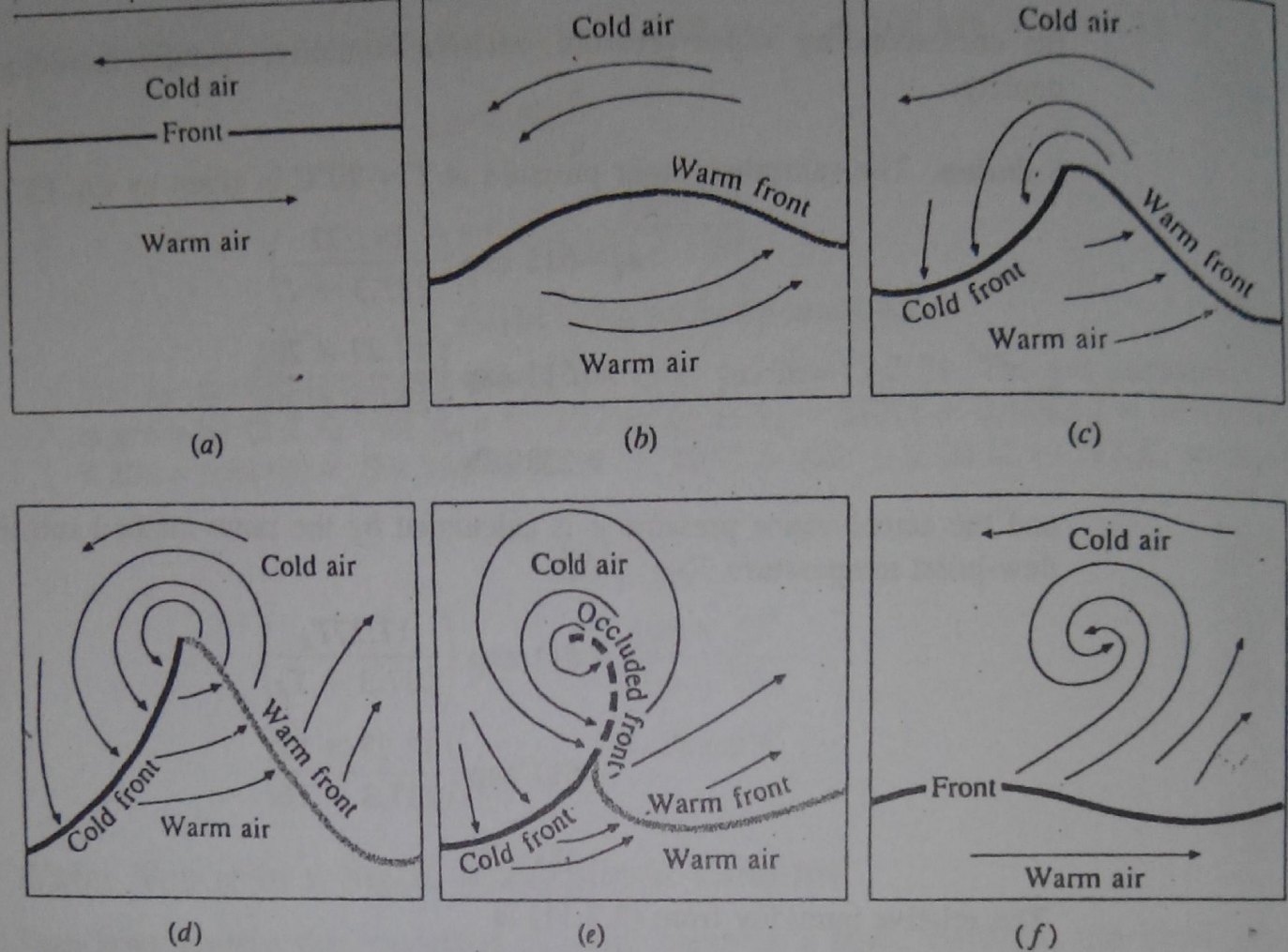


FIGURE 3.1.3

A plan view of the life cycle of a Northern Hemisphere frontal cyclone: (a) surface front between cold and warm air; (b) wave beginning to form; (c) cyclonic circulation and wave have developed; (d) faster-moving cold front is overtaking retreating warm front and reducing warm sector; (e) warm sector has been eliminated and (f) cyclone is dissipating.

hurricanes or typhoons. Extratropical cyclones are formed when warm and cold air masses, initially flowing in opposite directions adjacent to one another, begin to interact and whirl together in a circular motion, creating both a warm front and a cold front centered on a low pressure zone (Fig. 3.1.3). An anticyclone is a region of high pressure around which air flows clockwise in the northern hemisphere, counterclockwise in the Southern hemisphere. When air masses are lifted in atmospheric motion, their water vapor can condense and produce precipitation.

3.2 WATER VAPOR

Atmospheric water mostly exists as a gas, or vapor, but briefly and locally it becomes a liquid in rainfall and in water droplets in clouds, or it becomes a solid in snowfall, in hail, and in ice crystals in clouds. The amount of water vapor in the atmosphere is less than 1 part in 100,000 of all the waters of the earth, but it plays a vital role in the hydrologic cycle.

Vapor transport in air through a hydrologic system can be described by the Reynolds transport theorem [Eq. (2.1.9)] letting the extensive property B be the

mass of water vapor. The intensive property $\beta = dB/dm$ is the mass of water vapor per unit mass of moist air; this is called the specific humidity q_v , and equals the ratio of the densities of water vapor (ρ_v) and moist air (ρ_a):

$$q_v = \frac{\rho_v}{\rho_a} \quad (3.2.1)$$

By the law of conservation of mass, $dB/dt = \dot{m}_v$, the rate at which water vapor is being added to the system. For evaporation from a water surface, \dot{m}_v is positive and represents the mass flow rate of evaporation; conversely, for condensation, \dot{m}_v is negative and represents the rate at which vapor is being removed from the system. The Reynolds transport equation for this system is the continuity equation for water vapor transport:

$$\dot{m}_v = \frac{d}{dt} \iiint_{c.v.} q_v \rho_a dV + \iint_{c.s.} q_v \rho_a \mathbf{V} \cdot d\mathbf{A} \quad (3.2.2)$$

Vapor Pressure

Dalton's law of partial pressures states that the pressure exerted by a gas (its vapor pressure) is independent of the presence of other gases; the vapor pressure e of the water vapor is given by the *ideal gas law* as

$$e = \rho_v R_v T \quad (3.2.3)$$

where T is the absolute temperature in K, and R_v is the gas constant for water vapor. If the total pressure exerted by the moist air is p , then $p - e$ is the partial pressure due to the dry air, and

$$p - e = \rho_d R_d T \quad (3.2.4)$$

where ρ_d is the density of dry air and R_d is the gas constant for dry air (287 J/kg·K). The density of moist air ρ_a is the sum of the densities of dry air and water vapor, that is, $\rho_a = \rho_d + \rho_v$, and the gas constant for water vapor is $R_v = R_d/0.622$, where 0.622 is the ratio of the molecular weight of water vapor to the average molecular weight of dry air. Combining (3.2.3) and (3.2.4) using the above definitions gives

$$p = \left[\rho_d + \left(\frac{\rho_v}{0.622} \right) \right] R_d T \quad (3.2.5)$$

By taking the ratio of Eqs. (3.2.3) and (3.2.5), the specific humidity q_v is approximated by

$$q_v = 0.622 \frac{e}{p} \quad (3.2.6)$$

Also, (3.2.5) can be rewritten in terms of the gas constant for moist air, R_a , as

$$p = \rho_a R_a T \quad (3.2.7)$$

The relationship between the gas constants for moist air and dry air is given by

$$R_a = R_d(1 + 0.608q_v) \\ = 287(1 + 0.608q_v) \text{ J/kg}\cdot\text{K} \quad (3.2.8)$$

The gas constant of moist air increases with specific humidity, but even for a large specific humidity (e.g., $q_v = 0.03 \text{ kg water/kg of moist air}$), the difference between the gas constants for moist and dry air is only about 2 percent.

For a given air temperature, there is a maximum moisture content the air can hold, and the corresponding vapor pressure is called the *saturation vapor pressure* e_s . At this vapor pressure, the rates of evaporation and condensation are equal. Over a water surface the saturation vapor pressure is related to the air temperature as shown in Fig. 3.2.1; an approximate equation is:

$$e_s = 611 \exp\left(\frac{17.27T}{237.3 + T}\right) \quad (3.2.9)$$

where e_s is in pascals ($\text{Pa} = \text{N/m}^2$) and T is in degrees Celsius (Raudkivi, 1979). Some values of the saturation vapor pressure of water are listed in Table 3.2.1.

The gradient $\Delta = de_s/dT$ of the saturated vapor pressure curve is found by differentiating (3.2.9):

$$\Delta = \frac{4098e_s}{(237.3 + T)^2} \quad (3.2.10)$$

where Δ is the gradient in pascals per degree Celsius.

The relative humidity R_h is the ratio of the actual vapor pressure to its saturation value at a given air temperature (see Fig. 3.2.1):

$$R_h = \frac{e}{e_s} \quad (3.2.11)$$

The temperature at which air would just become saturated at a given specific humidity is its dew-point temperature T_d .

Example 3.2.1 At a climate station, air pressure is measured as 100 kPa, air temperature as 20°C , and the wet-bulb, or dew-point, temperature as 16°C . Calculate

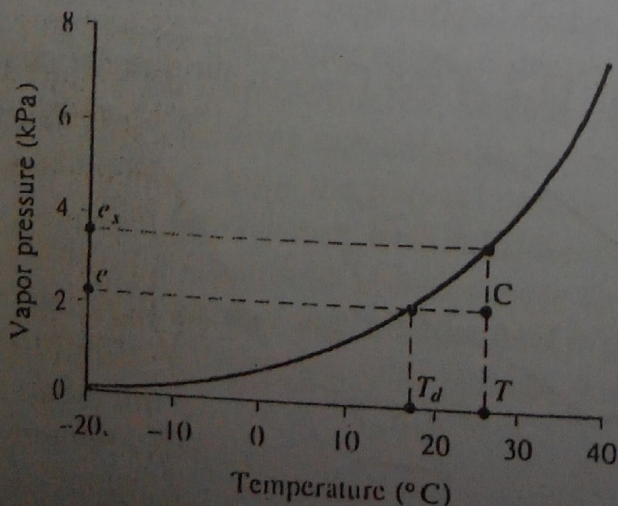


FIGURE 3.2.1 Saturated vapor pressure as a function of temperature over water. Point C has vapor pressure e and temperature T , for which the saturated vapor pressure is e_s . The relative humidity is $R_h = e/e_s$. The temperature at which the air is saturated for vapor pressure e is the dew-point temperature T_d .

the corresponding vapor pressure, relative humidity, specific humidity, and air density.

Solution. The saturated vapor pressure at $T = 20^\circ\text{C}$ is given by Eq. (3.2.9)

$$\begin{aligned} e_s &= 611 \exp\left(\frac{17.27T}{237.3 + T}\right) \\ &= 611 \exp\left(\frac{17.27 \times 20}{237.3 + 20}\right) \\ &= 2339 \text{ Pa} \end{aligned}$$

and the actual vapor pressure e is calculated by the same method substituting the dew-point temperature $T_d = 16^\circ\text{C}$:

$$\begin{aligned} e &= 611 \exp\left(\frac{17.27T_d}{237.3 + T_d}\right) \\ &= 611 \exp\left(\frac{17.27 \times 16}{237.3 + 16}\right) \\ &= 1819 \text{ Pa} \end{aligned}$$

The relative humidity from (3.2.11) is

$$\begin{aligned} R_h &= \frac{e}{e_s} \\ &= \frac{1819}{2339} \\ &= 0.78 \\ &= 78\% \end{aligned}$$

TABLE 3.2.1
Saturated vapor pressure of water
vapor over liquid water

Temperature $^\circ\text{C}$	Saturated Vapor Pressure Pa
-20	125
-10	286
0	611
5	872
10	1227
15	1704
20	2337
25	3167
30	4243
35	5624
40	7378

Source: Brutsaert, 1982, Table 3.4, p. 41. Used with permission.

and the specific humidity is given by (3.2.6) with $p = 100 \text{ kPa} = 100 \times 10^3 \text{ Pa}$:

$$\begin{aligned}q_v &= 0.622 \frac{e}{p} \\ &= 0.622 \left(\frac{1819}{100 \times 10^3} \right) \\ &= 0.0113 \text{ kg water/kg moist air}\end{aligned}$$

The air density is calculated from the ideal gas law (3.2.7). The gas constant R_a is given by (3.2.8) with $q_v = 0.0113 \text{ kg/kg}$ as $R_a = 287(1 + 0.608q_v) = 287(1 + 0.608 \times 0.0113) = 289 \text{ J/kg}\cdot\text{K}$, and $T = 20^\circ\text{C} = (20 + 273) \text{ K} = 293 \text{ K}$, so that

$$\begin{aligned}\rho_a &= \frac{p}{R_a T} \\ &= \frac{100 \times 10^3}{289 \times 293} \\ &= 1.18 \text{ kg/m}^3\end{aligned}$$

Water Vapor in a Static Atmospheric Column

Two laws govern the properties of water vapor in a static column, the ideal gas law

$$p = \rho_a R_a T \quad (3.2.12)$$

and the *hydrostatic pressure law*

$$\frac{dp}{dz} = -\rho_a g \quad (3.2.13)$$

The variation of air temperature with altitude is described by

$$\frac{dT}{dz} = -\alpha \quad (3.2.14)$$

where α is the lapse rate. As shown in Fig. 3.2.2, a linear temperature variation combined with the two physical laws yields a nonlinear variation of pressure with altitude. Density and specific humidity also vary nonlinearly with altitude. From (3.2.12), $\rho_a = p/R_a T$, and substituting this into (3.2.13) yields

$$\frac{dp}{dz} = \frac{-pg}{R_a T}$$

or

$$\frac{dp}{p} = \left(\frac{-g}{R_a T} \right) dz$$

Substituting $dz = -dT/\alpha$ from (3.2.14):

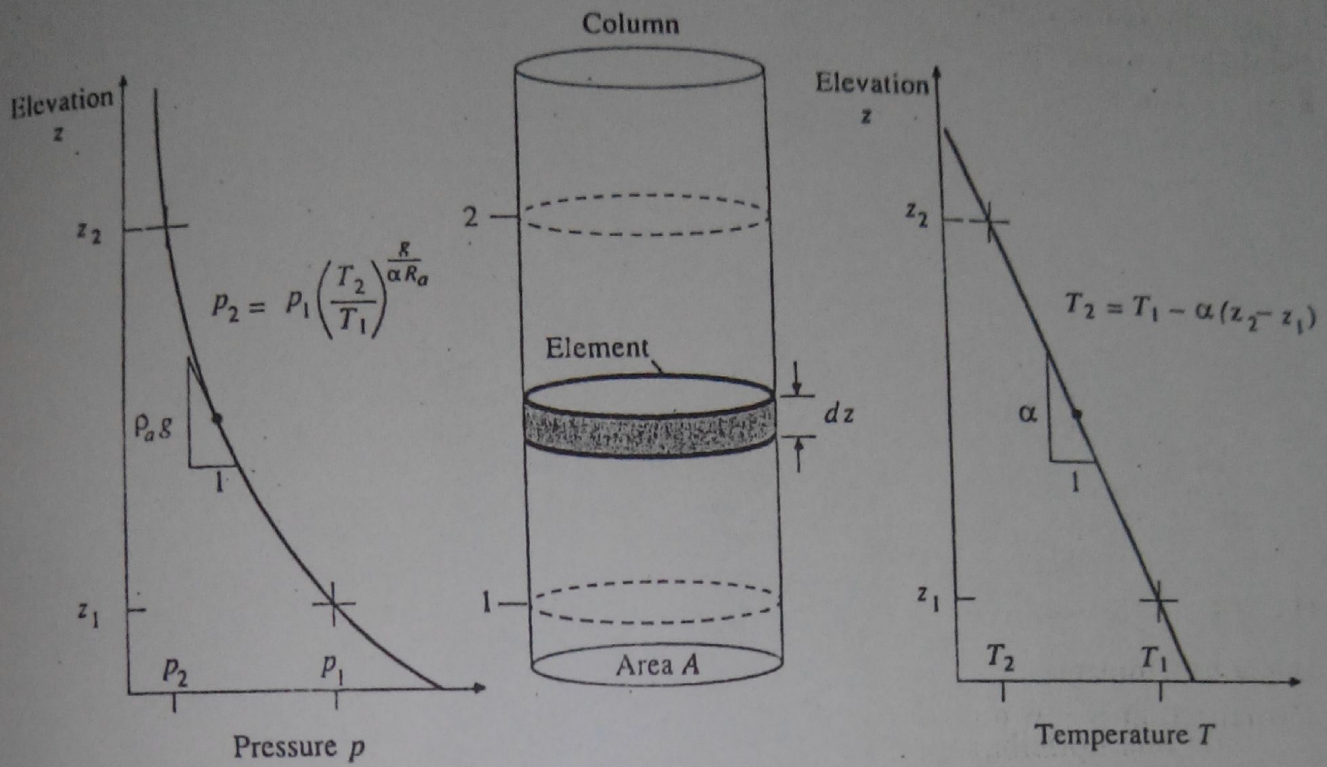


FIGURE 3.2.2
Pressure and temperature variation in an atmospheric column.

$$\frac{dp}{p} = \left(\frac{g}{\alpha R_a} \right) \frac{dT}{T}$$

and integrating both sides between two levels 1 and 2 in the atmosphere gives

$$\ln \left(\frac{p_2}{p_1} \right) = \left(\frac{g}{\alpha R_a} \right) \ln \left(\frac{T_2}{T_1} \right)$$

or

$$p_2 = p_1 \left(\frac{T_2}{T_1} \right)^{g/\alpha R_a} \quad (3.2.15)$$

From (3.2.14) the temperature variation between altitudes z_1 and z_2 is

$$T_2 = T_1 - \alpha(z_2 - z_1) \quad (3.2.16)$$

Precipitable Water

The amount of moisture in an atmospheric column is called its *precipitable water*. Consider an element of height dz in a column of horizontal cross-sectional area A (Fig. 3.2.2). The mass of air in the element is $\rho_a A dz$ and the mass of water contained in the air is $q_v \rho_a A dz$. The total mass of precipitable water in the column between elevations z_1 and z_2 is

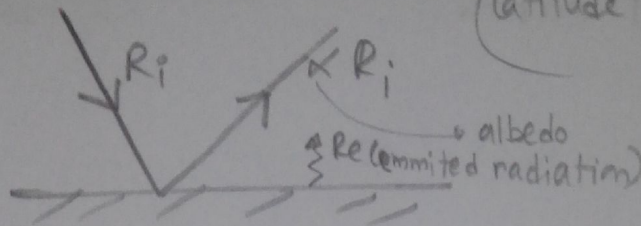
$$m_p = \int_{z_1}^{z_2} q_v \rho_a A dz \quad (3.2.17)$$

① Solar radiation:

R_e ————— Solar constant / Extra-terrestrial radiation

(latitude & time \Rightarrow depend)

$R_i \approx 0.75 R_e$
(clear day \Rightarrow)
cloud \Rightarrow $R_i \downarrow$



net radiation, $R_n = (1 - \alpha) R_i - R_e$

normal earth \Rightarrow
[α] 0.23

α = albedo = ratio of amount of solar radiation reflected by the surface to the amount of incident upon it.

\rightarrow depends on surface type
[white \Rightarrow \uparrow (max)]
[white snow \Rightarrow 95% reflect]

* Latent heat transfer (E+T) ^{for}

* Sensible u u
 ↳ heat (Temp)
 ↳ humidity

* Solar radiation માણાં થયું actinometer or radio meter.

* તાપમાન માણાં થયું daily sunshine hour એ કોઈ કિંચિત્

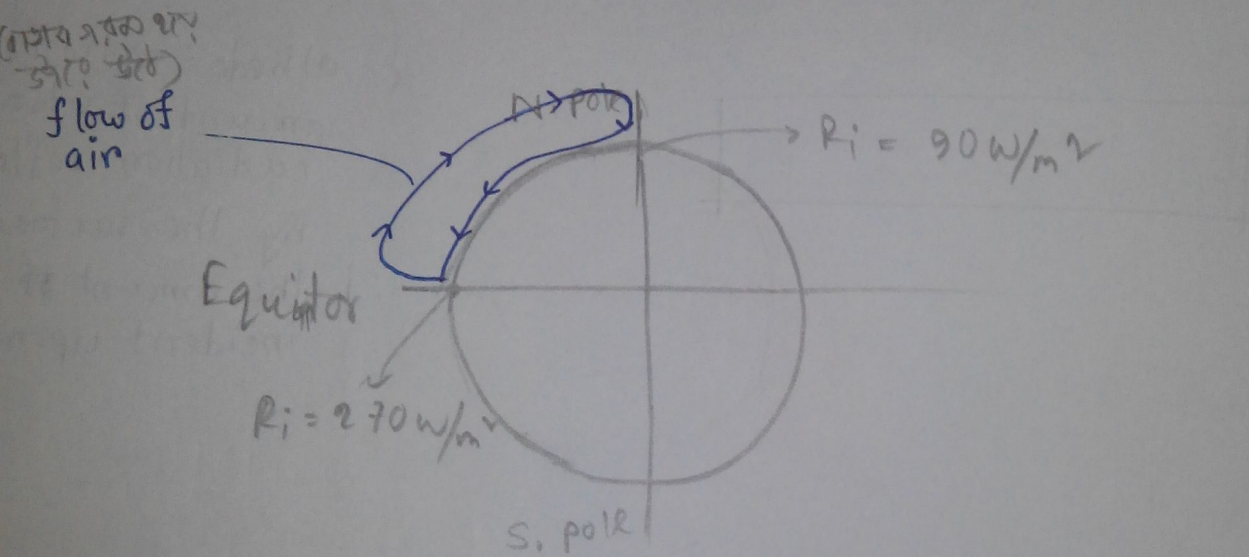
n = sunshine hour

② Atmospheric circulation:

→ (କଣ ହେବ?)

= → uneven heating of the earth.

→ rotation of earth



* Assuming earth to be non-rotating sphere,

ବାୟବ୍ୟ ପ୍ରବାହ ଥାଏ ଓଡ଼ିଆ ଡିଗ୍ରୀ north pole ଠାରୁ

ଫିରକ ଯାଏ cooling down ଓଡ଼ିଆ pressure diff ଥାଏ back

ଓଡ଼ିଆ । It's called Hadley circulation.
It's single cell circulation.

* But actual earth is rotating west → east.

ଏହା ଏକ ଏକ force ଯାହାକୁ 2ω । It's called Coriolis-force. It's the apparent force arising from the earth rotation & causing the changes in wind direction & velocity.

① pole ଠାରେ ଦିଗର ବୃଦ୍ଧି radius ↓.

∴ ↓ velocity ↑

② pole ଠାରୁ equator ଠାରେ ଦିଗର ବୃଦ୍ଧି radius ↑
∴ ↑ vel ↓

③ ଏହା ଏକ westerlized develop କାରଣ ଏହା ମନେ ହେବ ଏକ ବୃଦ୍ଧି ବୃଦ୍ଧି ବୃଦ୍ଧି, vice versa.

ଏହା 3 cell circulation. It's called general atmospheric circulation. ଏହା Troposphere ଠାରେ ଘଟେ ।



* dry air $\text{from (Polar} \rightarrow \text{middle)}$
 wet $\text{from (middle} \rightarrow \text{Polar)}$

* Because of vegetation, undulation of earth surface it will change.

* oscillation occasionally
 oscillation $\text{It's called cyclone.}$

* Cyclone: A region of low pressure around which air flows in a counter clock wise direction in the northern hemisphere.

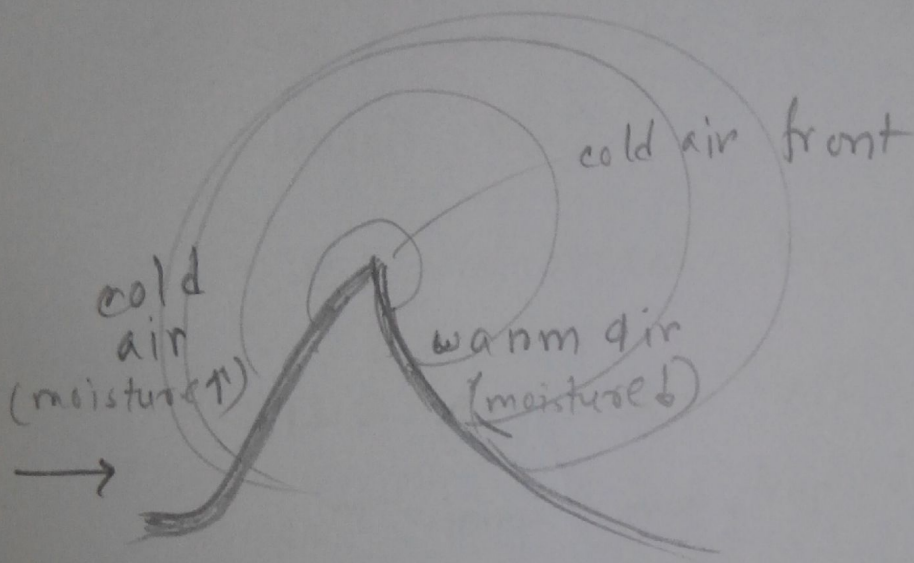
Cyclone is of 2 types:

(1) Tropical cyclone (forms at low latitude $(5^{\circ}-10^{\circ})$)
+ pre monsoon a ocean a depression creat
 in low pressure zone
for condensation it acquires energy. It hits the land

(USA \rightarrow hurricane)
(South \rightarrow typhoon)
ASIA

and depression ଯେ ବାତୀ ସୃଷ୍ଟି
କରେ ସାଥେ)

(2) Extra tropical cyclone : (forms at ~~the~~ when warm air and cold air masses interact and whirl together in a circular motion.



cold air front ଓ warm air overtake

କରି ଦେଇ circular air ସୃଷ୍ଟି କରେ ।

ଫଳରେ cold air front warm air

front ଓ overtake କରେ ସୃଷ୍ଟି କରେ ।

କରେ ।

Temperature

→ E+T depends on temp

→ ঘরে গায়ে গাণি রাখলে E up because of temp in air.

→ day / month এর avg temp কত? ফর্স্টার data collect করে?

= @d metrological Dept (BMD)

কীভাবে avg calc করি?

= max^m temp, min temp diff, বারোটি দিনে note করি

Mean daily temp, $T_{avg} = \frac{T_{max} + T_{min}}{2}$

(x) air দুভাবে গায়ে + sun একেই কততে গায়ে।
এমন অবস্থা সৃষ্টি হবে।

(x) দিনে গায়ে (min), দুভাবে (max) কততে গায়ে।

long term avg (30 yrs) ^{hydrology 5}

* Normal temperature :- avg value for a day, month or even year over a specific 30 yrs period.

11th March 2018 normal temp = last 30 yrs avg

* Lapse rate : Rate of decrease of temperature with altitudes.

It depends on the moisture content of the area.

heat sink (3-1),
VIC 20, latent heat & convert

Dry-adiabatic lapse rate (decrease of temp with altitude of dry adiabatic)

$9.8 \text{ } ^\circ\text{C}/\text{km}$

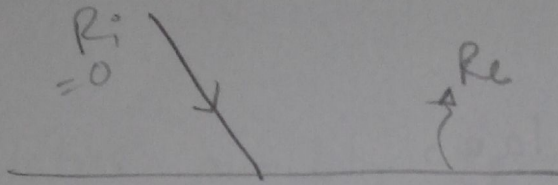
Saturated-adiabatic lapse rate

$6.5 \text{ } ^\circ\text{C}/\text{km}$

temp \downarrow \rightarrow condensation \rightarrow heat generate
 \downarrow
air \uparrow heat up
 \rightarrow rate \downarrow decrease

• Temperature inversion:

* Earth surface is temp \downarrow with altitude. (Earth emits more heat than it receives, so Earth surface cool down)



Increase of temp with altitudes is known as temp inversion.

* Water vapour / humidity: (e) partial pressure which is exerted by the vapour.

→ E + T is very important

→ vapour pressure is diff with altitude. E is 20, then it will be 20, then E will be,

expressed in two ways.

i. specific humidity

ii. Relative "

(air max ^m शतमानि वि०
• १०० वा संमानि वा(२)
(%)

i. specific humidity, q_w : mass of water vapour per unit mass of moist air.

$$q_v = \frac{\text{density of water vapour}}{\text{density of moist air}}$$

$$= \frac{p_v}{p_a}$$

from ideal gas law,

$$\text{atm pressure, } p = p_a \cdot R_a \cdot T$$

↳ gas const for moist air.

$$p_a = \frac{p}{R_a T}$$

$$R_a = (1 + 0.604 q_v) R_d$$

$$R_d = 287.5 \text{ Joule/kg}\cdot\text{K}$$

R_d = gas const for dry air

e , R_v = gas const for water vapour

vapour pressure exerted by water vapour

$$e = p_v R_v T$$

Here, R_v = gas const for water vapour

$$\frac{R_d}{0.622} \approx \frac{R_e}{0.622}$$

$$R_a \approx R_d$$

1% error diff

$$\rho_v = \frac{e}{R_v T}$$

$$q_v = \frac{R_a}{R_v} \cdot \frac{e}{P} = 0.622 \cdot \frac{e}{P}$$

ii. Relative humidity, $R_h = \frac{e}{e_s} \times 100$

e_s is a func of temp.

$$e_s = 611 e^{\frac{17.27T}{237.3+T}} \quad (\text{Pascal } \rightarrow \text{NTP})$$

Actual vapour pressure : saturation pressure at wet dew point temp

$$e = 611 e^{\frac{17.27 T_d}{237.3 + T_d}}$$

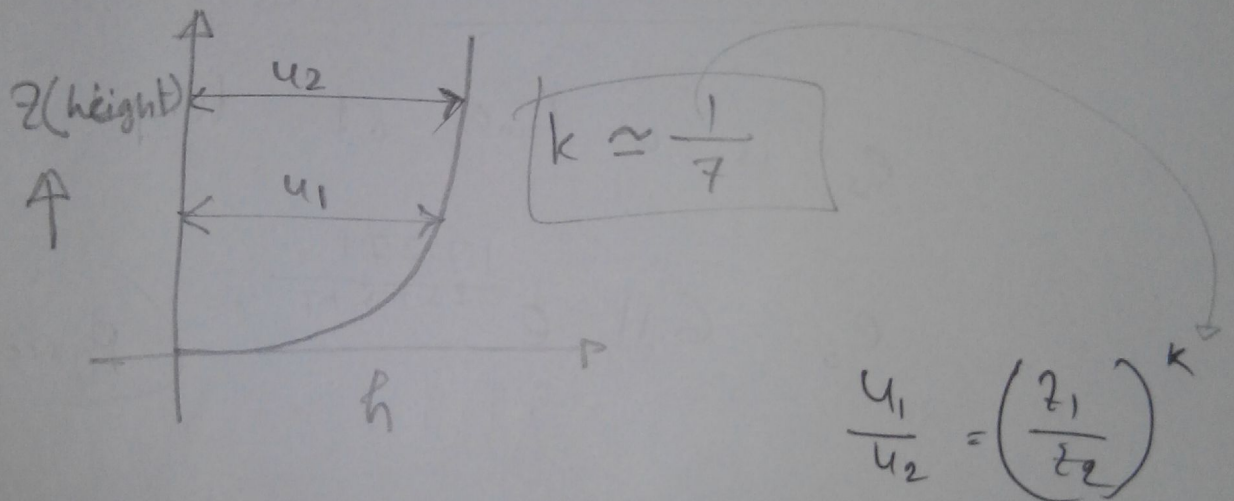
$T_d =$ dew point temp

- the temp at which air would just become saturated at a given specific humidity.

(wet bulb, dry bulb
 सिलेब्रिक साइकलॉमिटर
 सिलेब्रिक वेट बुल्ब
 टेम्प + साइकलॉमिटर
 100% साइकलॉमिटर

* Wind speed : \rightarrow Anemometer \rightarrow wind speed

\Rightarrow ଏକ କଳିବ୍ରତ କୁଳ୍ମ କାଲିବ୍ରେଟେଡ୍ କାଲିବ୍ରେଟେଡ୍ wind speed ମାପିବା ପାଇଁ ବ୍ୟବହୃତ ,



① Logarithmic

② power law profile .

Data given:

Type 3: Humidity & Pressure

atm. pressure, $p = 100 \text{ kPa}$,

$t = 20^\circ\text{C}$

Dewpoint temp, $T_d = 16^\circ\text{C}$

Find: vapour pressure, $e = ?$

rel. humidity, $R_h = ?$

$P_a = ?$

Solⁿ:

$$e_s = 611 e^{\frac{17.27 \times 20}{237.3 + 20}}$$

$$= 2339 \text{ Pascal}$$

$$e = 611 e^{\frac{17.27 \times 16}{237.3 + 16}}$$

$$= 1819 \text{ Pa. (Ans)}$$

Relative humidity, $R_h = \frac{e}{e_s} \times 100$

$$= \frac{1819}{2339} \times 100$$

$$= 78\%$$

$$P_a = \frac{P}{R_a T}$$

$$R_a = (1 + 0.608 q_v) 287$$

$$\begin{aligned} q_v &= 0.622 \frac{e}{p} \\ &= 0.622 \times \frac{1819}{100 \times 10^3} \\ &= 0.0113 \end{aligned}$$

$$\begin{aligned} R_a &= (1 + 0.608 \times 0.0113) \times 287 \\ &= 289 \text{ J/kg-K} \end{aligned}$$

$$P_a = \frac{100 \times 10^3}{289 (20 + 273)}$$

$$= 1.18 \text{ kg/m}^3$$

* Problem:

Data given, $p = 101.1 \text{ kPa}$, $T = 25^\circ\text{C}$, $T_d = 20^\circ\text{C}$

find : e , R_h , q_v , ρ_a .

* Problem:

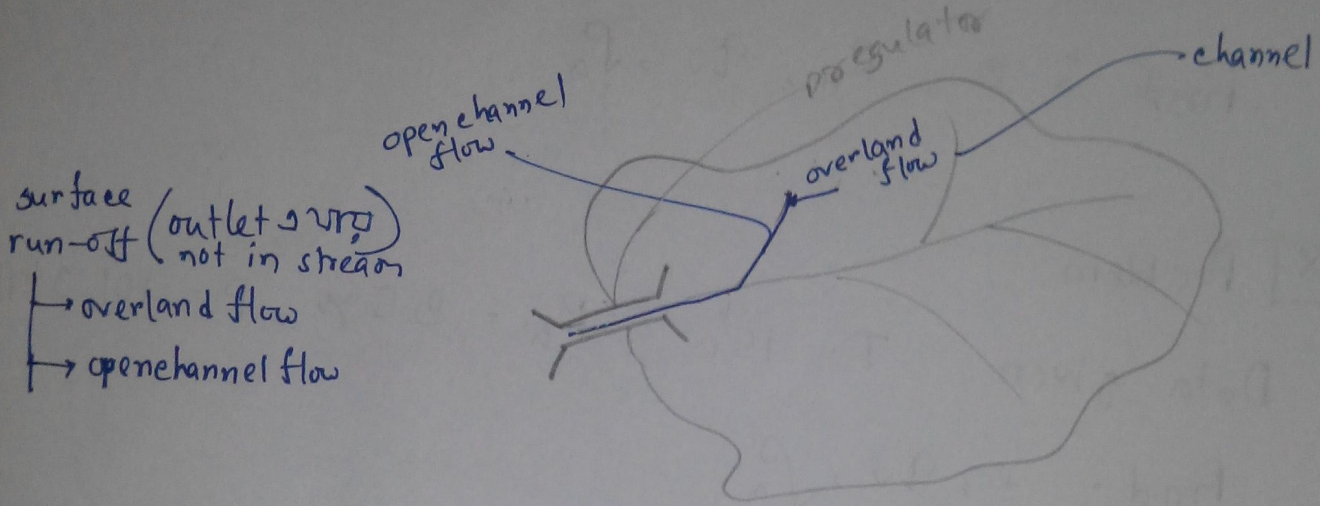
Data given: $T = 15^\circ\text{C}$, $R_h = 35\%$, $p = 101.3 \text{ kPa}$,

find: e , q_v , ρ_a .

CT \rightarrow ke 2 - ~~12~~ ~~13~~ (short ques.)

next ~~14~~ ~~15~~ (19 ~~20~~ ~~21~~)

* Rainfall run-off relationship :



⊗ regulator design

⊗ pump द्वारा जाता drainage बनाते हैं, पानी को,

⊗ जहाँ run-off जाता बाहर।

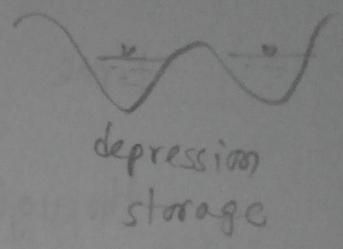
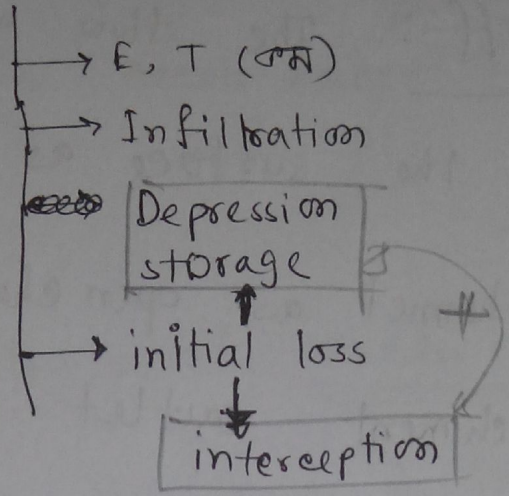
run-off is the overland flow.

→ head build up होता है।

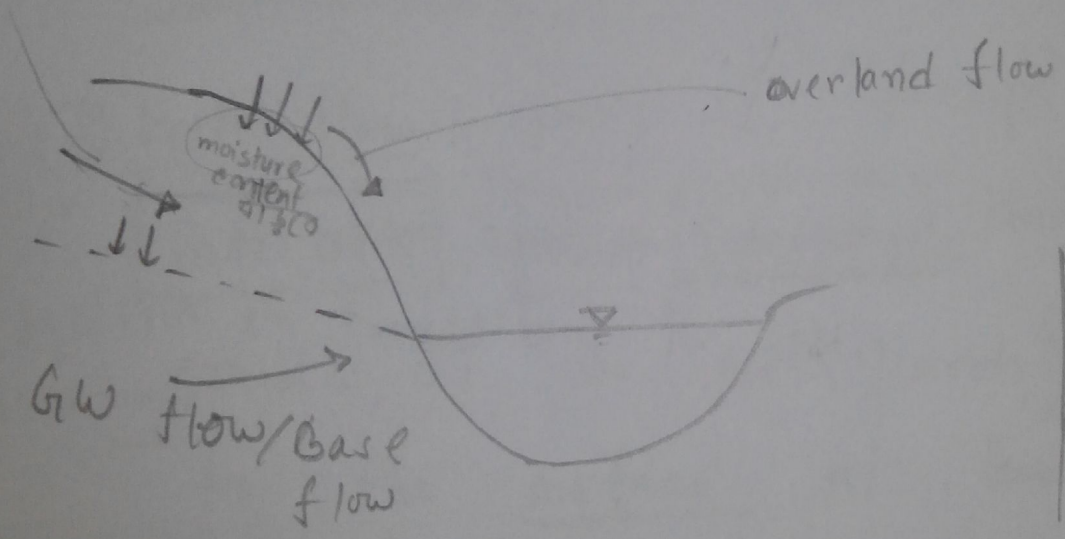
→ E + Infiltration + Depressions fill होता है जो 3 तरह का run off,

defⁿ: Run-off is draining or flowing off precipitation from a catchment area through a surface channel.

$R = P - \text{Losses}$ (water retention at (y))



Interflow



dry season ↓
base flow (पता
गानि न गानि)

Routes of run-off :

- overland flow
- GW flow
- Interflow

Surface run-off: The flow which travels all the time over the surface as overland flow and through channel as open channel flow to reach the catchment outlet.

Both overland flow + open channel flow is surface run-off.

(*) to the stream \rightarrow surface run-off
to the outlet of catchment \rightarrow overland flow

(*) Difference betⁿ overland flow and surface run-off.

(*) Rainfall as in ~~access~~ excess of flow is excess rainfall.

(*) Overland flow → the excess rainfall which moves over the land surface to reach the smaller channels.

(*) Interflow - part of precipitation ~~low~~ that infiltrates, moves laterally through upper crusts of the soils, and returns back to the surface at location away from the point of entry.

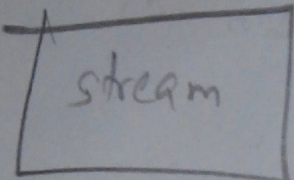
slow + quick interflow

unsaturated zone →

(*) Base flow - the delayed flow that reaches the stream essentially as GW flow.

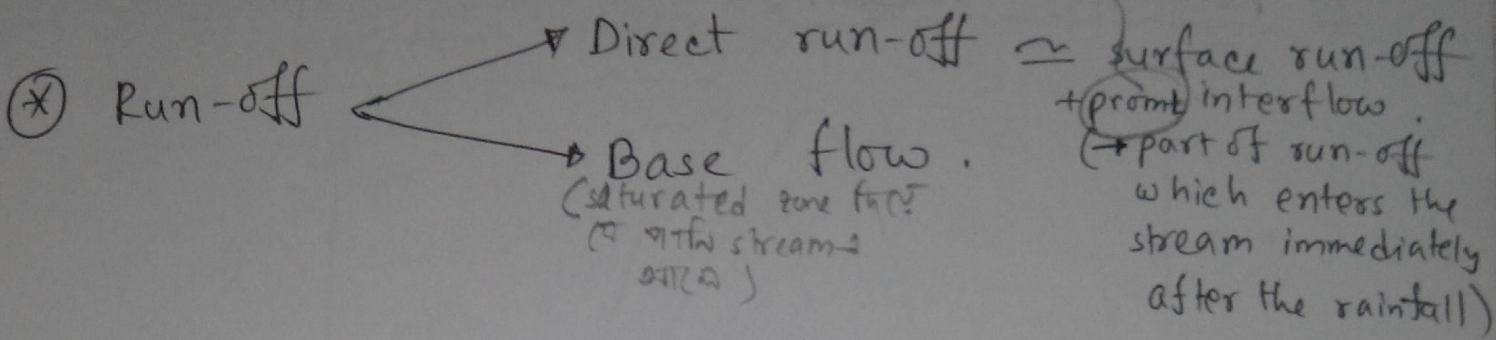
→ saturated zone →

Show in a diagram diff routes (paths,

to a  due to storm ??

→ surface runoff X X

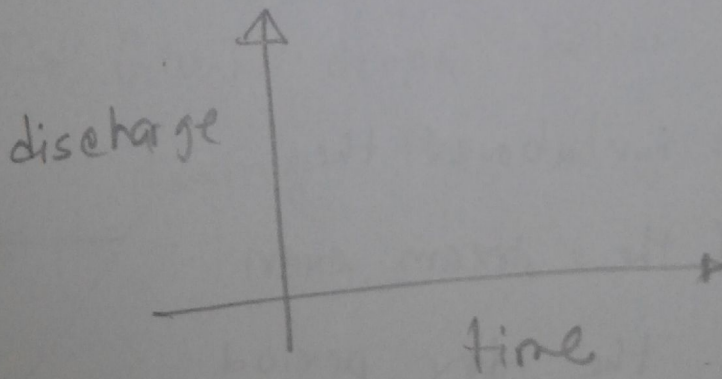
(see stream → outlet, outlet → river)



prompt interflow negligible.

अतः, direct run-off \approx surface-runoff

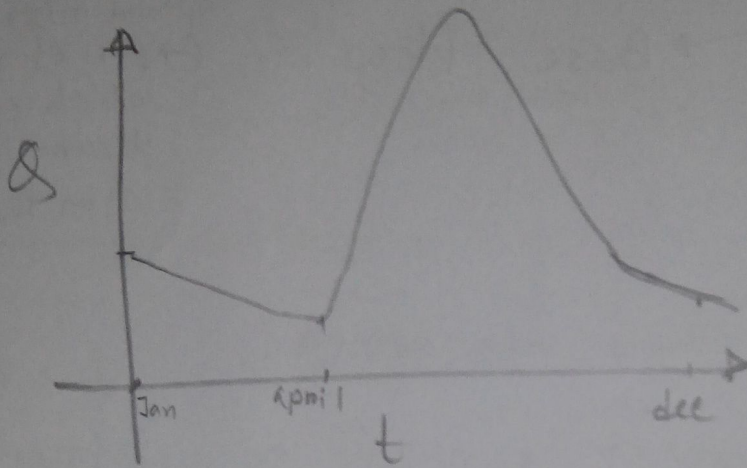
⊗ Classification of stream (based on annual hydrograph) :



hydrograph
 (प्राथमिक वर्षा वर्षा)
 (water year
 12 12)

water yr \rightarrow starts ^{from} 1st april
 to 31th march

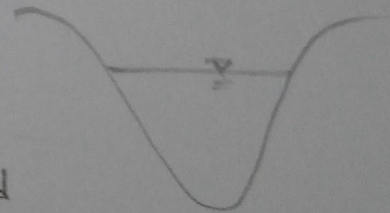
i. perennial stream:



→ always carries some flow.

→ considerable base flow throughout the year. (moonsoon ৩০%
বাকী base flow (নদ সারা বছর
জানিতই থাকবে))

→ GW T is above the bed of the stream even during the dry period



Bd (border) trans-river

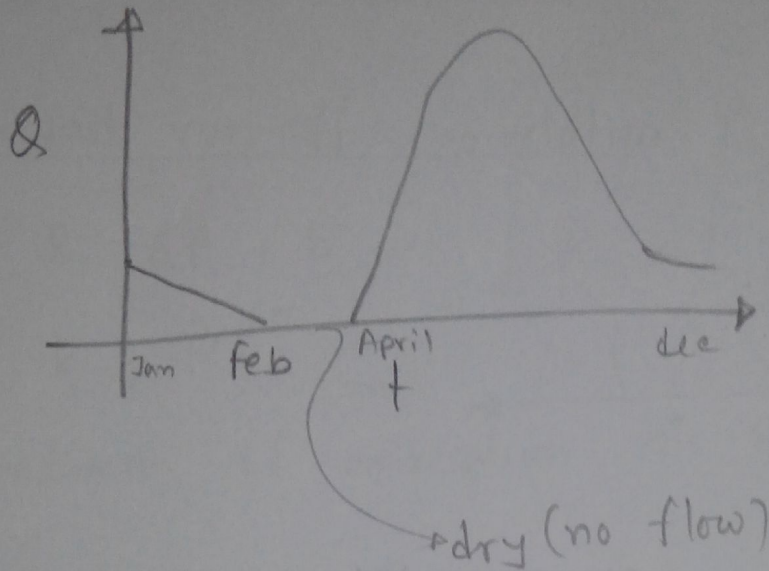
① India → 54

Myanmar → 3

Total = 57

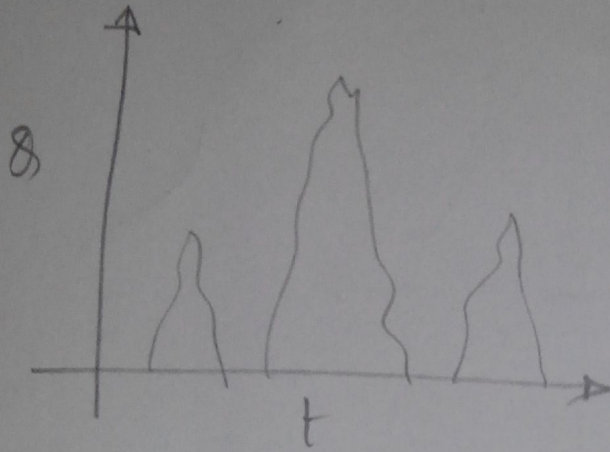
⊗ Comparives perennial stream/river to intermittent stream

ii Intermittent stream :



- becomes dry during the dry period
- limited contribution from GW.
- GW T drops below the bed level during the dry season.

iii. Ephemeral stream :



→ No base flow contribution .

→ Series of short duration spikes marking flash flows in response to storm.

→ becomes dry soon after the end of the stream flow .

* Estimation of run-off

i. Rain fall run-off co-relation:

$$R = aP + b$$

$$R = \beta P^m$$

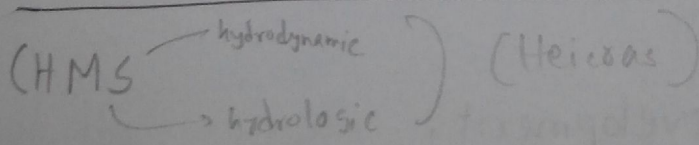
Co-efficient of co-relation 0.6-1 ज० का० रू० के० okay

ii. Empirical equⁿ: ज० catchment ज० का० developed

ज० का० applicable

(Bd ज० का० न०)

iii. Water simulation / hydrologic model:



$$R = P - L \rightarrow \text{infiltration loss (E+T loss)}$$

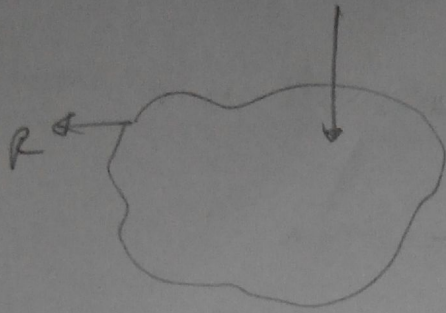
\rightarrow depends on soil, vegetation etc (bas^{ic} characteristics)

⊕ ए० ड० software develop ज० का० गा०

elev. data फु० रू०

स० catchment ज० का० R ज०

$$R = P - L$$



input \rightarrow climate & basin characteristics
+

output \rightarrow runoff

hydrologic model prepare

HEC-HMS

SWAT (Soil water assessment tool)

Standform Watershed Model (SWM) \rightarrow

MIKE-SHE

commercial model catchment

characteris data, run-off

Step:

(a) Model development: describe all relationships.
input = data
output = runoff.

(b) Model calibration: (time input, output both known) Then adjust

ଯଦି measured & simulated value match ନଥାଏ

ତେବେ ଏହା ପାଇଁ calibration.

acceptable range of adjustment →

(c) Model Validation / verification:

model calibrate କରାଯାଇ ପରେ validation କରାଯାଏ ।

(d) Application of model:

() ଯଦି data change ହୁଏ ତେବେ simulate କରାଯାଏ

ଯଦି କୌଣସି କାରଣରୁ run-off change ହୁଏ ।

iv. SCS - CN method : → simple + predictable + conceptual method.

Soil conservation service

curve no

→ India adopt २६.०६

from continuity,

$$P = R + I_a + F_a \quad \text{--- (1)}$$

Initial loss

Infiltration loss

hypothetic used,

$$\frac{R}{P - I_a} = \frac{F_a}{S} \quad \text{--- (2)}$$

Actual infiltration

[perfectly impervious surface / no infiltration loss]

max^m potential run-off

max^m potential retention infiltration

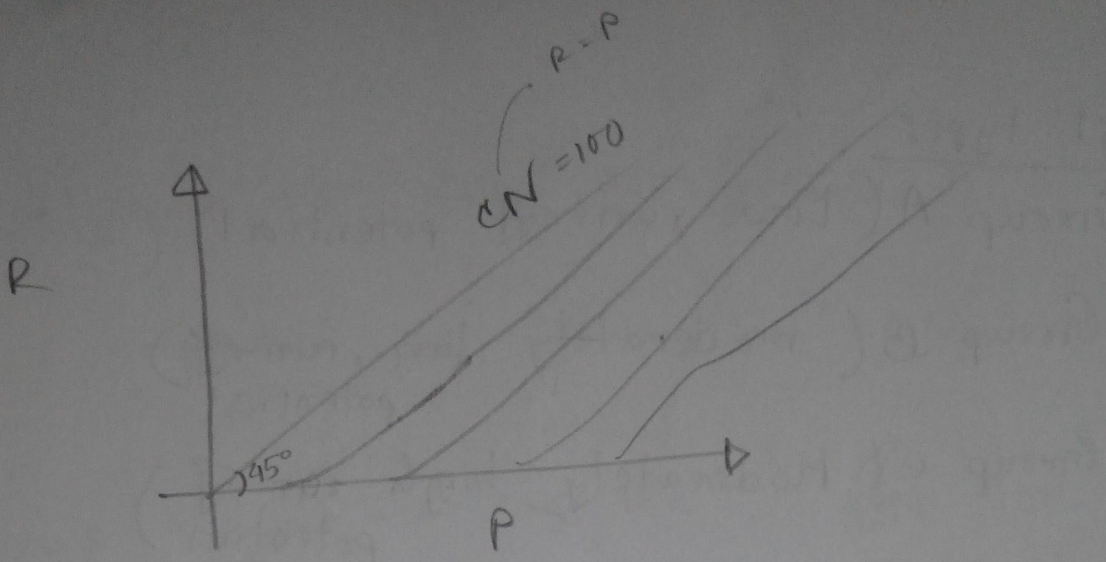
Hw →

$$R = \frac{(P - I_a)^2}{P - I_a + S}$$

Again, $I_a = 0.2S$ [SCS no. of catchment area to study]

$$\therefore R = \frac{(P - 0.2S)^2}{P + 0.8S}$$

$$R = f(P, S)$$



Curve number = $\frac{R}{P}$

$S = \frac{1000}{CN} - 10$ S is in inch

$S = \frac{25400}{CN} - 254$ S is in mm

CN ଯେତେ ଉଚ୍ଚ ହେବ, CN ସମାନ S ସମାନ,

Then P, S ସମାନ (ମ) ସମାନ R ସମାନ ଯାଏ ।

- CN depends on
- ① soil type (sandy, clay, etc)
 - ② Antecedent moisture condition. (AMC)
already wet ଉଚ୍ଚ (runoff > dry runoff)
 - ③ Land use
(urban, forest, road, wheat crop, rice crop etc)

① soil type:

Group A (Low run-off potential) (sandy type soil)

Group B (moderately low run-off potential)

Group C (Moderately high run-off potential)

Group D (High value potential) (plastic clay)

or a table etc show differ.

② Antecedent moisture condition (AMC):

AMC group	Total 5 day antecedent rainfall (mm)	
	Dormant season	Growing season
I (dry)	< 13	< 36
II (avg)	13 - 28	36 - 53
III (wet)	> 28	> 53

or (see a table of value 2 (at avg condition of))

CN II

⊗ dry season s.t. soil water content ?

= empirical eqⁿ betⁿ CN_{II} , CN_I , CN_{III}

⊗ Table 5.6(a) → page 188

(CN for AMC II
s.t. soil) read
(value)

⊗ weighted avg $CN = ?$ (method 2) (no. of area condition same at)

$$\boxed{\otimes} \quad CN_{I} \text{ (dry)} = \frac{CN_{II}}{2.281 - 0.01281 \cdot CN_{II}} \quad (\text{value})$$

$$\text{and } CN_{III} \text{ (wet)} = \frac{CN_{II}}{0.427 + 0.00573 \cdot CN_{II}}$$

⊗ soil type → soil A, B, C } → soil type
→ road, road, residence } → land use

$$= CN_{II}$$

→ Then conversion CN_{II}

$$\rightarrow S = \boxed{} \rightarrow R = \boxed{}$$

* India modified for black soil.

* Problem 5.13 (page 235)

(a) Urban land

avg antecedent moisture condition

from Table 5.6(c)

Land use	Total %	Soil group B (95%)			Soil group C (40%)		
		%	CN	Product	%	CN	product
Residential	80	28%	85	2380	32%	90	2880
Paved road	20	7%	98	686	8%	98	784
				<u>3066</u>			<u>3664</u>

$$\text{average } CN_{II} = \frac{3066 + 3664 + 2330}{100} = 90.6$$

as percent = 90.6

$$\text{Runoff} = \frac{P \left[150 \text{ mm} - 0.2 \times S \right]^2}{150 + 0.8 \times S}$$

26.35

$$= 122.44 \text{ mm}$$

$$S = \frac{25400}{90.6} - 254 \text{ (mm)}$$

$$= 26.35 \text{ mm}$$

Runoff 122.44 mm @ 2170

Soil group D (25%)

7.	CN	product
207.	92	1890
57.	98	490
		<hr/>
		2380

vol^m बाइलिन ,

$$\text{vol}^m = R \times \text{area}$$
$$=$$

vegetation agricultural land

(b) Pasture Land (CN diff, run-off diff)

Land use	Total %	Soil group B (35%)		Soil group C (40%)			
		% CN product		% CN product			
Pasture, poor	100%	35	79	2765	90	86	3440

P 25 85%
Run-off 968

$$CN_{II} = \frac{2765 + 3440 + 2225}{100} = 84.3$$
 (Note: 2225 is likely a typo for 2765)

$$S = \frac{25400}{84.3} - 254 = 47.3 \text{ mm}$$

$$R = \frac{(150 - 0.2 \times 47.3)^2}{150 + 0.8 \times 47.3} = 105.2 \text{ mm}$$

105.2 mm, urban = 122.44

% increase $\left\{ \frac{122.44 - 105.2}{105.2} = 16.3\% \right.$

Soil group D (257)

%	CN	product
25	89	2225

* v. Rational method :

quick ~~run~~ peak runoff value for design.

Peak runoff ~~का~~ peak Q ~~का~~

$$Q_p = C_i A$$

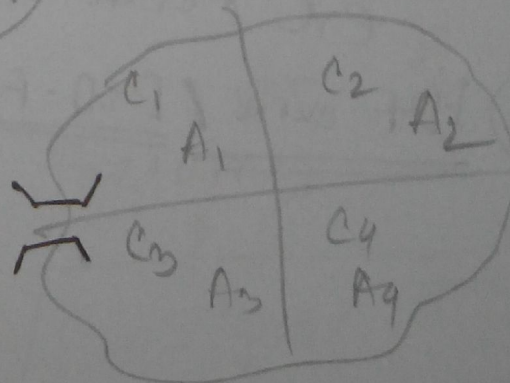
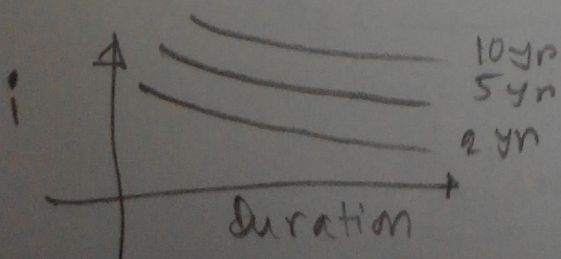
↓ Area of the catchment

C_i = runoff co-efficient
 depends on land use, soil type
 [page 299 7.1(a) table]

rainfall intensity
 $\frac{\text{magnitude}}{\text{time}}$ = rate of P

(*) : जानने सिर??

time कम निम्न intensity ~~का~~
 u को y को



$$C = \frac{\sum C_i A_i}{A_i}$$

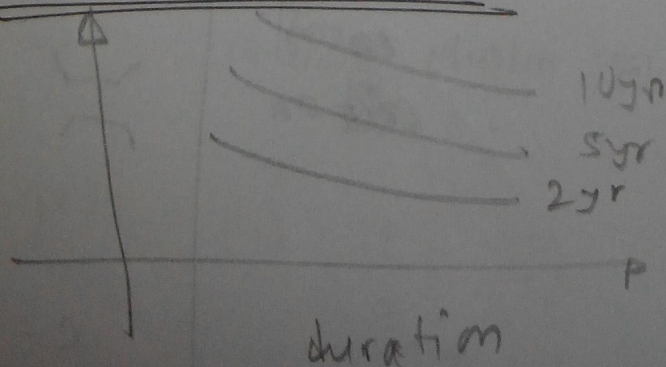
* Short duration (shr) or diff yr or data available,
 diff year or diff max^m value for
 20yr or 20 R random value for.

* regulator design for 7 years for
 value of (for)

→ max value $\frac{1}{20} = 5\%$ probability of
 occurrence (for 20 yr
 (T=20yr) or 5 times)
 → avg " $\frac{1}{5} = 20\%$ " " " "
 (T=2yr) (for 2 yr or
 5 times)

So, it is called return period /
 (how frequently it will come)
 frequency or T or Probability of
 exceedence, P.

* IDF curve / I-D-F relationship:



IPF is represented by an equⁿ.

$$i = \frac{K T^x}{(D+a)^n}$$

frequency (yr)

Duration (hr)

$$\rightarrow \text{cm/hr}$$

$K, x, a, n = \text{const}$
 (depends on catchment + P of catchment)

empirical formula

Here, $D = \text{time of concentration at least}$

time of concentration = time taken for a drop of water from farthest part of the catchment to reach the outlet

$i = \text{mean intensity of precipitation for a duration equal to time of concentration and desired frequency}$

$$\text{Example } (i) = 5 \text{ year } 2 \text{ cm/hr}$$

Kirpichev formula,

$$t_c = 0.01997 L^{0.77} S^{-0.385}$$

min slope max^m length of travel in meter

depends on catchment shape
 (shape \rightarrow $\frac{L}{W}$ \rightarrow $\frac{W}{L}$)

(*) Comparison betⁿ L & e (0.2)

Ex 7.2 :

$$A = 500 \text{ ha}, L = 3000 \text{ m}, T = 25 \text{ yrs.}$$

$$\text{elev. diff, } \Delta H = 25 \text{ m}$$

$$A_1 = 250 \text{ ha}, c_1 = 0.10$$

$$A_2 = 50 \text{ ha}, c_2 = 0.11$$

$$A_3 = 200 \text{ ha}, c_3 = 0.3$$

$$i = \frac{6.311 T^{0.1523}}{(D+0.5)^{0.945}}$$

Soln:

$$c_{\text{avg}} = \frac{250 \times 0.1 + 50 \times 0.11 + 200 \times 0.3}{500}$$

$$= 0.181$$

i calc करके T जमान t_e निकालो।

$$t_e = 0.01947 \times L^{0.77} S^{-0.385}$$

$$(\text{min}) = 0.01947 \times (3000)^{0.77} (0.0083)^{-0.385}$$

$$= 58.3 \text{ min.}$$

$$S = \frac{\Delta H}{L}$$

$$= \frac{25}{3000}$$

$$= 0.0083$$

$$t_e = \frac{58.3}{60} = 0.98 \text{ hrs}$$

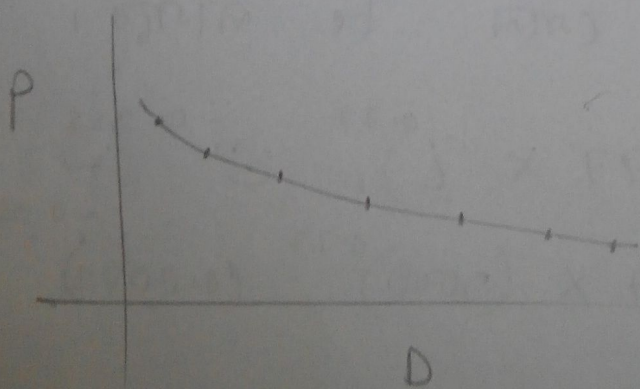
$$i = \frac{6.311 \times (25)^{0.1523}}{(0.98 + 0.5)^{0.945}}$$

$$= 7.11 \text{ cm/hr.}$$

$$Q_p = \frac{0.181 \times (7.1 \times 10^{-2})^{\frac{m}{hr}} \times (500 \times 10^4)^m}{(60 \times 60)^{sec}} \left[Q_p = \frac{C_e i A}{3.6} \right]$$

$$= 17.84 \text{ m}^3/s.$$

ex 7.1: for diff points (Duration, depth)
(not in eqn)



$$t_c = 27.4 \text{ min}$$

D (min)	20	30
P (m)	40	50

27.4 min

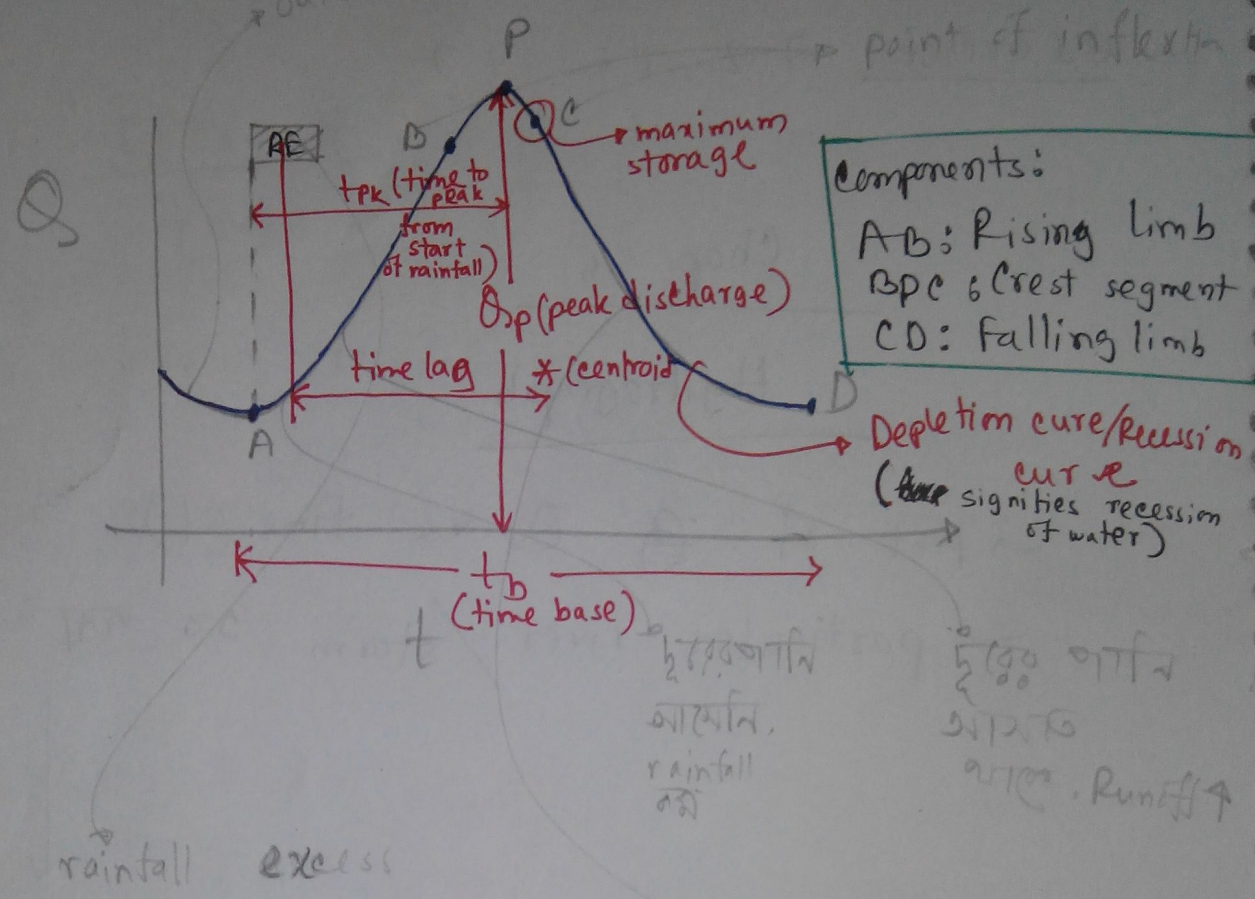
(linear interpolation)

$$t_c = 27.4 \text{ min or}$$

$$\sqrt[n]{}, P = 47.4 \text{ mm}$$

$$i = \frac{47.4 \text{ mm}}{27.4 \text{ (min)}} = 103.8 \text{ mm/hr}$$

outlet of Q on line, discharge rate Q(t)



Components:
 AB: Rising limb
 BPC: Crest segment
 CD: Falling limb
 Depletion curve/recession curve
 (signifies recession of water)

rainfall excess

catchment area 10-2 (km²) represent 1000 km²
 rainfall
 runoff

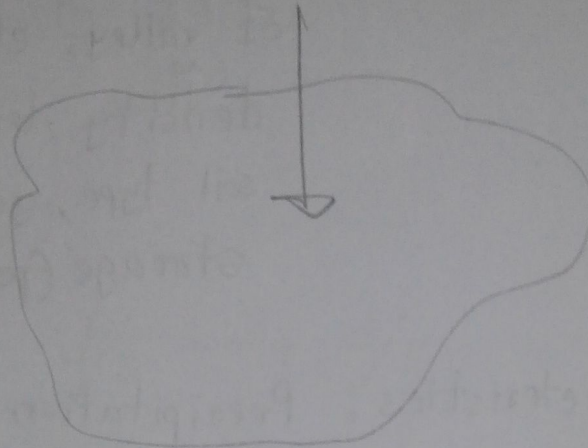
(*) Rainfall - Loss = Run-off

mainly infiltration

(*) $RE = P - \phi \Delta t$
 effective rainfall

Rainfall or P
 which run-off
 generate that
 that's effective
 rainfall or RE or
 rainfall excess.

⊗ surface storage
interflow \cup
GW/base \cup [\cup represents max^m storage]



⊗ Recession depends on basin characteristics.
(doesn't vary much with time)

Elements

Q_p = peak discharge

t_{pk} = time to peak from rainfall start.

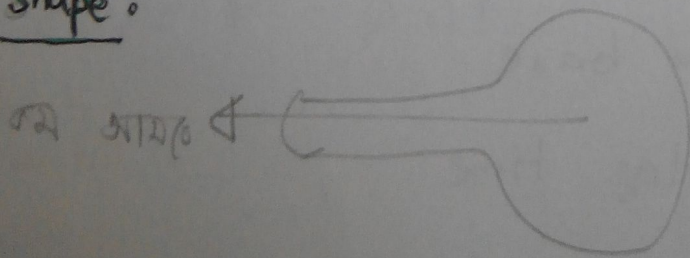
t_b = time base

T_L = Lag time

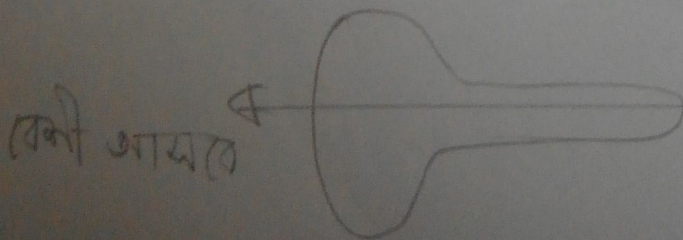
* factors effecting flood hydrograph :

1. Basin characteristics : ^(↑↑) size, ^(↑↑) shape, slope, nature of valley, elevation, drainage density, ^(↑↑) land use and cover, soil type, geological condition, storage (pond, lake)
2. Storm characteristics : Precipitation, intensity, ^(↑↑) duration, magnitude, and movement
3. Climatic ~~the~~ factor : initial loss, evapotranspiration (ET)

shape :



most of the land outlet so ^(↑↑) travel time, to outlet



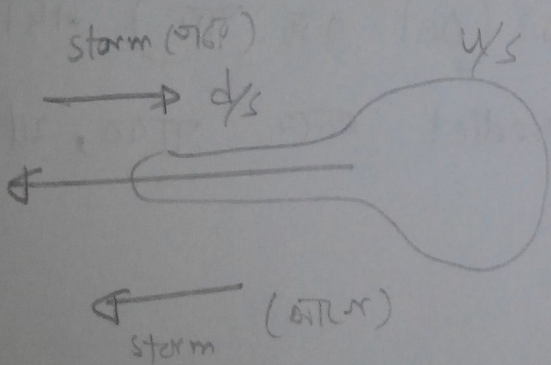
most of the land outlet so ^(↑↑)

Drainage density:

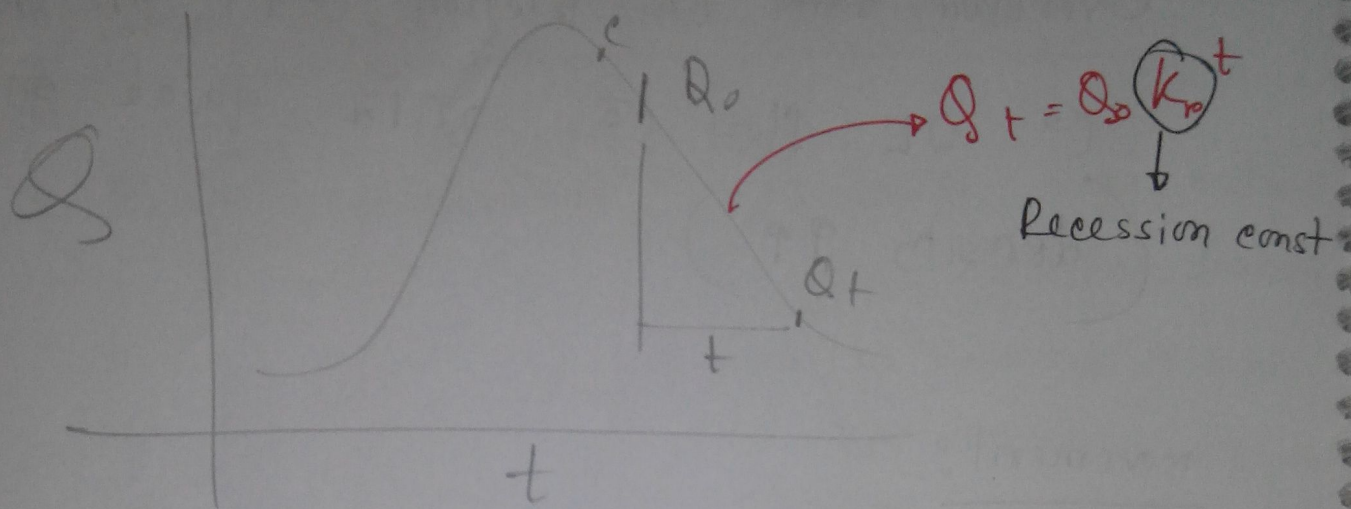
Overland ^{flow} ~~fall~~ ଓ friction ଥିବୁ, ତାହା OCF
ଓ OL flow ଓ ଗାନ୍ଧି quick ଶାନ୍ତି ।
(density ↑↑)

movement:

U/S → d/s [ଆଗରୁ ହେଉ]
d/s → U/S [ପରେ ହେଉ]



⊗ Recession curve ଯେ shape use କର
low flow model prediction କରାଯାଏ
ପାରି ।



→ Q_0, t જાણવા થી Q_t વધારવા
 શક્ય

→ Dry season માં (Oct થી જાન) નદીમાં
 જાણીને Q predict કરવા શક્ય, যদি
 k_r જાણીને,

→ We call it low flow model
 as it is for dry season.

→ Depletion curve develop કરવા જાણીને
 થી માત્ર avg નિષ્પેદા.

$$\frac{Q_t}{Q_0} = k_r^t$$

$$\Rightarrow \log \left(\frac{Q_t}{Q_0} \right) = t \log (k_r)$$

$$\Rightarrow \log (k_r) = \frac{1}{t} \log \left(\frac{Q_t}{Q_0} \right)$$

⊗ K_r consists of K_{rs} (surface storage)

" " K_{ri} (interflow ")

" " K_{rb} (base flow ")

$$K_r = K_{rs} \cdot K_{ri} \cdot K_{rb}$$

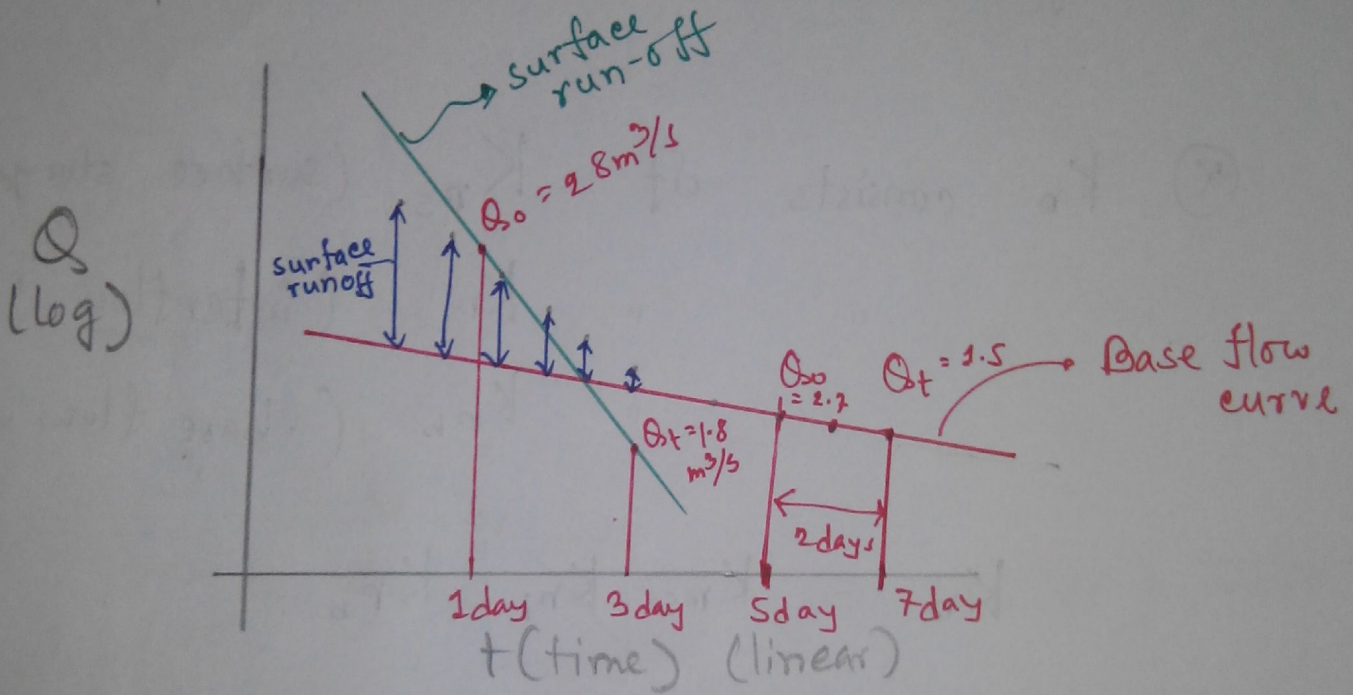
\rightarrow ΔP ΔP (negligible)
 \rightarrow value = 1 \therefore
 $= K_{rs} \cdot K_{rb}$

Ex 6.1

fig 6.4

page 247

semi-log (क्षुण्णित नितो नव)



last 3 point straight line.

Base flow curve/equⁿ, $\frac{Q_t}{Q_0} = k_r b^t$

$$\Rightarrow \log(k_r b) = \frac{1}{t} \log\left(\frac{Q_t}{Q_0}\right)$$

$$\Rightarrow \log(k_r b) = \frac{1}{2} \log\left(\frac{1.5}{2.7}\right)$$

$$\Rightarrow K_{rb} = 0.75$$

⊗ hydrograph analysis = K_n ବା σ_{DT}
 ନୀତି

$$K_{ri} = 1. \text{ [negligible]}$$

Surface run-off, $\frac{Q_t}{Q_0} = K_{rs} b^t$

$$\Rightarrow \log(K_{rs}) = \frac{1}{t} \log\left(\frac{Q_t}{Q_0}\right)$$

$$\Rightarrow \log(K_{rs}) = \frac{1}{2} \log\left(\frac{0.18}{28}\right)$$

$$\Rightarrow K_{rs} = 0.25$$

practise: graph Δ σ_{DT} [problem 6.1]
 (semi log) page 287

⊗ dry season Δ discharge ମାନ(ମ) Δ σ_{DT} ନିର୍ଣ୍ଣୟ
 Δ discharge predict Δ σ_{DT} ବା σ_{DT}

problem 6.2

June 1 $Q = 80 \text{ m}^3/\text{s}$

June 21 $Q = 40 \text{ m}^3/\text{s}$

এর মানে কোন
ঘটিছে নেই।

$K_r = ?$ July 10 এ $Q = ?$

Solⁿ:

$t = 20 \text{ days}$

$Q_t = 40$

$Q_0 = 80$

$$\frac{Q_t}{Q_0} = K_r b^t$$

$$\Rightarrow \log\left(\frac{Q_t}{Q_0}\right) = \frac{1}{t} \log(K_r)$$

$$\Rightarrow \log\left(\frac{40}{80}\right) = \frac{1}{20} \log(K_r)$$

$$\Rightarrow K_r = 0.96$$

June 1, July 1 ,

$$Q_t = Q_0 K_r$$

$$= 80 \times 0.96^{30}$$

$$= 28.34$$

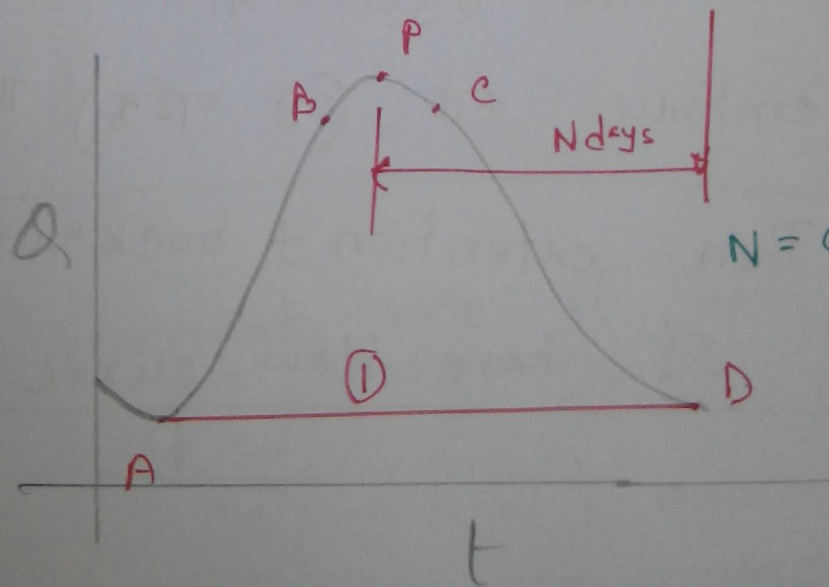
→ direct/surface run-off : আমাদের সৌর প্রবাহন
 স্থিতির দাখে দাখে
 যে জানি আসতে।

→ rainfall hydrograph Δ base flow (৩য় স্তর) আশে
 math ৩.

→ গা, Base flow separate করে actual
 run-off টি বের করে হবে।

⊗ 3 methods of separate base flow:

1. Straight line method :



● DP always
 সোয়া খড়না.

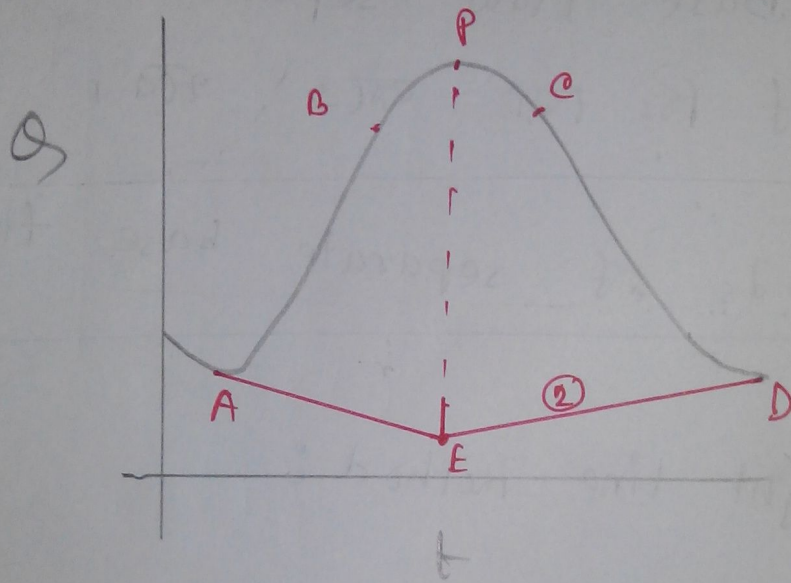
$$N = 0.85 A^{0.2}$$

↓
 Km^২

Disadv

ଏହା କେବଳ rationality ନୁହେଁ,

2. Extention of previous base flow curve

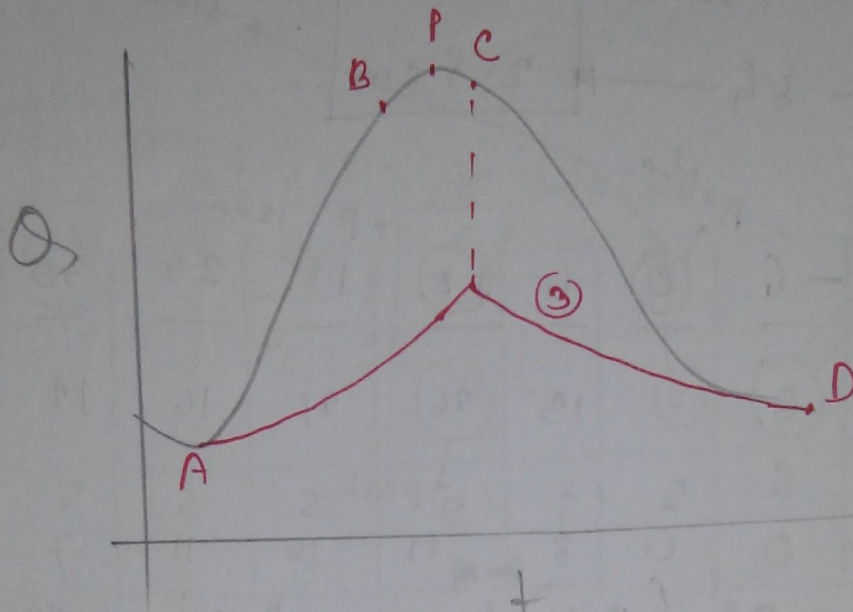


Peak flow ହେଉଥିବା ସମୟର ବାହାରେ base flow
continue ରହେ (E ବାହାରେ). Then connect to D.

3.

Then extention + backward extention
of base flow curve.

3. Backward extension of base flow curve



no such authenticity.

method ① is widely used.

(*)

ϕ_{index} = constant rate of infiltration (loss)

$$\phi = \frac{P - R}{t_e}$$

Precipitation (time 1)
(time 2)
Period of rainfall excess

$\phi_{index} < \text{rainfall intensity} = \text{capacity of infiltration}$ if soil then no run-off
 $\phi_{index} > \text{rainfall intensity} = \text{capacity of infiltration}$ if soil then run off

Example 6.2 [page 250] Estimate PE & ϕ index.

$A = 27 \text{ km}^2$

$D = 4 \text{ h} \rightarrow 3.8 \text{ cm}$

$9 - 8 \text{ h} \rightarrow 2.8 \text{ cm}$

magnitude high
2.8 cm runoff
direct runoff start

base flow
5.165

$A = 0 \text{ hr}$

$P = 12 \text{ hr}$

2.8 cm discharge

$T(\text{hr})$	-6	0	6	12	18	24	30	36	42
$Q(\text{m}^3/\text{s})$	6	5	13	26	21	16	12	9	7
Base flow (m^3/s)	6	5	5	5	5	5	5	5	5
Direct runoff hydrograph (DRH) (m^3/s)	0	0	8	21	16	11	7	4	2

ordinates of DRH
 $\sum O_i = 69$

$N = 0.83 (27)^{0.2}$
 $= 1.6 \text{ days}$

$[N = 0.83 (A)^{0.2}]$

$= 38.5 \text{ hr from peak}$

$(38.5 + 12) \text{ hr} = \boxed{50.5 \text{ hr}}$ from start of P.

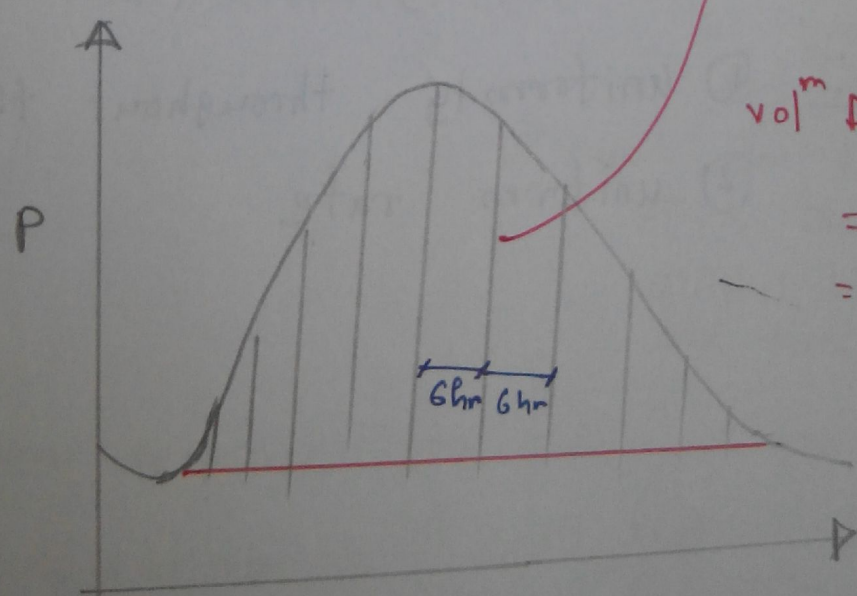
$D = 48 \text{ hr}$

48	54	60	66
5	5	4.5	4.5
5	5	4.5	4.5
0	0	0	0

50.5 hr

এর মানে
(48 গাছ)
তার $D = 48$

$\text{vol}^m \text{ DF} = \text{primal rule}$



$\text{vol}^m \text{ DF} = \sum O_i \Delta t$

$= 69 \times (6 \times 60 \times 60)$
 $= 1.49 \times 10^6 \text{ m}^3$

$RF = \frac{1.49 \times 10^6}{27 \times 10^6} \text{ m}^3$

$= 0.055 \text{ m}$

$= 5.52 \text{ cm}$

$\phi_{\text{index}} = \frac{(3.8 + 2.8) - 5.52}{8} = 0.135 \text{ cm/hr}$

Leel 2

10/04/17

Unit hydrograph

→ Catchment का Q unit hydrograph का
रूप है ।

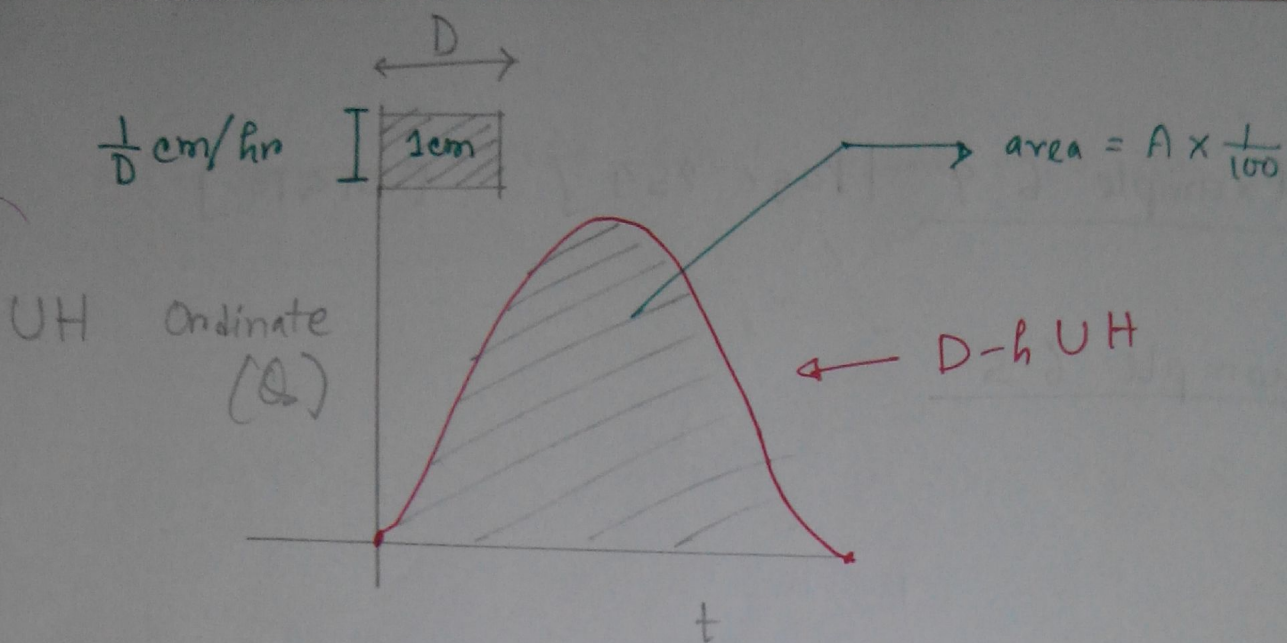
Defⁿ: hydrograph of direct run-off resulting
(R - Infiltration)

from an unit (1cm) depth rainfall excess

occurring uniformly over the basin and

assumption:

at uniform rate for a specified duration (D).



Assumptions:

1) RE is uniformly distributed over the catchment.

2) Rate of RE is const.

3) Time invariance [यदि 6hr hydrograph है तो
its hydrograph of
 $\alpha = 6$ hr.]

But 6hr - 12hr का मान
एक ही ordinate change
होगा। अर्थात्, ordinates
are time invariance]

4) Linear response [यदि कोई linear है तो (+), (-) का जो
बाधा, UH represent DR of 1cm depth

principle of addition

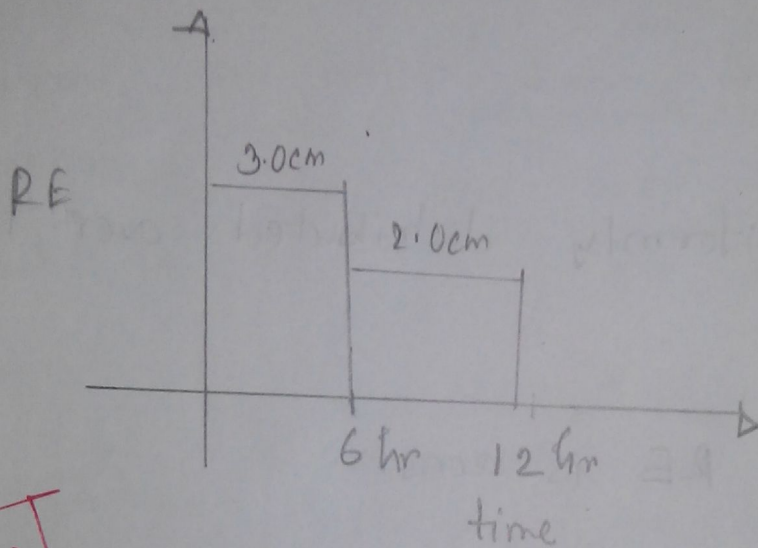
" " multiplication

3 cm का जो मान होगा उसी ordinate का जो
होगा है।
यदि कोई 6hr का है तो corresponding
ordinate का मान होगा]

Example 6.4 [Page 254] [ATWTP]

Example 6.5

Rainfall Intensity & time → hydrograph.



It's rainfall excess (RE) hydrograph.

Time equal interval $\Delta t = 3$ hr

Time (hr)	0	3	6	9	12	15	18	21	24	30	36
Ordinate of 6h UH	0	25	50	85	125	160	185	160	110	60	
DRH 1 (3cm)	0	75	150	255	375	480	555	480	330	180	
DRH 2 (2cm)	-	-	0	25x2 = 50	50x2 = 100	170	250	370	320	220	
DRH opd (m ³ /s)	0	75	150	305	475	650	805	850	650	400	

5.18 stem. 60. 5.18 effect count 201.

42	48	54	60	(65)(66)	(69)	(75)
36	25	16	8	(53)(2.7)	0	0
101	15	48	24	(81)	0	
140	12	50	32	(88)	10.6	
227	197	29	75		10.6	

5.18 DRH 2.55 5.18
LS 5.18 value 201

(*)

t vs cumulative rainfall (କୃତ୍ୟ ସମୟ)

or t vs infiltration " " "

$$RF = RF - \text{infiltration.}$$

ଉତ୍ପାଦ UH ପାଇ

ଯଦି ଦୁଇଟି / ତିନି storm ଓ ତ data (କୃତ୍ୟ)

ଥାରେ ଯୋଗ କରାଯାଏ ।

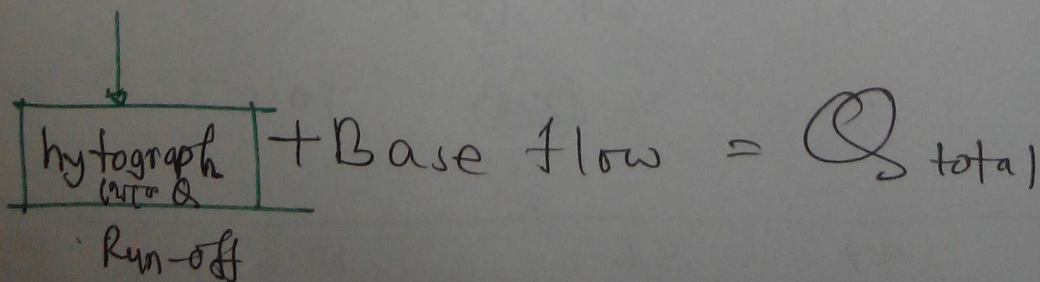
Q_{total} ଓ catchment ପାଇ ।

Data:

→ Rainfall pattern

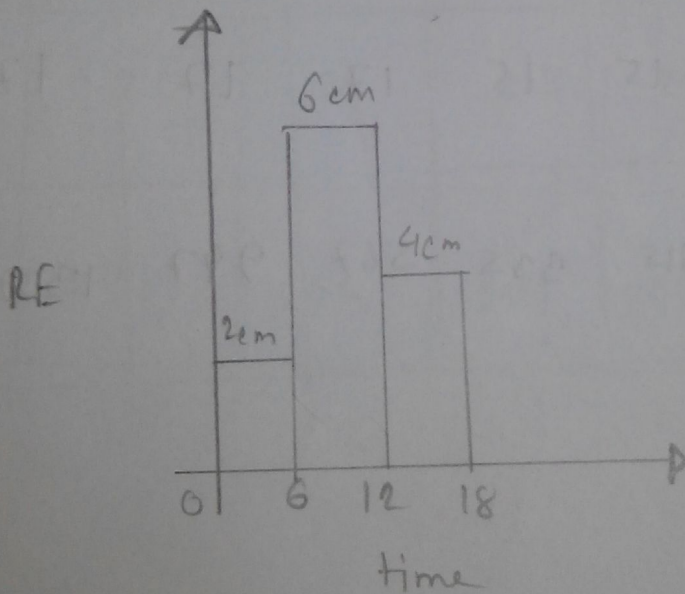
→ loss

→ UH



Example 6.6

Time (hr)	0	6	12	18
ΣP	0	3.5	11	16.5
<u>time</u>		<u>P (cm)</u>	<u>Δt</u>	<u>RF (cm)</u>
0-6hr	3.5	$(11-3.5)$ $= 7.5$	1.5	$3.5-1.5$ $= 2$
6-12hr		7.5 Sem	1.5	$7.5-1.5$ $= 6$
12-18hr		5.5 Sem	1.5	$5.5-1.5$ $= 4$



base flow = $15 \text{ m}^3/\text{s}$ beginning, increasing $2 \text{ m}^3/\text{even } 12 \text{ hr}$

<u>T(hr)</u>	0	3	6	9	12	15	18
<u>Ordinate</u>	250	50	25	85	50	125	185
DRH 1 (2cm)	0	50 (25x2)	100	170	250	320	370
DRH 2 (6cm)	.	.	0	25x6 =150	300	510	750
DRH 3 (4cm)	0	100 (25x4) =100	200
DRH	0	50	100	320	550	930	1320

Lee 13

29/04/14

30	36	42	48	54	57	60	63	69
60 110	36 60	42 36	48 25	16.8	12.8	8	5.30	
220	120	72	50	32	16	0		
960	660	360	216	150	96	31.8	(5.3x6) =31.8	≈ 32
740	640	440	240	144	100	48	(12x4) =48	
1920	1420	872	506	326	100 212	80		
19	21	21	23	23	25	25		
1939	1441	893	529	349	237	105		

Next example

→ ±10% allow error isolated flow hytograph
ନିମ୍ନେ ଥାଏ ।

→ BF estimate କରାଯାଏ ।

BF Beginning

BF end ($0.83 A^{0.2}$) point (କରା ଯାଏ)

ସାମାନ୍ୟ value straight line ଚିତ୍ରଣ କରାଯାଏ ।

→ BF subtrat କରାଯାଏ ।

→ RE ଗଣନା = $\frac{DRH \text{ ordinate sum}}{\text{area}}$ = RE (x)

n cm ଗଠ
ନିମ୍ନ hytograph

→ $\frac{RE}{\text{area}}$ = n ଦିଆଯାଏ କିମ୍ବା ନିମ୍ନ

Example 6.7

$A = 423 \text{ km}^2$

Arbitrarily (BF) 0.5 (0.5)
 $90 \text{ hr} \rightarrow 2.5 \text{ (2.5)}$
 $18 \text{ hr} \rightarrow 0.5 \text{ (0.5)}$

exam const time 90 hr (same 90 hr)

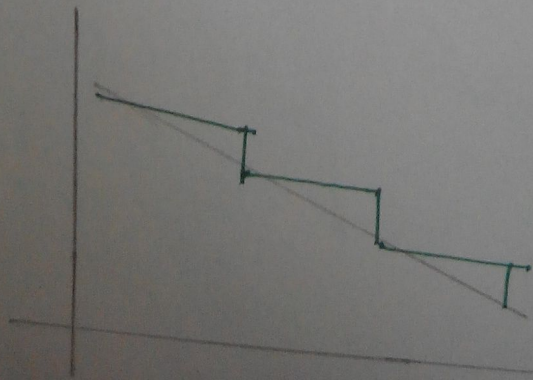
$T(\text{hr})$	-6	0	6	12	18	24	30	36	42
$Q(\text{m}^3/\text{s})$	10	10	30	87.5	115.5	102.5	85	71	59
BF	10	10	10	10.5	10.5	10.5	11	11	11
$Q - \text{BF}$ DRH ordinate (m^3/s)	0	0	20	77.5	105	92	74	60	48
UH ordinate (DRH/Q)	0	0	6.7	25.7	35	30.7	24.7	20	16

Peak \rightarrow highest discharge

$N = 0.83 (423)^{0.2} = 2.78 \text{ days} = 66.76 \text{ hours} = 66.8 \text{ hr.}$

$+ 18 \text{ (Peak)}$

 84.8



stepping

BF
End

48	54	60	66	72	78	84	90	96	102
47.5	39	31.5	26	21.5	17.5	15	12.5	12	12
11.5	11.5	11.5	12	12	12	12.5	12.5	12	12
36	27.5	20	14	9.5	5.5	2.5	0	0	0
12	9.2	6.7	4.7	3.2	1.8	0.8	0	0	0

1st $DR = 591 \times 6 \times 60 \times 60 = 12.76 \text{ Mm}^3$

$\text{vol}^m \text{ of DRH} = 591 \times 6 \times 60 \times 60 = 12.76 \text{ Mm}^3$

ordinate sum

$\checkmark RF = \frac{12.76 \times 10^6}{423 \times 10^6} = 0.03 \text{ m} = 3 \text{ cm}$

ex : 6.8 (catchment area, time base = ??)

peak = given

$$P = 4$$

$$BF = v \text{ (const)}$$

$$Q - BF = DR$$

a Depth = 5.9, loss = $0.3 \text{ cm/h} \times 3 \text{ h} = 0.9 \text{ cm}$
of RF $\therefore RF = 5.9 - 0.9 = 5 \text{ cm}$

Peak of 3h Flood hydrograph = $270 \text{ m}^3/\text{s}$
BF = $20 \text{ m}^3/\text{s}$

Peak of 3h DRH = $250 \text{ m}^3/\text{s}$

$$\therefore \text{UH} = \frac{250}{5} = 50 \text{ m}^3/\text{s}$$

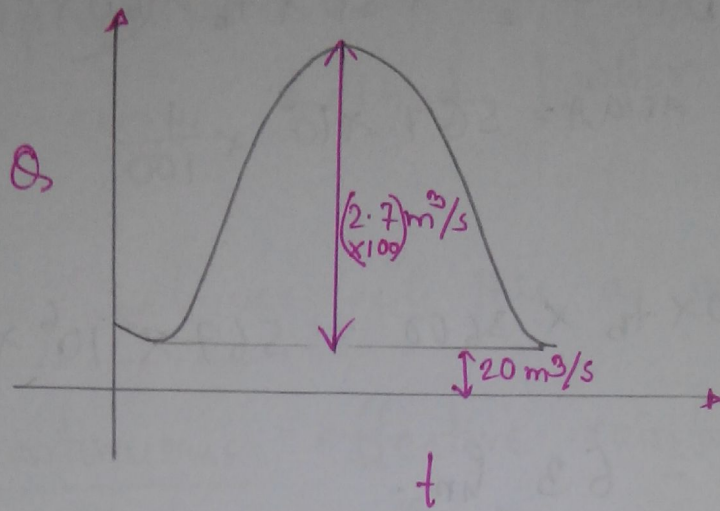
b vol^m of catchment = area of UH = area of triangle

$$\Rightarrow (567 \text{ km}^2 \times 1 \text{ cm}) = \frac{1}{2} \times (t_b \times 60 \times 60) \times 50 \frac{\text{m}^3}{\text{s}}$$

$$\Rightarrow (567 \times 10^6 \times \frac{1}{100}) \text{ m}^3 = \frac{1}{2} \times t_b \times 60 \times 60 \times 50$$

$$\Rightarrow t_b = 63 \text{ hr.}$$

Lec 14



$$P = 5.9 \text{ cm}$$

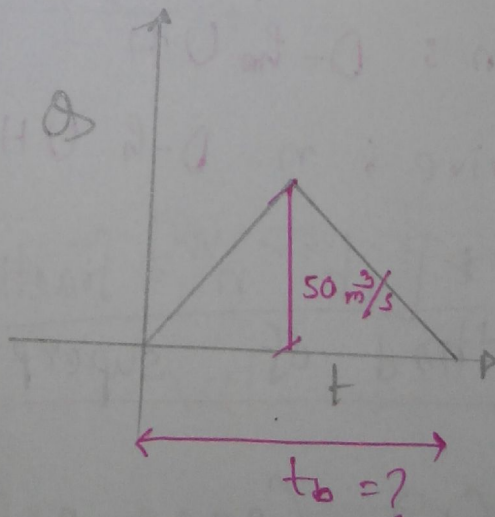
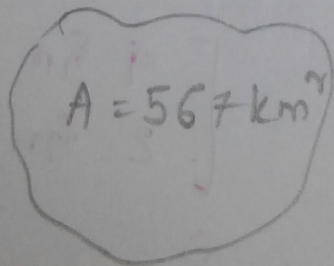
$$\phi_{\text{index}} = 0.3 \text{ cm/hr}$$

$$\therefore RE = 5.9 - 3 \times 0.3 = 5 \text{ cm}$$

a Peak of ~~Q~~ DRH = $270 - 20 = 250 \text{ m}^3/\text{s}$.

$$UH = \frac{250}{5} = 50 \text{ m}^3/\text{s}$$

b



catchment area \times 1 cm = triangle area (area of UH)

vol^m of DH = $\frac{1}{2} \times 50 \times t_b \times (60 \times 60)$ ↗ hr-2 correction

catchment area, $A = 567 \times 10^6 \times \frac{1}{100}$

$\therefore \frac{1}{2} \times 50 \times t_b \times 3600 = 567 \times 10^6 \times \frac{1}{100}$

$\Rightarrow t_b = 63 \text{ hr.}$

change:

$t_b = 60 \text{ hr}$ $\frac{2}{3} \text{ m}$ catchment area = ?

⊗ Given: D-h₀ UH
Derive 6 m D-h UH.

[4 hr UH (not)
2 hr UH (not)
∴ n = 1/2]

[if 'n' = fraction of]
[Method of superposition] use [1 hr UH]

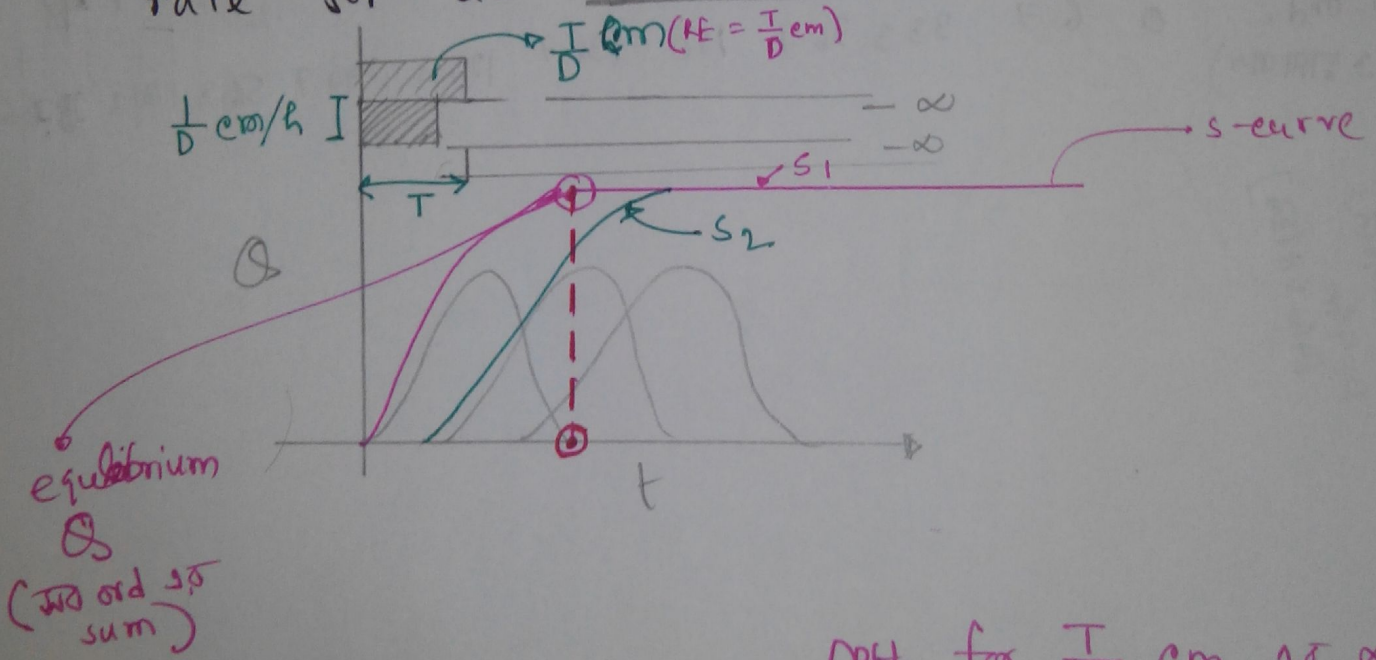
Ex: 6.9 (page 26)

methods

1. Superposition [n = integer]
2. S-curve method [when n = fraction + integer both]

S-curve / S-hydrograph: It is a hydrograph produced

by continuous effective rainfall at a constant rate for an infinite period.



$$S_1 - S_2 = DRH \text{ for } \frac{T}{D} \text{ cm of rain}$$

$$\therefore \frac{S_1 - S_2}{\frac{T}{D}} = UH \text{ ord.}$$

(X) यदि 4 hr UH 3 4 hr ग.कार वापस DR sum
 (X) " " " " 2 hr " " (0+4+8+...) hr
 वापस,

$$\text{vol}^m \text{ of DH} = \frac{1}{2} \times 50 \times t_b \times (60 \times 60)$$

↗
hr to m conversion

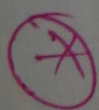
$$\text{catchment area, } A = 567 \times 10^6 \times \frac{1}{100}$$

$$\therefore \frac{1}{2} \times 50 \times t_b \times 3600 = 567 \times 10^6 \times \frac{1}{100}$$

$$\Rightarrow t_b = 63 \text{ hr.}$$

change:

$$t_b = 60 \text{ hr} \quad \frac{2}{m} \text{ catchment area} = ?$$



Given: D-hr UH

Derive 6 m D-hr UH.

$$\left[\begin{array}{l} 4 \text{ hr UH (not)} \\ 2 \text{ hr } \end{array} \right] \therefore n = \frac{1}{2}$$

Method of superposition use if 'n' = fraction of

example 6.11

$T = 12 \text{ hr}, D = 4 \text{ hr} \therefore \frac{T}{D} = 3$

UH starts with 0 & ends up with 0

Time (hr)	0	4	8	12	16	20	24	28	32	36	40	44
Ordinate of 4 hr UH	0	20	80	130	150	130	90	52	27	15	5	0
S_1 curve	0	20	100	230	380	510	600	652	679	694	699	699
S_2 curve	-	-	-	0	20	100	230	380	510	600	652	679
$S_1 - S_2$	0	20	100	230	360	410	370	272	169	94	47	20
UH ord. ($\frac{T}{D} = 3$ क्षणान्तर)	0	6.7	33.3	76.7	120	136.7	123.3	90.7	56.3	31.3	15.7	6.7

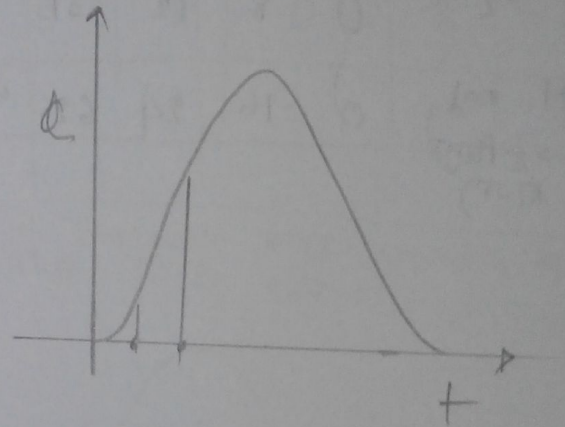
$T = 12 \text{ hr}$
 $(12 \text{ hr } f_{UH}(t))$
 $D = 4 \text{ hr}$
 $[4 \text{ hr } UH f_{UH}(t)]$



(एक math 3 4 hr UH, and 4 hr उच्च होना, जो 5-curve easily लाता है)

48	52
0	0
699	699
694	699
5	0
1.7	0

[आपको 2hr UH चाहिए, But
 ex: 6.12 value 4hr UH चाहिए, क्या
 graph की रकब?]
 जो जो निसाना = graph जो interpolate करेगा,
 (not linear interpolate)



ex: 6.12

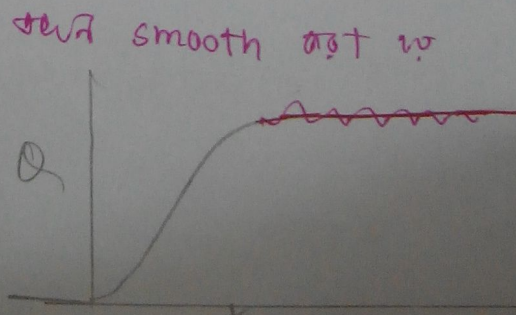
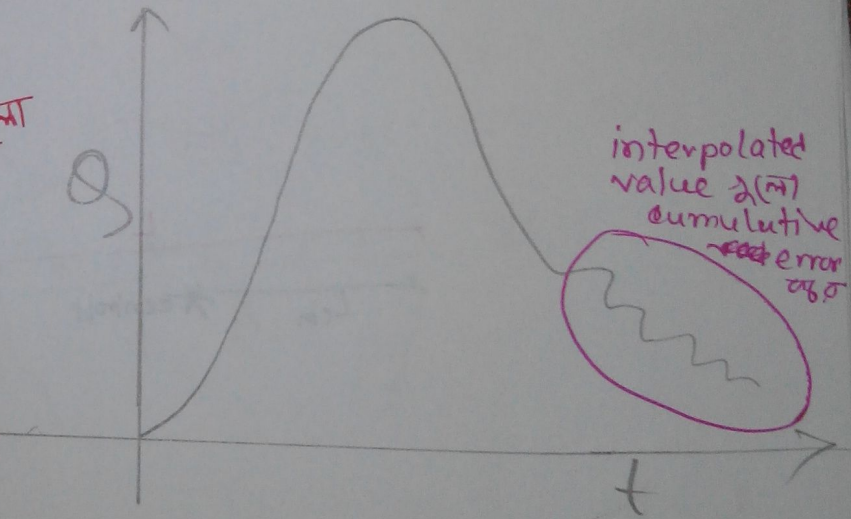
interpolated value (2hr shift) (दिए गए मानों के लिए)

T(hr)	0	2	4	6	8	10	12	14	16	18	20	22
UH req.	0	8	20	43	80	116	130	146	150	142	130	112
S ₁ curve (4hr shift) (दिए गए मानों के लिए)	0	8	20	51	100	161	230	307	380	449	510	561
S ₂ curve (2hr shift)	0	0	8	20	51	100	161	230	307	380	449	510
S ₁ - S ₂	0	8	12	31	49	61	69	77	73	69	61	51
UH req. ($\frac{T}{D} = \frac{1}{2}$ दिन) (दिए गए मानों के लिए)	0	16	24	62	98	122	138	154	146	138	122	102

24	26	28	30	32	34	36	38	40	42	44
90	70	52	38	27	20	15	10	5	2	0
600	634	652	669	679	689	694	699	699	701	699
561	600	631	652	669	679	689	694	699	699	701
39	31	21	17	10	10	5	5	0	2	-2
78	62	42	34	20	20	10	10	0	4	-4

ଅନୁକ୍ରମିତ fluctuate କରୁଛି, smooth କର
value ନିମ୍ନ 280,

needs adjustment
(exam ଏ value ଥିଲା
କେଉଁ ନା କେଉଁନା
କମେଇ, କମେଇ
value କେଉଁ
କେଉଁ graph
କିମ୍ପା ନାମକ)



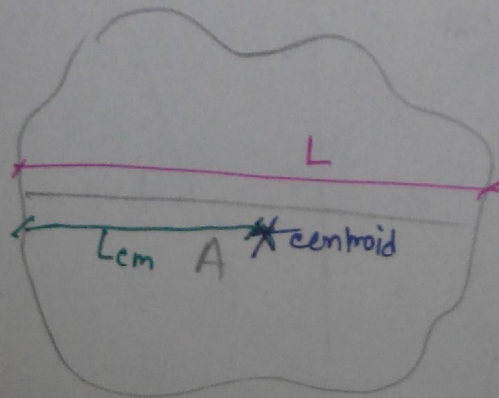
⊗ Synthetic unit hydrograph:

→ Artificially developed

→ empirical eqn basin characteristic

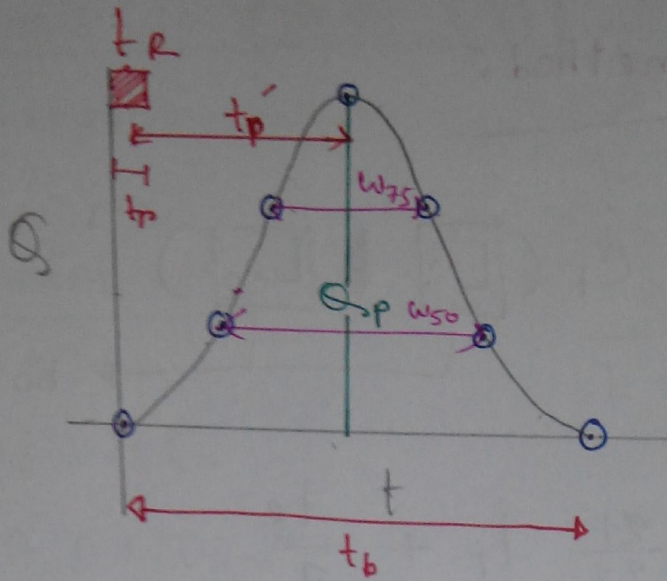
use to prepare

Defⁿ: Unit hydrograph derived using empirical eqn which relate unit hydrograph with basin characteristics.



1. area
2. furthest point to outlet (L)
3. centroid (*)
4. centroid → point to outlet (Lcm)

✱



t_R = standard unit hydrograph

t_p = non "

→ जलन shape is UH एतः पारः, But जलन unit hydrograph correctly point out करे करे, जो W_{75} , W_{50} point शून्य defⁿ

करे.

Example 6.14 (page 274)

meteorologically equⁿ = $C_p \cdot C_t$ constant,

Catchment A

$$L = 30 \text{ km}$$

$$L_{ca} = 15 \text{ km}$$

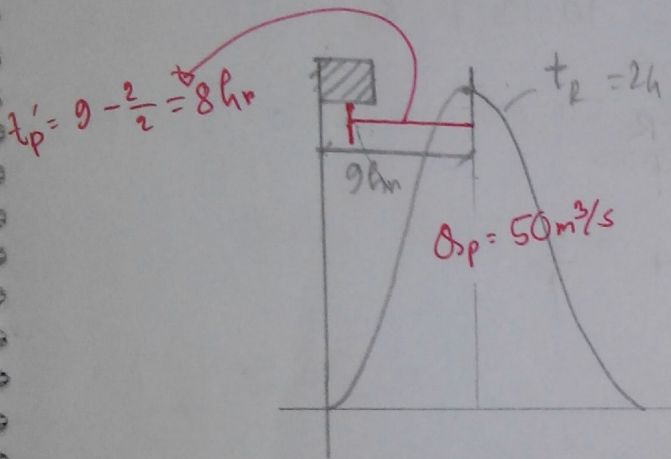
$$A = 250 \text{ km}^2$$

Catchment B

$$L = 45 \text{ km}$$

$$L_{ca} = 25 \text{ km}$$

$$A = 400 \text{ km}^2$$



$$t_p' = \frac{2L}{2L} t_p + \frac{2}{4}$$

$$\Rightarrow 8 = \frac{2L}{2L} t_p + \frac{2}{4}$$

$$\Rightarrow t_p = 7.86 \text{ hr}$$

$$\text{now, } t_p = C_t \left(30 \times 15 \right)^{0.3} = 7.86$$

$$\Rightarrow \boxed{C_t = 1.26} \rightarrow \text{same for A, B}$$

$$Q_p = \frac{2.78 C_p \cdot 250}{8} = 50$$

$$\Rightarrow \boxed{C_p = 0.58} \rightarrow \text{for A, B same}$$

Catchment B

$$t_p = C_t (L \cdot Lea)^{0.3} = 1.26 \times (45 \times 25)^{0.3} = 10.37 \text{ hr}$$

$$t_p' = \frac{21}{22} t_p + \frac{t_R}{4} \rightarrow \text{assume } t_R \text{ for B } 2 \text{ hr UH}$$

$$= \frac{21}{22} \times 10.37 + \frac{2}{4} = 10.4 \text{ hr}$$

$$Q_p = \frac{2.78 \times C_p \times A}{t_p'} = \frac{2.78 \times 0.58 \times 400}{10.4} = 62 \text{ m}^3/\text{s}$$

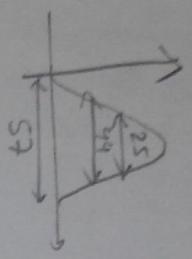
$$t_b = 72 + 3 t_p' = 72 + 3 \times 10.4 = 103.2 \text{ hr}$$

$$\text{and } t_b = 5 \left[t_p' + \frac{t_R}{2} \right] = 5 \times \left(10.4 + \frac{2}{2} \right) = 57 \text{ hr}$$

total more acceptable?? w75. w50 (x2)

$$W_{50} = \frac{5.87}{\left(\frac{Qp}{A}\right)^{1.08}} = \frac{5.87}{\left(\frac{62}{400}\right)^{1.08}} = 44 \text{ km}$$

$$W_{75} = \frac{W_{50}}{1.75} = \frac{44}{1.75} = 25 \text{ km}$$



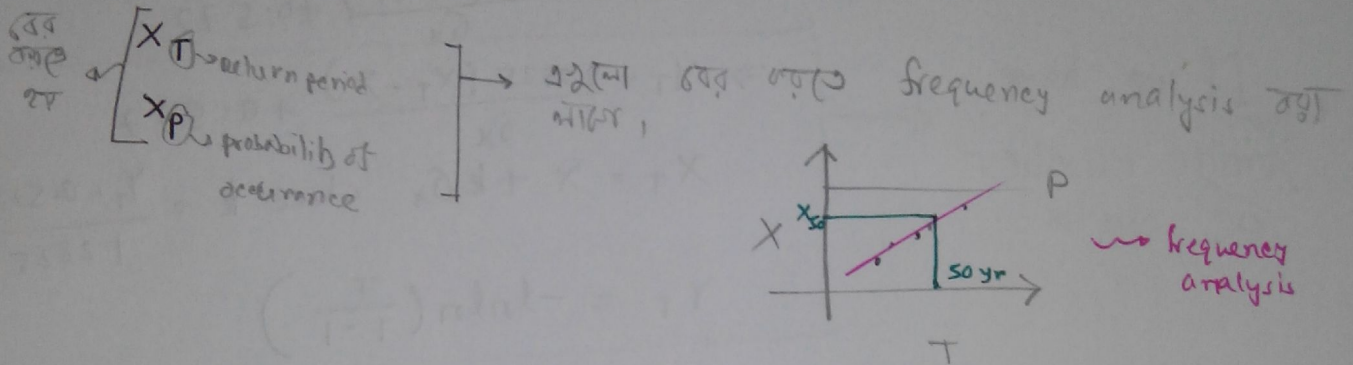
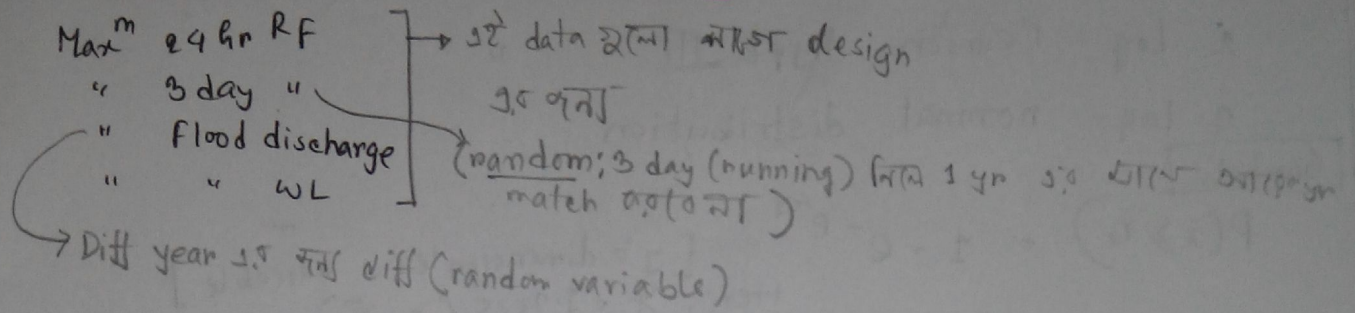
of 57 km (area) for $t_b = 57 \text{ km}$

⊗ Flood discharge estimate more very imp tool. But these is limitations of UH.

⊗ Limitation of using UH:

1. Assumptions of uniform distribution of PE both in [Time] and [space] are rarely met in [catchment of storm] at centre of [area] →, [57 km] uniform intensity [beginning + end] to [area] + [intensity]
2. Applicable to catchment area from 250 km² to 5000 km².

Statistical methods in hydrology:



frequency analysis can be carried out in 2 ways.

1. **empirical method** [20 yr 20 yr data (in descending order)]
 (Weibel's formula) $T = \frac{N+1}{m}$

20 yr data	X (highest)	rank (m)	$T = \frac{N+1}{m}$
	X_1	1	plotting position
	\vdots	2	
	\vdots	\vdots	
	X_{20} (lowest)	20	

disadv: we can't extrapolate it beyond and upto a limit.

20 yr data upto 2.5 yr limit only.

But important structure 20 yr return period use upto 100 yr.

2. Analytical method

$$X_T = \bar{X} + k \sigma_{n-1}$$

\bar{X} → avg
 k → frequency factor (normal distribution use table (value depend on T))
 σ_{n-1} → standard deviation for sample

1. Gumbel's extreme value distribution (widely used)
2. Log - Pearson type III
3. Log - normal distribution

Gumbel's:

$$P(x \geq x_0) = 1 - e^{-e^{-y}}$$

[y = dimensionless variable]

Here, $y = \frac{1.2825 (x - \bar{x})}{\sigma_x} + 0.577$

If we have
finite number
of data

$$\Rightarrow y_T = \frac{1.2825 (x_T - \bar{x})}{\sigma_x} + 0.577$$

$$x_T = \bar{x} + k \sigma_x \quad \text{where } k = \frac{y_T - 0.577}{1.2825}$$

$$y_T = -\ln \ln \left(\frac{T}{T-1} \right)$$