

WRE 303

Hydrology

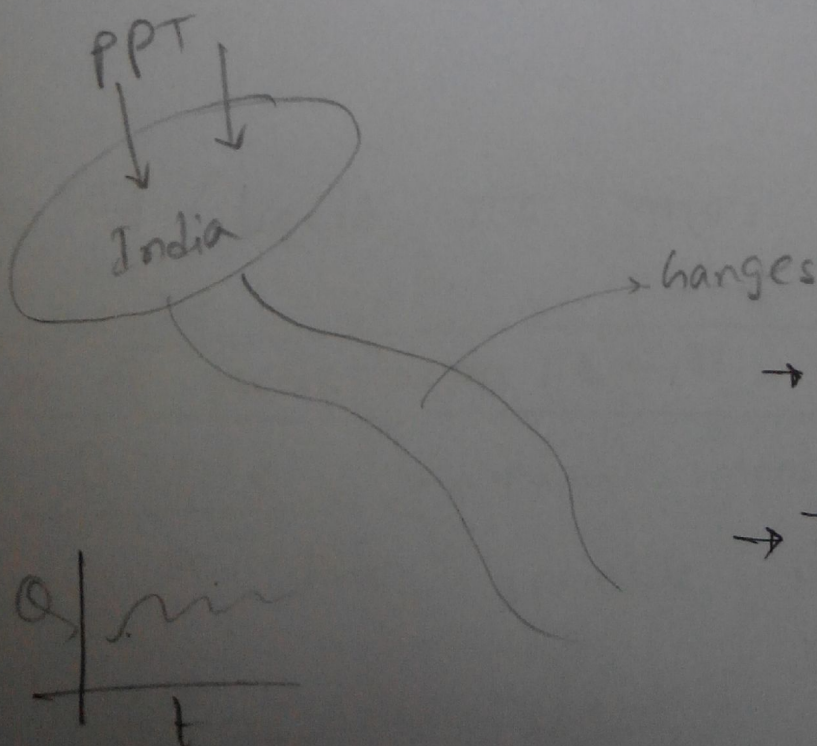
Nasrin mam

lec 1

27/02/17

6:57 AM (always) + 10:00 (after mid)

- precipitation
- Evaporation and evapotranspiration
- Infiltration
- Remote sensing + app of telemetry
- streamflow
- Hydrologic routing.



- computing the WL in d/s by updated info from u/s is called routing.
- To forecasting flood routing is needed.

\* Definition of hydrology

\* Branches " "

\* app " "

→ reservoir storage capacity

→ peak discharge and its vol<sup>m</sup> in design of irrigation

→

\* 1 CT sudden + 1 CT गाले वनते।

## Chapter 2

### Precipitation

→ all forms of moisture that reach the ground from atm.

ex: rain, snow.

#### formation:

\* vapour → nuclei + dust + vapour → drop (ପିଚ୍ଚି ଚନ୍ଦ୍ର)



ଏହା size ହେଲେ gravity ଯୋଗୁଁ ଚାଲି ଚାଲି ଖାଦ୍ୟ

\* buoyance (↑) + gravity (↓) + friction/drag (↑) → ଏହି force ଥିଲେ ଖାଦ୍ୟ ହୁଏ ।

\* solid / liquid ରୂପେ ଖାଦ୍ୟ  
↓  
କିରାଣି

#### form of precipitation (2.2 ଚର୍ଚ୍ଚ)

\* Drizzle . < 0.5 mm intensity 2 mm/hr

A fine sprinkle of numerous water droplets

of size less than 0.5 mm, and intensity less than 1 mm/hr.



# Weather system for precipitation (2.3)

## Type:

1. Cyclonic precipitation
2. Convective "
3. Orographic "

1. Cyclonic: low pressure area

নিম্নচাপ

Type: 1. tropical cyclon

2. extra tropical "

2. convective:

→ উদ উ main component  
warm বাতাস উঠে উঠে

3. Orographic:

সমুদ্র বাতাস উঠে উঠে  
বাষ্পীভবন হলে condensation  
হবে, precipitation হবে,  
এই বা বাতাসটি pass করতে  
গায়ে মেঘটিতে moisture থাকবে।

অসম্পন্ন উত্তর-দক্ষিণ → no moisture, no rain  
সবচেয়ে দক্ষিণ-পশ্চিম → বৃষ্টি হতে পারে কারণ  
from ocean

\* Cyclonic cyclone:

\* Tropical cyclone:  $\text{মহাবুড়াকি - ককটিকাকি}$  মাঝামাঝি  
zone (tropical zone)

middle  $\approx (0^\circ)$  wind speed  $\downarrow$  rainfall  $\uparrow$ .

\* Extra tropical cyclone:

\* Anticyclone:

\* Convective precipitation:

## \* Rain fall characteristics:

• size:  $\rightarrow 0.5 - 6 \text{ mm}$  dia.

• intensity, duration and depth.  
 (mm/hr) (height of water) (time)

• intensity =  $\frac{\text{mm}}{\text{hr}}$  (rate)

• duration = hr (time)

• depth = mm (height)

$$\text{Depth} = \text{intensity} \times \text{duration}$$

$$= \frac{\text{mm}}{\text{hr}} \times \text{hr}$$

$$= \text{mm}$$

\* intensity  $\uparrow$  duration  $\downarrow$  vice versa

• Intensity and area

• " and raindrop

### Measurement of precipitation (2.5)

• Measurement : climate station or instrument

for measure or, it's called rain gauge.

$$\text{rainfall vol} = \text{area} \times \text{ht}$$

$$\text{intensity} = \frac{\text{vol}}{\text{area}} \text{ (incremental time)}$$

## X Rain gauge setting:

→ must be level

→ must be set ~~at~~ near the ground.

→ 5.5m X 5.5m area to set gauge,

→ obstruction যাবে না থাকবে,

⊗ Short description: → Symon's gauge.

→ recording gauge  
(2 funnels)

→ recording gauge  
(const reading)  
→ non recording gauge  
(20% filter time  
→ reading)

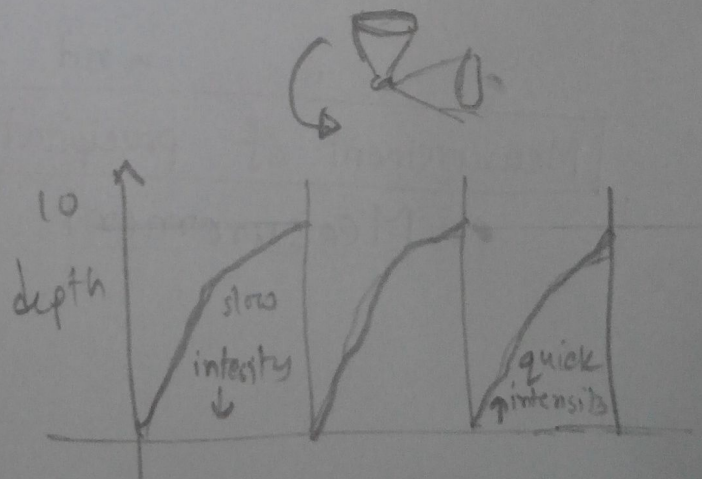


fig 5 2:6

\* Rain gauge network (2.6)

flat regions → Ideal : 4 station (50-900 km<sup>2</sup>)  
 acceptable : " " 900-3000 "

mountainous → Ideal : " " 100-250 km<sup>2</sup>  
 Acceptable : " " 250-1000 "

desert → arid and polar → " " 1500-10,000 km<sup>2</sup>

islands → " " 25 km<sup>2</sup>

\* Adequacy of rain gauge stations (2.6.2)

optimal no. of stations,  $N = \left(\frac{Q_v}{\epsilon}\right)^2$   
 Co-eff of variation,  $C_v = \frac{(1000 \sigma_{m-1})}{\bar{p}}$

$$\sigma_{m-1} = \sqrt{\left[ \frac{\sum_{i=1}^m (p_i - \bar{p})^2}{m-1} \right]}$$

↑ population
↑ sample  
(m-1)
(m)

$$\bar{p} = \frac{1}{m} \left( \sum_{i=1}^m p_i \right)$$

**Problem**

Type 1: Adequate station

Example:

The avg annual rainfalls in cm at 4 existing raingauge stations in a basin are 105, 79, 70 and 66. If the average depth of rainfall over the basin is to be estimated within 10% error. Determine the additional number of gauges needed.

Sol<sup>n</sup>:

$$\sigma_{41} = \sqrt{\frac{\sum_{i=1}^4 (P_i - \bar{P})^2}{4-1}}$$

Here,  $\bar{P} = \frac{105+79+70+66}{4} = 80$

$$\begin{aligned} \therefore \sigma_3 &= \sqrt{\frac{(105-80)^2 + (79-80)^2 + (70-80)^2 + (66-80)^2}{3}} \\ &= 17.53 \end{aligned}$$

$$C_v = \frac{100 \sigma_{m-1}}{\bar{p}} = \frac{17.53 \times 100}{80} = 21.91$$

no. of station,  $N = \left(\frac{C_v}{\epsilon}\right)^2$

$$= \left(\frac{21.91}{10}\right)^2$$

$$= 4.8$$

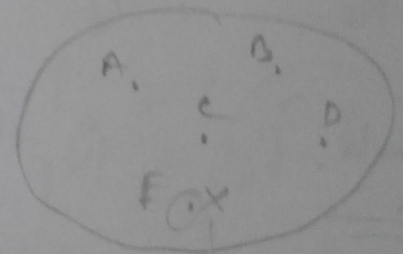
$\sigma^2$  এর ex  $\therefore$  additional = 1.  $\approx 5$  (০২)

example 2.1

### \* Preparation of data : (2.7)

→ যদি station এর মাঝে variation  $\pm 10\%$  এর মধ্যে থাকে then it's applicable.   
 এ stationগুলোর avg নিয়ে যদিই দিতে পারবে,

→ But  $\pm 10\%$  না 2%



2010 (00) if missing  
 2011 (100) (১০০)  
 missing data calc  
 ০.৫০ ২০০  
 for long record.

Formula 1: (when  $< 107\%$ )  
 Formula 2: (when  $> 107\%$ ) simple avg

$$P_x = \frac{N_x}{M} \left[ \frac{P_1}{N_1} + \frac{P_2}{N_2} + \dots + \frac{P_m}{N_m} \right]$$

3 sites  
 rainfall  
 avg annual rainfall

	E	A	B	C	D
July 1 - 2010	$\frac{P_E}{N_E}$	$\frac{P_A}{N_A}$	$\frac{P_B}{N_B}$	$\frac{P_C}{N_C}$	$\frac{P_D}{N_D}$
Sep 2 - 2010	?				

$\frac{P_E}{N_E} = \text{avg of other } \left( \frac{P_x}{N_x} \right)$

Formula 2:  $P_E = N_E \times \left[ \text{avg } \frac{P_x}{N_x} \right]$   
 (when  $> 107\%$ )

when diff  $< 107\%$  ,  $P_x = \frac{1}{M} [P_1 + P_2 + \dots + P_m]$   
 " "  $> 107\%$  ,  $P_x = \frac{N_x}{M} \left[ \frac{P_1}{N_1} + \frac{P_2}{N_2} + \dots + \frac{P_m}{N_m} \right]$

Problem Type 2: missing rainfall

Ex: Rain gauge station X didn't func for a part of a month during which a storm occurred. The storm produced rainfalls of  $P_a, P_b, P_c$  mm at three surrounding stations A, B, C respectively. The normal annual rainfalls at the stations X, A, B, C are respectively  $N_x, N_a, N_b$  and  $N_c$  mm. Estimate the missing storm rainfall at station X.

Ans: 75 mm

Sol<sup>n</sup>: 
$$P_x = \frac{1}{3} \left[ \frac{N_x}{N_a} P_a + \frac{N_x}{N_b} P_b + \frac{N_x}{N_c} P_c \right]$$

$$P_x = \frac{1}{3} \left[ \frac{770}{882} \times 89 + \frac{770}{936} \times 70 + \frac{770}{944} \times 96 \right]$$
$$= 75 \text{ mm}$$

example 2.2

(or)

Test for consistency of record (2.7.2)

data collect કરાવે બદલાવે તો climate & topography નો change થતો.

or વાંધે ભાંગે જાય જાનિ obstruct થવું

or fire ભાંગે તો પૂરું થઈને જાય તો

તબક્કા આવે into તે modify કરે એવું

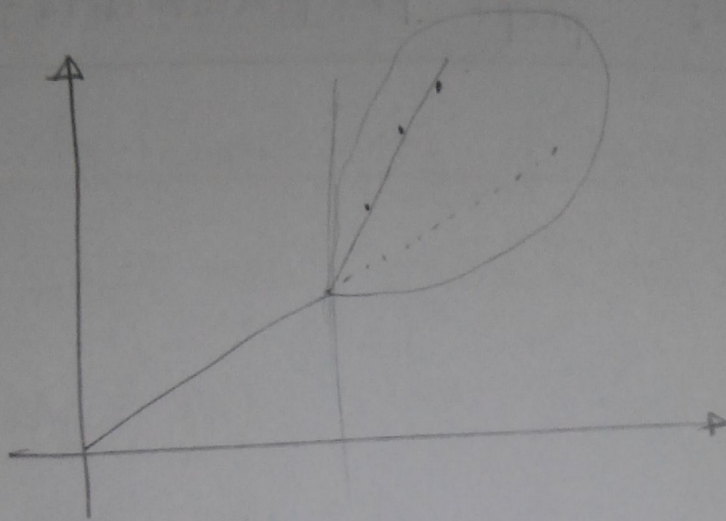
regime નો તિરે આવી શકે inconsistency થવું.

Causes:

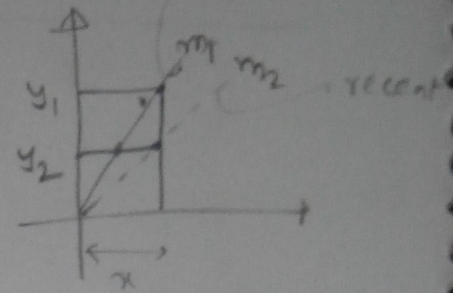
- shifting of a rain gauge to a new station
- the neighborhoods change
- ecosystem change due to forest fire and etc.
- record error.



accumulated  
precipitation  
station X,  
 $\leq P_x$



avg acc. p of  
other



$$y_1 = m_1 x$$

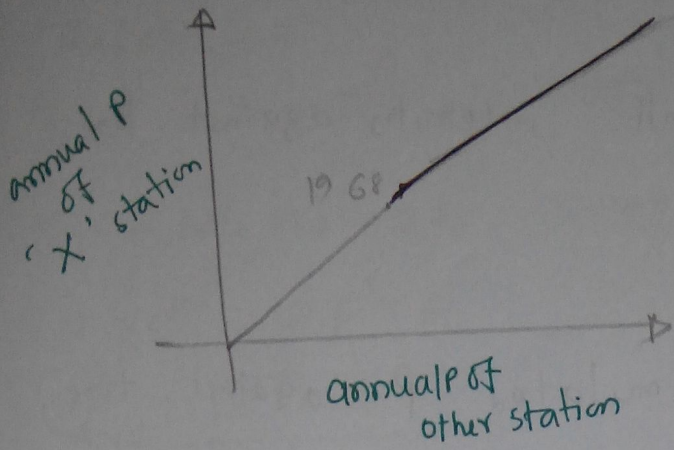
$$y_2 = m_2 x$$

$$y_2 = m_2 x \frac{y_1}{m_1}$$

\* **Problem:** **Problem** Type 36 consistency of data

yr	Annual rainfall M	Annual Others avg	Cumulative	Annual others cumulative
1979	612	588	612	588
1978	426	410	1038	998
1977	825	787	1863	1785
1976	685	653	2548	2438
1975	356	377	2904	2815
19874	568	570	3472	3385
1973	438	390	3910	3775
1972	386	400	4296	4175
1971	497	490	4793	4665
1970	635	590	5428	5255

of consistency  
In In 51st  
new line draw  
upto 1970.



series 1  
series 2

graph ↘ cumulative  
245 268

$$m_2 = 1.0295$$

$$m_1 = 0.8779$$

but

$$y_2 = m_2 \times \frac{y_1}{m_1}$$

এটি  
cumulative  
না

example 2.3 (৩৫)

## Presentation of Rainfall data (2.8)

### Hyetograph (2.8.2)

→ plot of rainfall intensity against time.

### Mass curve (2.8.1)

→ plot of accumulated  $P$  against time.

### Point rainfall / station rainfall (2.8.3)

→ It refers to the rainfall data of a station.

## Mean precipitation over an area; (2.9)

Thesis

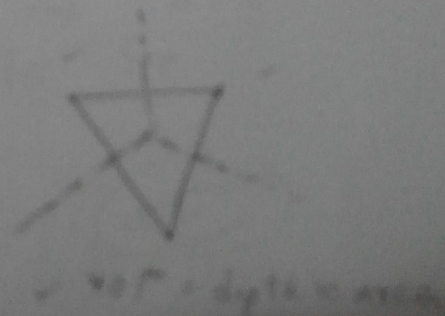
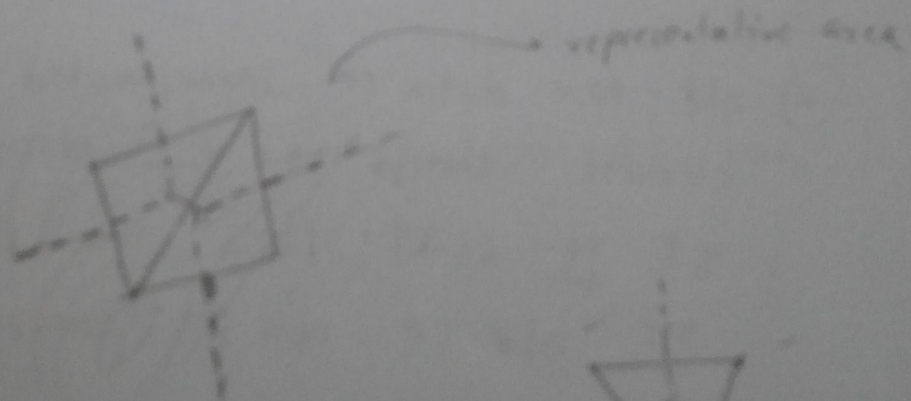
① SA notation add  $\sigma^2$  triangle form  $\sigma^2$ ,

② middle  $\rightarrow$   $2\sigma$   $\sigma^2$

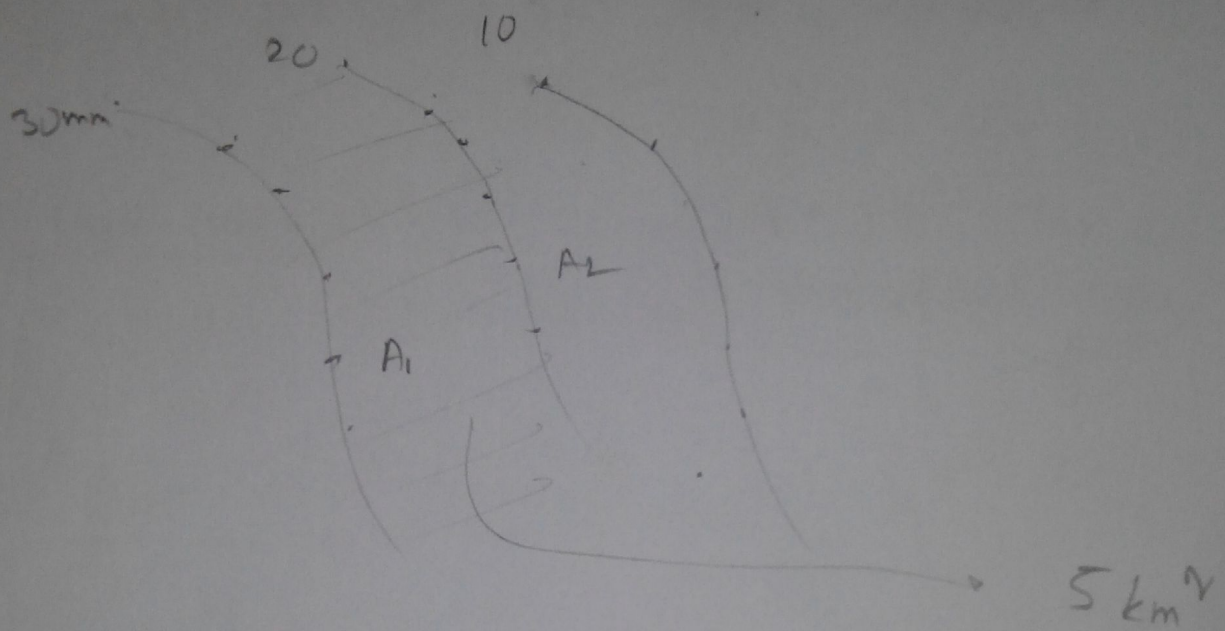
③ var,  $SP \times A$

④ avg 

$SP \times A$
$SA$



3. Isohyetal : same height or rainfall



$$vol^m = \frac{5 \times 25}{km^2 (mm)}$$

$$avg = \frac{vol^m}{Area}$$

(\*) old - new data को incorporated करके नया, climate change को भी ध्यान देना है।  
 नया new data (नया) निम्न time short है, वही, वही  
 वही old को new को convert करे।

Problem

Type 4: Average depth (Thiessen + A.M.)  
Isohyet 13/6/04

Problem: (Thiessen polygon)

ex: 1  
Thiessen polygon

Isohyet (mm)	Area (km <sup>2</sup> )	Vol <sup>m</sup> (mm <sup>3</sup> )
75	10	10 x 75 = 45 x 10 <sup>12</sup>
85	85	7225 x 10 <sup>12</sup>
95	113	10735 x 10 <sup>12</sup>
105	98	10290 x 10 <sup>12</sup>
115	136	15640 x 10 <sup>12</sup>
125	67	8375 x 10 <sup>12</sup>

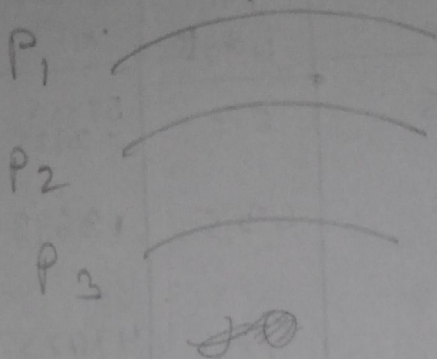
$\Sigma A = 509 \text{ km}^2$   
 $= 509 \times 10^{12} \text{ mm}^2$

$\Sigma 53015 \text{ } \times 10^{12}$

$\therefore \text{average} = \frac{53015 \times 10^{12}}{509 \times 10^{12}} \text{ mm}$   
 $= 104.155 \text{ mm}$   
 $= 104.155 \text{ mm}$

example  $\frac{2.5 + 2.6}{2}$

ଆମା ମିତର math



କି ଆମା storm ଏବଂ ଏକ centre ଥାଏ (maximum rainfall) ଏବଂ ଯେତେ ଦୂର ଯିବ, ସେତେ କମ୍ ରାସୁଣ ଥାଏ.

ex 2 :  
Isohyte

fx: [Isohyte]

Isohyt (mm)	57.50	45	40	35	30	25	20	15	10
Enclaved area (km <sup>2</sup> )	55	1300	2060	2700	2955	3600	4030	4800	6000

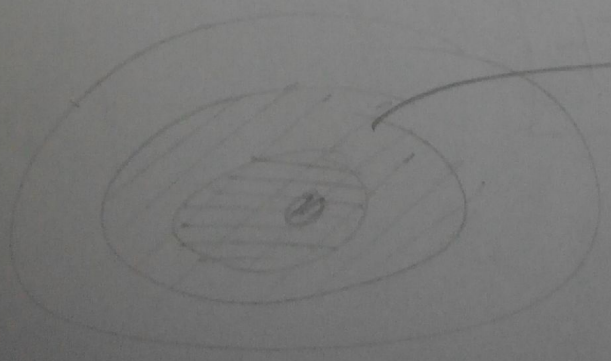
→ cumulative area

The data

ଆମା ମିତର 2ଟି isohyte ଯାଏ

Startins: Isohyte 57.5 mm, A = 55 km<sup>2</sup> (storm centre)

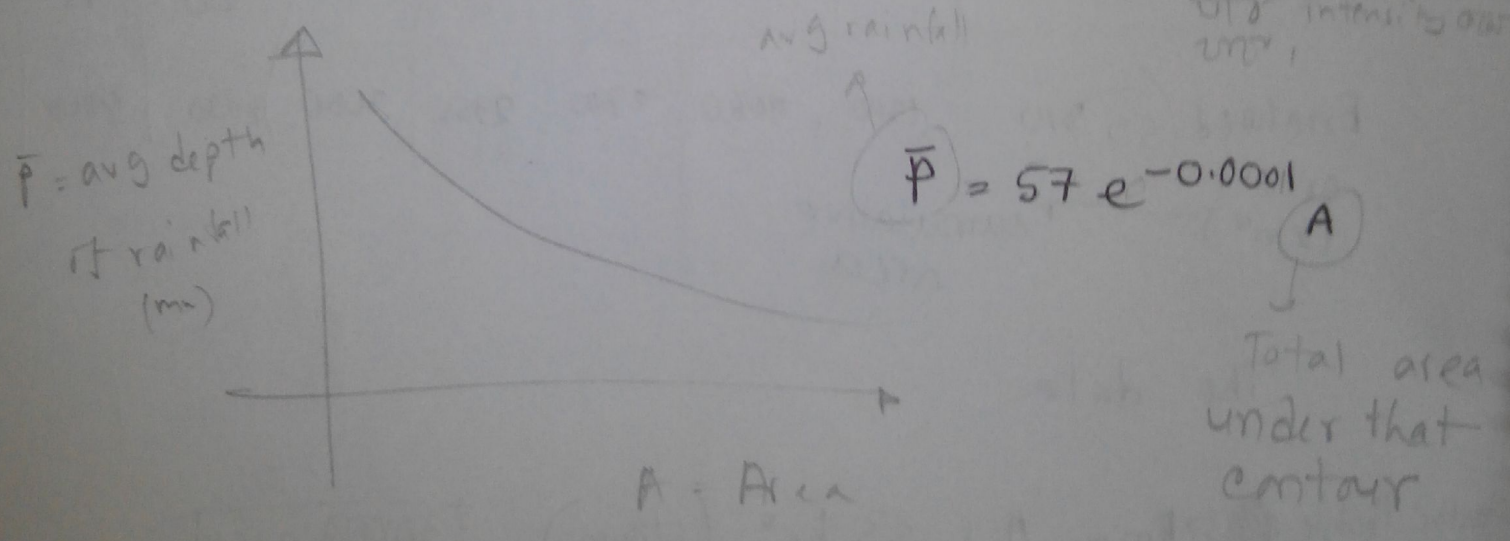
ଆମା area (କେମି ମିତର)  
ଆମା area (କେମି ମିତର)  
cumulative area (କେମି ମିତର)



area A କି ବାଡ଼ି contour graph ଏବଂ plot କରୁ ଏବଂ କେନ୍ଦ୍ର ବିନ୍ଦୁ ଥିବା କେନ୍ଦ୍ର ଉପରେ instrument ଆମା (planimeter) ଆମା କରୁ ଏବଂ area ମିତର ମିତର.

Sol<sup>n</sup>:

① Isohyte	② Enclused area	③ Increment area	④ avg isohyte	⑤ incremental vol <sup>m</sup>	⑥ Total vol <sup>m</sup>	⑦ avg depth = $\frac{\text{Total vol}^m}{\text{Total area}}$
57	55	55	57	$57 \times 55 = 3135$	3135	$\frac{3135}{55} = 57$
50	310	255	53.5	13643	16778	$\frac{16778}{310} = 54$
45	1300	990	47.5	47025	63802	49



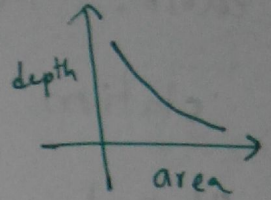
example 2.7

Depth-area duration relationship (2.10)

Depth-area relation: (2.10.1)

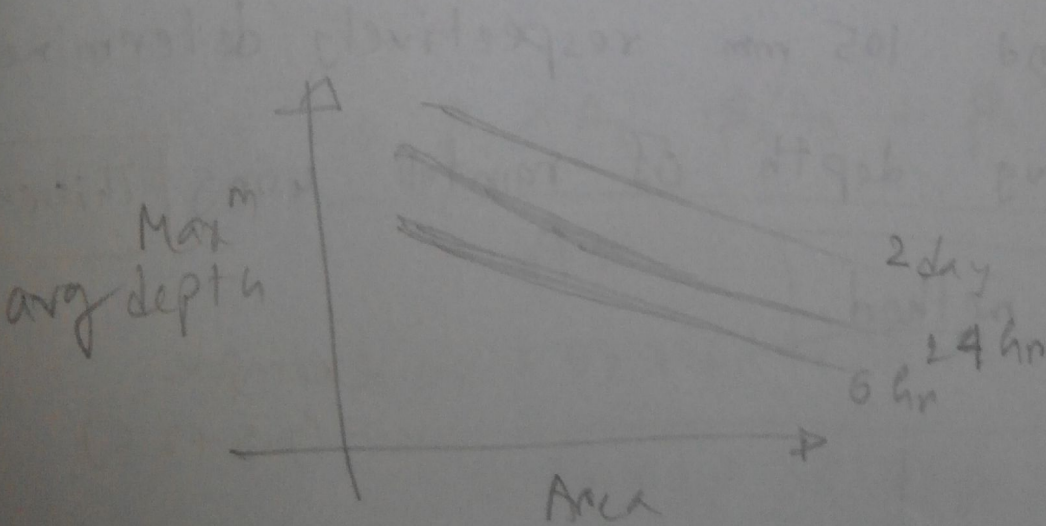
$$P = P_0 \exp(-kA^n) = P_0 \cdot e^{-kA^n}$$

Q. 5000 km<sup>2</sup> under a P? = graph/equ<sup>n</sup>



Q. Total future max<sup>m</sup> rainfall in N area over 100 yrs max<sup>m</sup> precaution steps

Max<sup>m</sup> depth area duration (2.10.2)



DAD curve.

ex 3: Thiessen polygon

Ex: A semicircle of diameter 20 km with

an equilateral triangle of side 20 km below its

dia is a plot close approximation to a river

basin. The position co-ordinates of 5 rain gauges

station A, B, C, D, E located within the basin

is. i. t. a co-ordinate axes system whose x-axis

and origin are coincident with dia and

centre are  $(5, 5), (-5, 5), (-5, -5), (5, -5)$

and  $(0, 0)$  km respectively. If the rainfall

measured at these rain gauge are 85, 92, 72

80 and 105 mm respectively determine

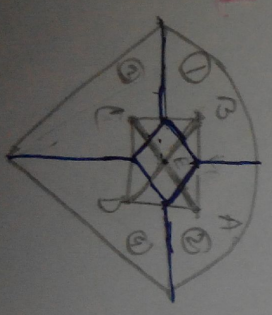
the avg depth of rainfall using Thiessen

- ① network of triangles
- ② perpendicular bisector
- ③ polygons and the boundary around each station

Thiessen polygon method.

example

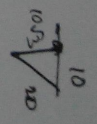
$$(3 \cdot 5 + 2 \cdot 6)$$



$$\underline{\underline{2316004}}$$

$$\text{Total area} = \frac{\pi \times 10^4}{2} + \frac{1}{2} \times 20 \times 10\sqrt{3}$$

$$= 950.285 \text{ km}^2$$



$A_1, A_2$  area  $\Rightarrow$

$$\frac{\pi \times 10^4}{4} - 12.5$$

$$= 66.04 \text{ km}^2$$

$$\frac{1}{2} \times 5 \times 5 = 12.5$$

$A_3, A_4$  area  $\Rightarrow$

$$\frac{1}{4} \times 20 \times 10\sqrt{3} - 12.5$$

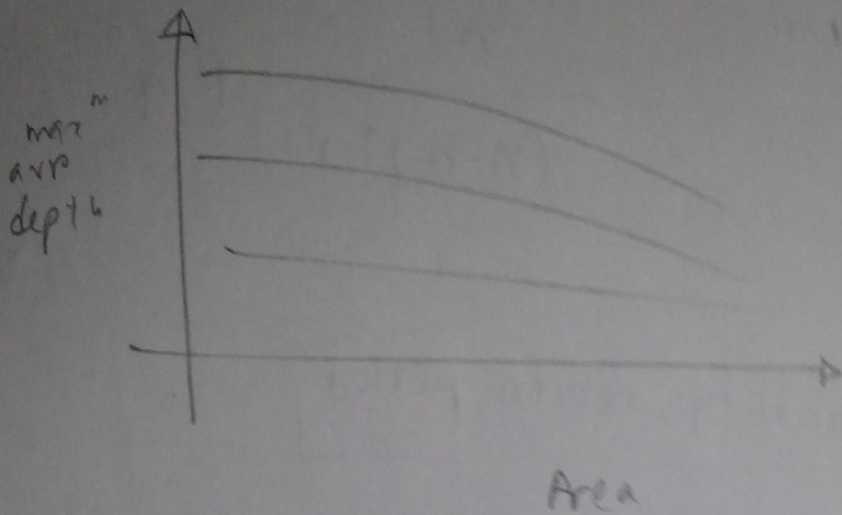
$$= 74.1025 \text{ km}^2$$

$$A_5 = 12.5 \times 4 = 50 \text{ km}^2$$

$$a_m = \frac{P_1 \times A_1 + P_2 \times A_2 + P_3 \times A_3 + P_4 \times A_4}{A_5}$$

$$= \frac{85 \times 66.04 + 92 \times 66.04 + 77 \times 74.1025 + 80 \times 74.1025 + 50 \times 105}{66.04 \times 2 + 74.1025 \times 2 + 50}$$

$$= 86.51 \text{ mm}$$



example 2.8 (Not clear)

frequency of point rainfall: (2.11)

- T (return period)  $\rightarrow Q_{1988} \rightarrow$  large
- T (return period)  $\leftarrow Q_{1991} \rightarrow$  small
- Rain  $_{1991} \rightarrow$  small

high value  $\rightarrow$  exceed  
probability  
small

$$P = \frac{1}{T}$$
 probability  
 flood आसारा

$$T = \frac{1}{P}$$
 १ वर्षा आसारा  
 आसारे

T = return period.

1988 ए.व. flood 50mm  
 १ वर्षा आसारा आसारे  
 १० return period = 50

$p, n, r$  →  $n$  yr  
 →  $n$  years of data available probability

$$P_{n,n} = {}^n C_n p^r q^{n-r}$$

$$= \frac{n!}{(n-r)! r!} p^r q^{n-r}$$

→  $(1-p) = (1 - \frac{4}{7}) = \frac{3}{7}$

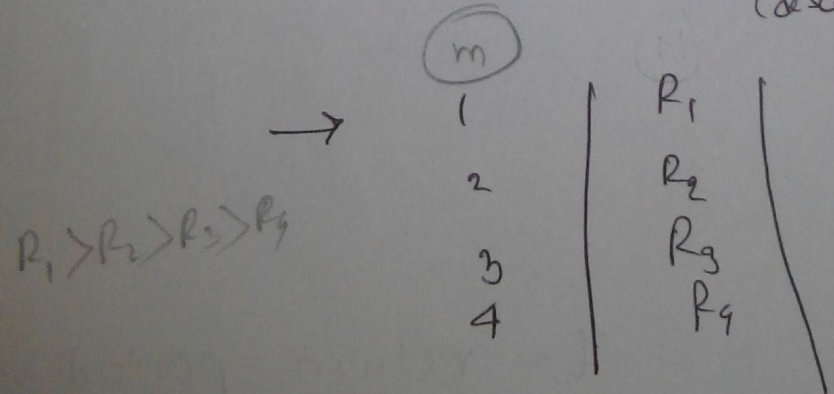
→  $\frac{1}{T}$

→  $n$  years of data available probability

**Problem** Type 5: Probability, return period

⊗ 30 yr of annual rainfall data is given (or) 50 (or) 70 yr of annual rainfall data, find the return period

- Step:
- collect  $R$
  - sort in decreasing order (descending)



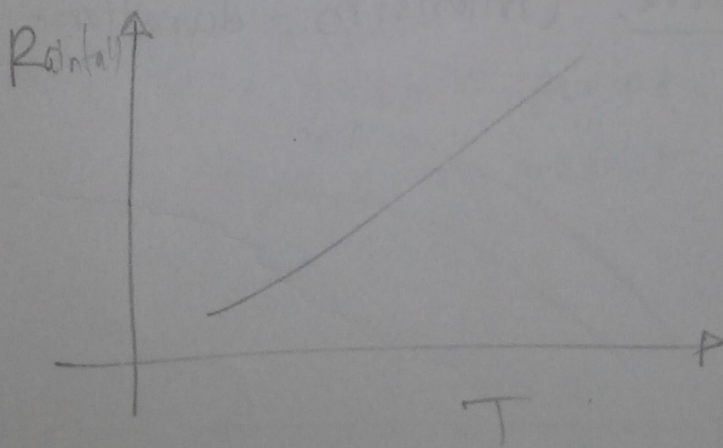
wei bull  $\rightarrow p = \frac{m}{N+1}$   $\rightarrow$  No. of data

highest,  $p = \frac{1}{1+30}$   $T = \frac{1}{p}$

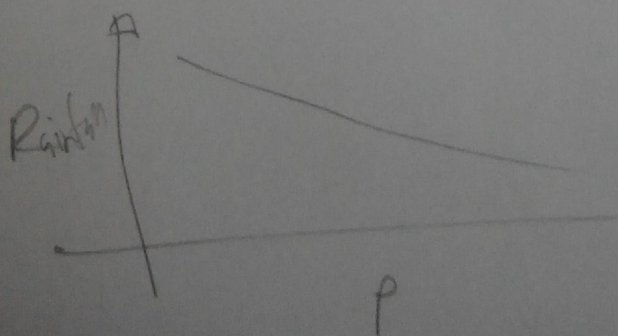
2nd,  $p = \frac{2}{1+30}$

3rd ...  $p = \frac{3}{1+30}$

∴  
30th ...  $p = \frac{30}{31}$  ( ~~...~~ 30th p (rank) )



example  
2.9 +  
2.10 +



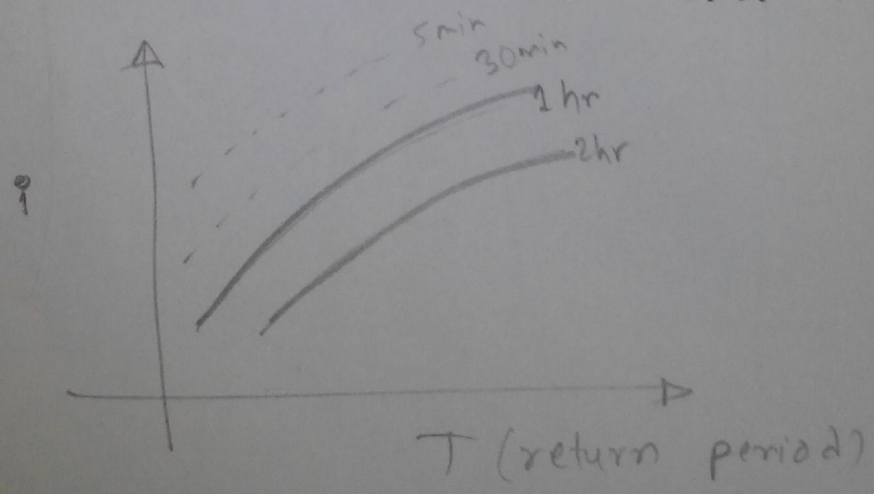
→ data plotting

1980 → 1990,  $T = 12 (11 + 1)$   
 1988 yr  $T = 100$  yr  
 1985 - 1988,  $T = 4 + 1 = 5$   
 $T = 100$  yr

→ sampling data correct 270  
 → 100 yr yr  $P$   $100 \text{ yr} / 99$

\* I-D-F curve: (Intensity - duration - frequency curve)

Maximum I-D-F relationship (2.12)

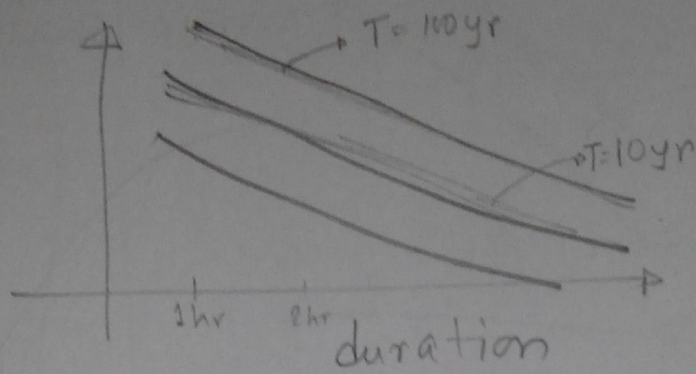


$$\text{intensity} = \frac{\text{Rainfall}}{\text{duration (1hr)}}$$

PT IT

1 yr rain

time frame



return period

exam → qualitative curve draw

Duration ↑ i ↓  
T ↑ i ↑

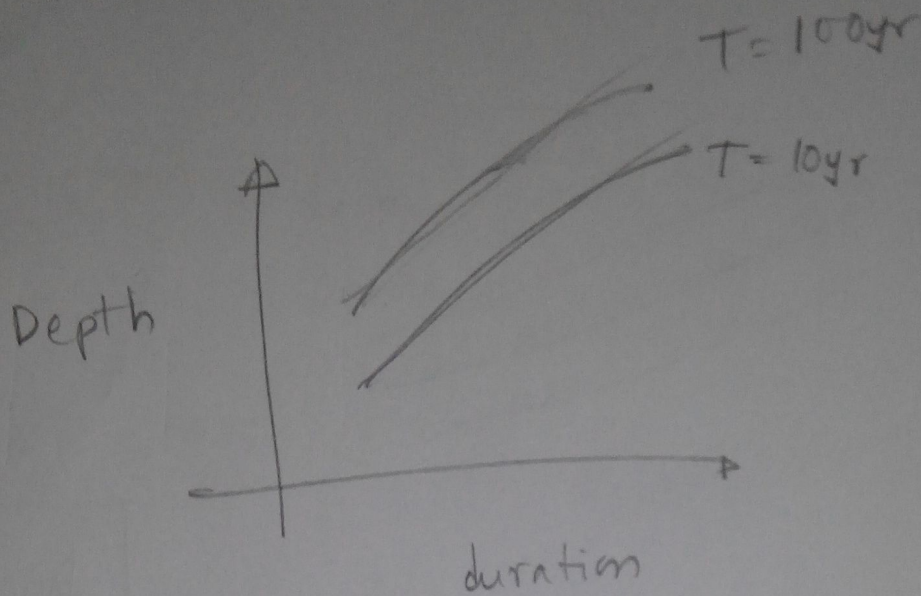
$$i = \frac{k t^n}{(D+a)^n}$$

→ equn (2.15)

D ↑ i ↓  
T ↑ i ↑

D = duration  
t = return period.  
k, n, a, n = const

$i \propto t$   
 $i \propto \frac{1}{D}$



duration  $\uparrow$  depth  $\uparrow$   
(cumulative)  
 $\delta \propto D$

Probable max<sup>m</sup> precipitation (PMP) (2.13)

$$PMP = \bar{P} + k \sigma$$

short note

$\bar{P}$  = mean of annual max<sup>m</sup> rainfall

$\sigma$  = st. deviation

$k$  = frequency factor

**Problem:**

**Type 6: PMP**

univariate approach  
 that has a max intensity = ?

	1 hr (mm)	2nd hr (mm)	3rd hr (mm)	max 1hr	max 2nd hr	3rd hr
Day 1	5	4	6	6	10	
Day 2	3	3	4	4	7	
Day 3	4	2	5	5	7	
Day 4	3	2	2	3	4	

$\overline{\text{max}^m P} = 6$   
 $\overline{\text{max}^m P} = 10$

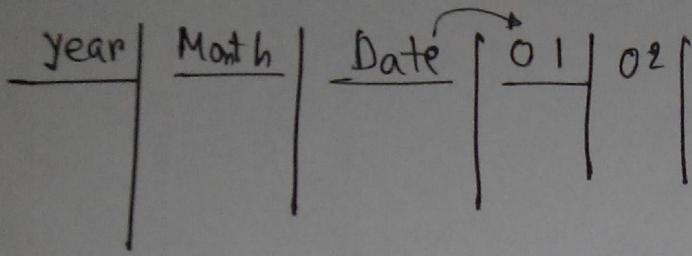
4 দিনের মধ্যে 1hr এর জন্য  $\text{max}^m P = 6 \text{ mm}$   
 $\therefore \text{intensity} = \frac{6}{2} \text{ mm/hr} = \boxed{3 \text{ mm/hr}}$

4 দিনের মধ্যে 2hr এর জন্য  $\text{max}^m P = 10 \text{ mm}$   
 $\therefore \text{intensity} = \frac{10}{2} \text{ mm/hr} = \boxed{5 \text{ mm/hr}}$

exercise: moving avg -

**Type 7: Moving average**

example: 2.9



Assignment 6

compute 5 day moving avg at max<sup>m</sup> intensity for 5 day duration.

[ 5th day ke liye month ke 5th day ke liye ]

total depth ke liye

then intensity.



$$E_L = C (e_w - e_a)$$

2. Air & water temp : temp  $\uparrow$  water hold capacity  $\uparrow$   
 $E \uparrow$

3. Wind :  $\uparrow \uparrow$  (as water moisture goes up)

4. Atm pressure :  $\frac{5}{2} \uparrow$   $E \uparrow$

5. Quality of water :

impurities  $\uparrow$  temp change  $\uparrow$

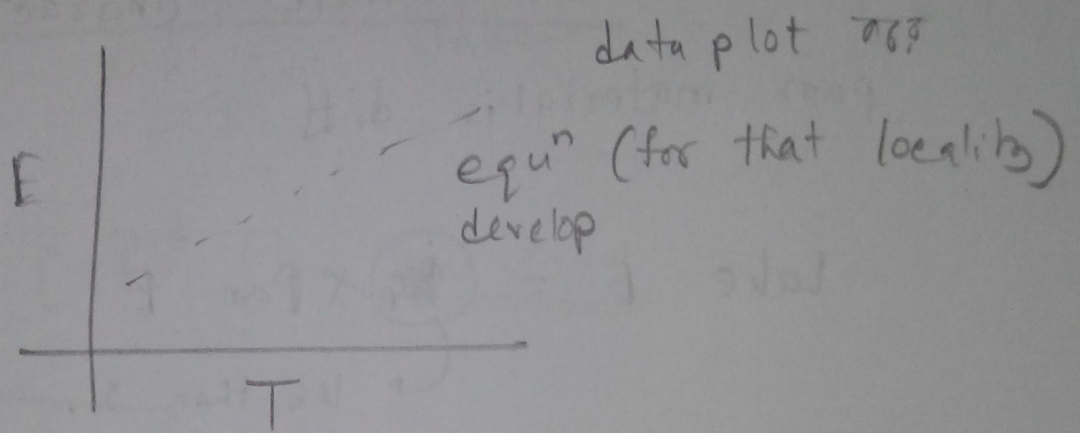
6. Size of water body :  $A \uparrow$   $E \uparrow$

7. Heat storage in water body :

depth  $\uparrow$  storage capacity  $\uparrow$

\* Measurement %

1. evaporimeter ~~data~~ data
2. Empirical equ<sup>n</sup>
3. Analytical methods.



Analytical is more difficult to do variable change,  
 but empirical easier.

\* Evaporimeter:

- (a) class A ✓
  - (b) ISI
  - (c) Colorado sunken pan
  - (d) US geological survey floating pan
- ସମସ୍ତ  
 ଉପର  
 ଖର୍ଚ୍ଚ  
 କରାଯାଏ  
 କିନ୍ତୁ  
 ସର୍ବୋତ୍ତମ

Evaporimeter:

\* E from lake < E from pan, why?

→ heat storing capacity diff.

→ height of rim

→ heat transfer characteristics of pan material is diff.

Lake E =  $C_p$  \* Pan E

→ less than 1.

(X) (X) (X)

\* pan co-eff =  $C_p$

exam  $\frac{1}{2}$  or  $\frac{1}{3}$   
(0.7 - 0.8)

\* Empirical Evaporation eqn:

$E = k f(u) [e_w - e_a]$

Meyer's :  $E = K_m (e_w - e_a) \left(1 + \frac{u_g}{16}\right)$

$u_g$  = monthly mean vel (km/hr) at 9m above the ground

$u_g =$  <sup>monthly mean</sup> vel (km/hr) 9m above from ground

$K_m = 0.36$  for large deep water [E ↓]  
 $0.5$  " shallow " " [E P]

Example [3.2]

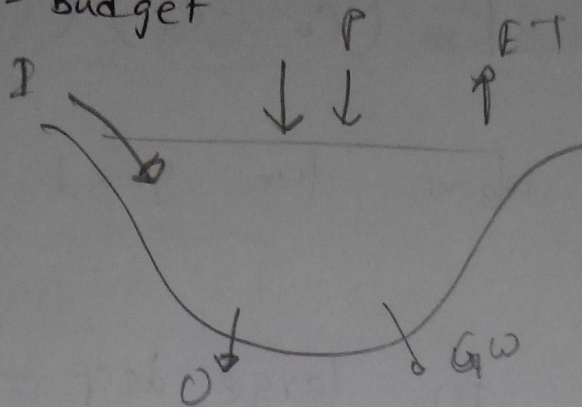
ex 3.1

A. Measurement

B. analytical method

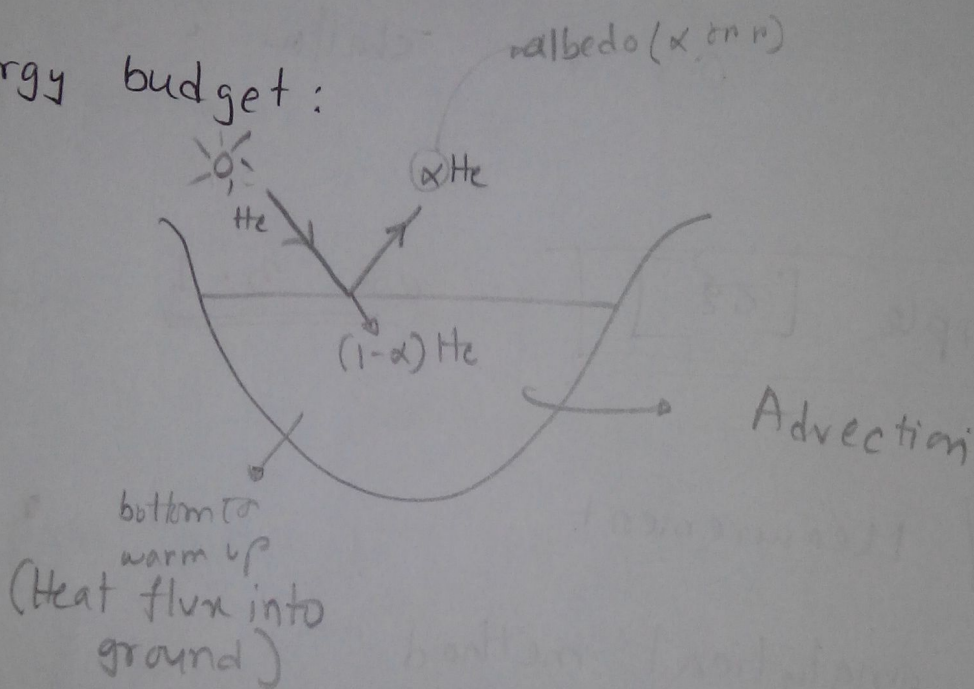
1. water-budget method
2. energy-balance
3. Mass-transfer method

1. Water-budget



- P (+)
- I (+)
- O (-)
- ET (-)
- Gw (-)

2. Energy budget:



$$H_n = (1 - \alpha) H_s$$

black body radiation

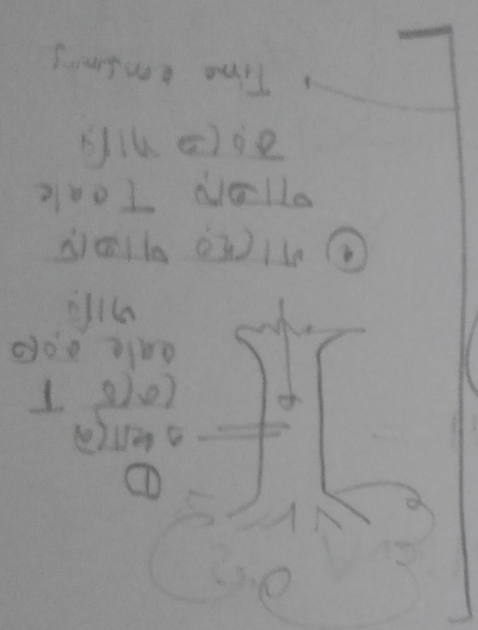
$\left[ \begin{array}{l} \text{যদি } \text{temp} > 0^\circ\text{C} \text{ হয়} \\ \text{তবে radiation emit} \\ \text{হবে, it's also loss} \end{array} \right]$

$$H_n = H_a + H_e + H_g + H_s + H_r$$

analytical, exp.  $\downarrow$   
 wa variable (MVA)  
 time consuming

$$PET = \frac{A h_n + E a r}{A + r}$$

\* Penman's eqn: (Theory + empirical) mixed



E  $\rightarrow$  water body + soil mass (rate)  
 T  $\rightarrow$  heat rate

\* Evaporation

$\rightarrow$  surface area of pan depth of water  
 $\rightarrow$  oil film (chemical film)  
 $\rightarrow$  Mechanical cover

\* Methods of reduce evaporation:

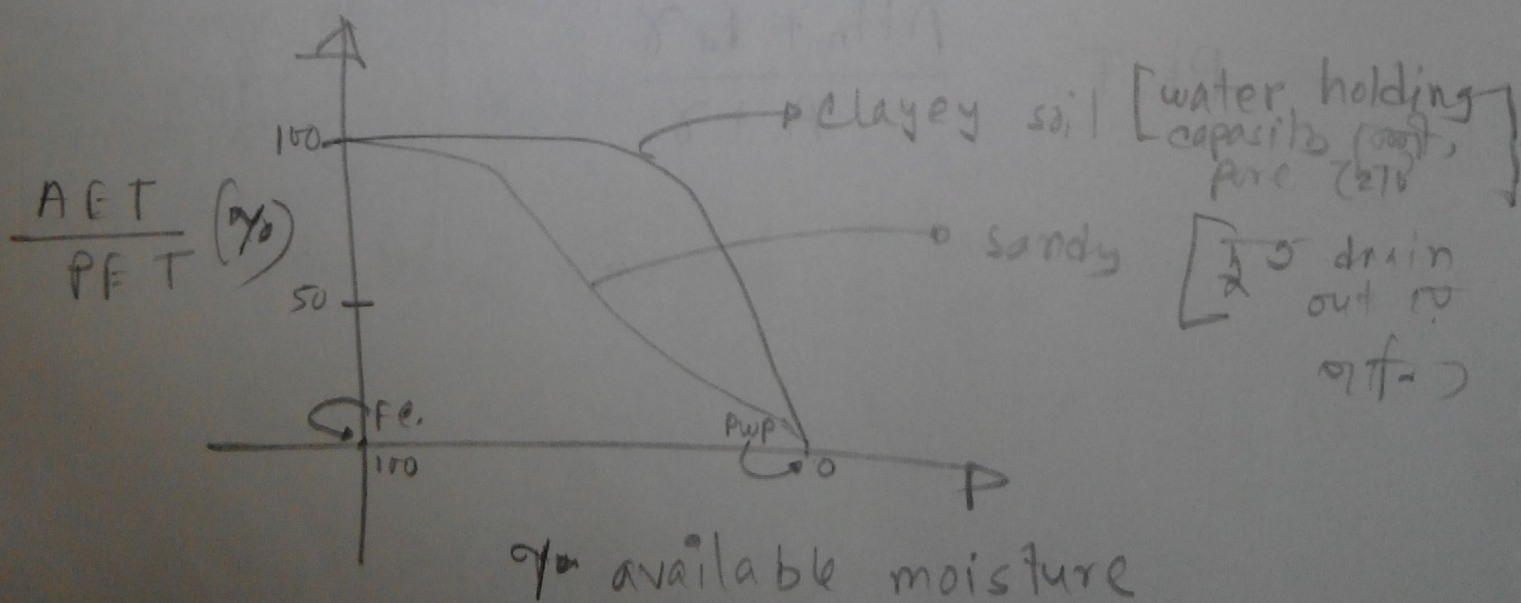
Defn:

exam

\*\*\*

1. Potential ET [max<sup>m</sup> অবস্থায় যত ET হতে পারে] unlimited energy temp
2. Actual ET [Existing অবস্থায় এ ET]
3. Field capacity [max<sup>m</sup> এ moisture hold হতে পারে]
4. Permanent wilting point [dry হলে এত এত সঞ্চে moisture কখনো remove করা যায় না, সঞ্চে]
5. Available water

= Field capacity - Permanent wilting point.



FC = 100% m/c  
PWP = 0% m/c

exam 2      60      — 90  
 rearrange      ↓      ↓  
 sat (T)      0      100  
 9776

CT → 9th week (Precipitation)

① Water budget eqn

② FT experimentally

③ Penman eqn for FT

$$E_p = \frac{A + T_p}{A + B}$$

A = slope of saturation vapor pressure vs

temp curve

B = wind velocity parameter

C = wind velocity parameter

D = psychrometric const

## lec 7

ex 2.11 [3rd edition]

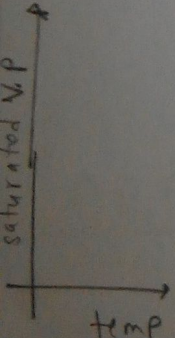
• humidity  $\uparrow$  sunlight  $\downarrow$   $\overline{z(m)}$  ET  $\downarrow$ .

⊗ water budget equ<sup>n</sup>.

⊗ ET experimentally tough.  $\therefore$  equ<sup>n</sup> for ET calc  $\therefore$  ,

⊗ Penman's equ<sup>n</sup> for ET:

$$PET = \frac{A H_n + E_a \gamma}{A + \gamma}$$

 A = slope of saturation vapour pressure vs temp at curve [KPa/°C]

$H_n$  = net radiation [ $H_n = (1 - \alpha) b_0$ ]

$E_a$  = wind velocity parameter

$\gamma$  = psychrometric const. = 0.49 mm mercury/°C

1.  $H_n = H_a (1 - \rho) \left( a + b \frac{n}{N} \right) - \sigma T_a^4 \left( 0.56 - 0.092 \sqrt{e_a} \right) \left( 6.1 + 0.9 \frac{n}{N} \right)$

$H_a$ : actual/incident radiation  
 $\rho$ : albedo  
 $\frac{n}{N}$ : max<sup>m</sup> possible sunlight  
 $\sigma T_a^4$ :  $2.01 \times 10^{-9}$  mm/day  
 $e_a$ : actual vap pressure

2.  $E_a = 0.35 \left( 1 + \frac{4e}{160} \right) (e_w - e_a)$

$E_a$ : km/day

relative humidity  $\times e_w = e_a$

3.  $e_w = 4.584 e^{\left( \frac{17.27 \times T}{237.3 + T} \right)}$

mm of Hg

4.  $A = \frac{4098 \left[ 0.6108 e^{\frac{17.27 T}{T + 237.8}} \right]}{(T + 237.3)^{0.5}} \times \frac{760 \text{ mmHg}}{101.325 \text{ kPa}}$

$A$ :  $^{\circ}C$



\* Problem:

Calc. PET in June by (a) Penman  
(b) Thornthwaite

lat = 28°N

elv = 230 m above MSL

Mean obs. sunshine = 9h/day.

Month	Temp (°C)	Relative humidity	wind vel at 2m above RL (km/h)
Jun	33.5	52	10.0

Soln:

$$A = \frac{4098 \left[ 0.6108 e^{\frac{17.27 T}{T+237.4}} \right]}{(T+237.3)^2} \times \frac{700}{101.325}$$

$$= 2.16$$

saturated vap. pressure,  $e_w = 4.584 e^{\frac{17.27 \times 33.5}{237.3 + 33.5}}$

$$= 38.82 \text{ mm of Hg.}$$

28°N, June 5th, Table  $\tau_{ET0}$ , interpolation.  
 $H_a = 16.36 \text{ mm/day}$

$$N = 13.94 \text{ hr/day} \quad [\text{from Table}]$$

$$n = 9 \text{ hr/day}$$

$$e_a = \text{relative humidity (RH)} \times e_w$$

$$= 0.52 \times 38.82$$

$$= 20.177 \text{ mm of Hg}$$

$$\gamma = 0.49 \rightarrow \text{স্থলস্থ}$$

$$a = 0.29 \cos \phi \quad \text{latitude}$$

$$= 0.29 \cos 28$$

$$= 0.256$$

$$b = 0.52 \rightarrow \text{স্থলস্থ}$$

$$\text{albedo, } r = 0.05 \quad , \quad \sigma = 2.0 \times 10^{-9}$$

[For waterbody]

$$T_a = \text{in Kelvin}$$

$$H_a = 16.36 \text{ [table 3.4]}$$

$$H_n = 16.36 \times (1 - 0.05) \left( 0.256 + 0.52 \times \frac{9}{13.94} \right) \left[ \frac{2.0 \times 10^{-2}}{(33.5 + 273)^2} \right] \times (0.56 - 0.092 \times \sqrt{20.177}) \times (0.1 + 0.9 \times \frac{9}{13.94})$$

T at kelvin

$$= 9.1966 - ~~0.225~~ 1.77$$

$$= 7.42$$

$$E_a = 0.35 \left( 1 + \frac{u_2}{160} \right) (e_w - e_a)$$

$$= 0.35 \left( 1 + \frac{10 \times 24}{160} \right) (38.82 - 20.177)$$

$$= 16.31$$

$$\textcircled{a} \text{ PFT} = \frac{2.16 \times 7.42 + 16.31 \times 0.49}{2.16 + 0.49}$$

$$= 9.064 \text{ mm/day}$$

$$= (9.064 \times 30) \text{ mm in June}$$

$$= 271.9$$

exam (3)(3)(3) why actual differs from PET

- soil saturated  $\Rightarrow$   $\frac{ET}{PET}$  [soil dry  $\Rightarrow$   $\frac{ET}{PET}$ ]
- vegetation  $\Rightarrow$  [coverage diff]
-

Lec 8

यदि हमन कापण्ड पावे लेखाने wind speed,  
sunshine hour नाहे or एरे खाति ना लेखाने  
empirical formula use कराये ,

But that's not accurate much.

(\*) Blaney - criddle equ<sup>n</sup>:

$$E_T = e \cdot 54 k f$$

$$f = \sum P_n \left( \frac{\bar{T}_f}{100} \right)$$

at °f

Tables:

① क एरररर table  
खाहे  
(differs by crops)

② Monthly day time  
hour %

\* Problem:

for an area in South India (Lat = 12°N)  
the mean monthly temp are given

Month	June	July	Aug	Sep	Oct
Temp (°C)	31.5	31.0	30	29	28
Temp (°F)	89.5	89.2	86	84.2	82.4

calc the seasonal consumptive use  
of water for the rice crop in the  
season June 16 to Oct 15.  
using Blaney-Coiddle formula.

not whole month ( $\frac{1}{2}$  factor multiply)  
20 200 last by 12

Sol<sup>n</sup>:

$$F = \frac{\sum P_h \bar{T}_f}{100}$$

June  $\Rightarrow \frac{1}{2} \times P_h \times T_f = \frac{1}{2} \times 8.68 \times 49.5$

July  $\Rightarrow P_h \times T_f = 8.94 \times 49.2$

Aug  $\Rightarrow P_h \times T_f = 8.76 \times 48.6$

Sep  $\Rightarrow P_h \times T_f = 8.26 \times 48.1$

Oct  $\Rightarrow \frac{1}{2} \times P_h \times T_f = \frac{1}{2} \times 8.31 \times 47.6$

---


$$\sum P_h \bar{T}_f = 1675.9$$

$$F = \frac{\epsilon P_h T_f}{100}$$

$$F = 16.754$$

For rice,  $k = 1.10$

$$E_T = 2.54 \times 1.10 \times 16.754$$

$$= 46.81 \text{ (in a crop season) in cm}$$

$$= 46.81 \text{ cm.}$$

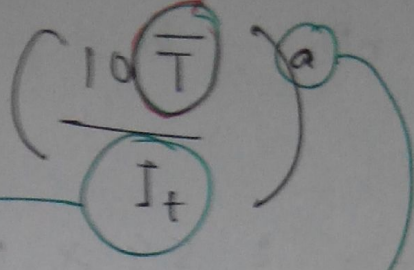
$$\left(\frac{I}{2}\right) = 1.01$$

$$E_T = 2.54 \times 1.10 \times 16.754$$

$$= 46.81 \text{ cm}$$

# \* Thornthwaite formula:

adjustment of no. of hours of daylight and day in month (related to Lat)  $E_T = 1.6 L_a$



empirical const

$$= 6.75 \times 10^{-7} I_t^3 - 7.71 \times 10^{-5} I_t^2 + 1.792 \times 10^{-2} I_t + 0.49239$$

the total 12 monthly values of heat index

$$= \sum_T^{12} i, \text{ where } i = \left( \frac{\bar{T}}{5} \right)^{1.514}$$

→ PET in em

$$1) i = \left( \frac{\bar{T}}{5} \right)^{1.514}$$

$$2) I = \sum_{i=1}^{12} i \quad [12 \text{ months sun}]$$

$$3) a \rightarrow f(I)$$



\*) Reference Crop Evapotranspiration (ET<sub>r</sub>)

→ ଲା ଶାଢ଼ା ଗାଢ଼ା surface area ↑ ET ↑

→ ଯେ ଢେରୀ ସମ୍ମାନ ET ↓

certain crop for reference କରା ନିରା,  
 ଥିଲେ ଏହା ବାସି ଏ ଅନ୍ୟ କ୍ଷମା ଏବଂ ଢେର  
 କରା ।

FAO → Food & agriculture organization.

Def<sup>n</sup>: hypothetical grass ଏବଂ ht = 0.12 m

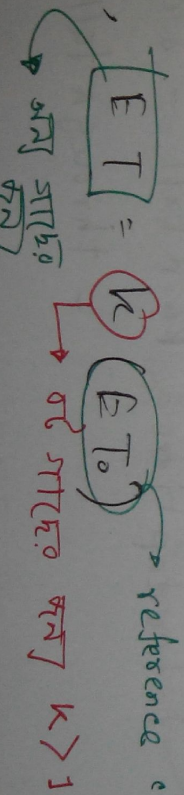
surface resistance = 70 s<sub>m</sub><sup>-1</sup>

albedo = 0.23

ଏହା ଏକ ET ସାମାନ୍ୟ standard

$ET_0$

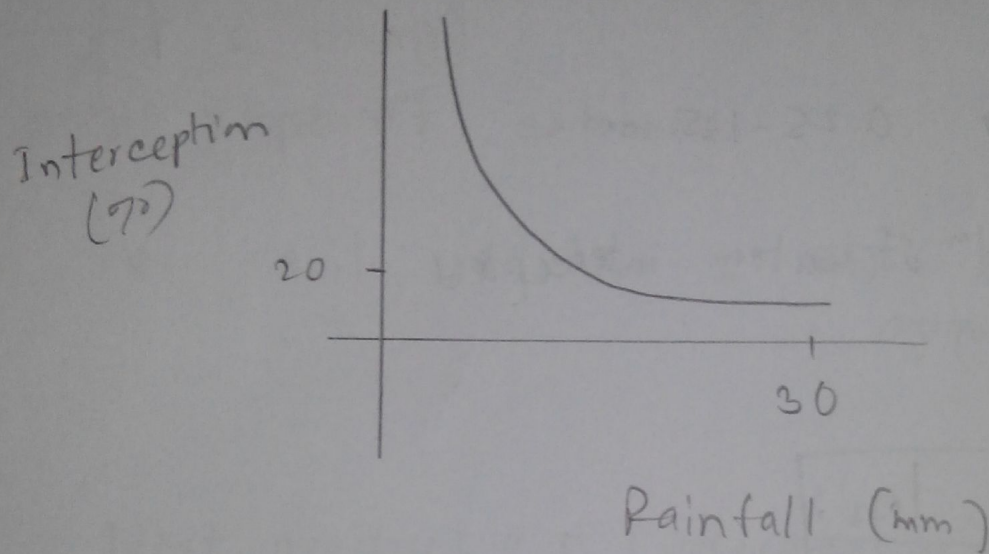
again,  $ET = k (ET_0)$  reference crop ଏବଂ ଏହା





ଦୃଷ୍ଟିରୁ  $\frac{I}{P}$  ଉପରେ interception ପ୍ରାୟ 1507.

" ଗାଢ଼ି " (ଅଧିକ) " " ଯେତେ ସାମାନ୍ୟ,



Factors:

→  $\frac{I}{P}$  ଉପରେ ଗାଢ଼ି surface area

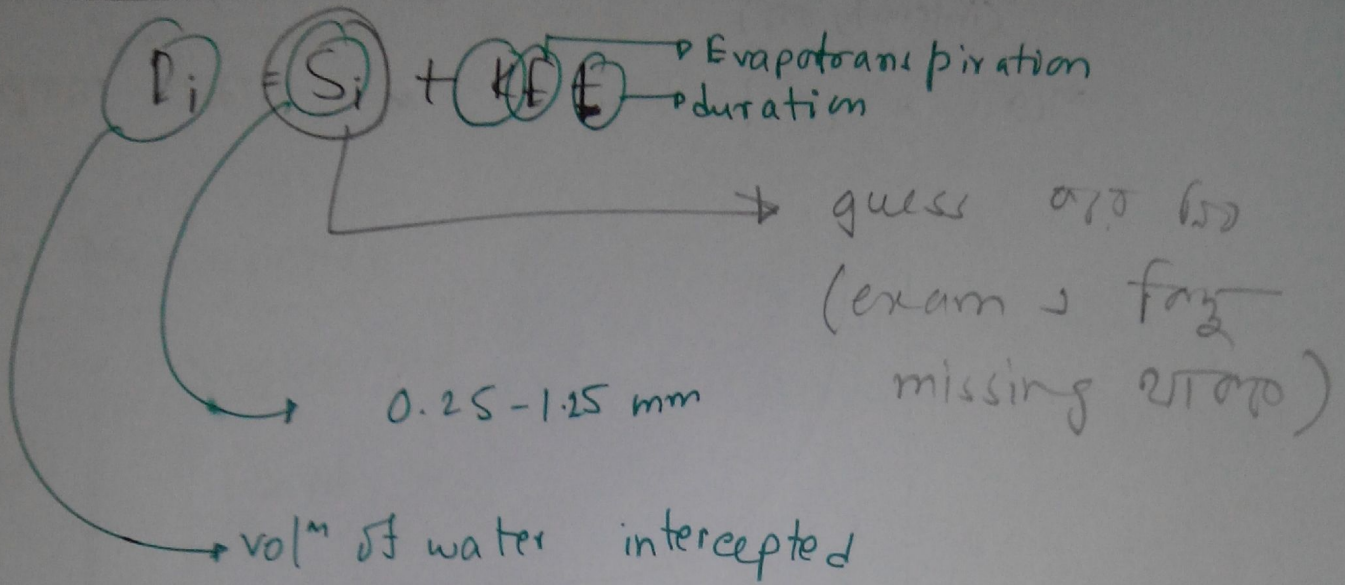
→ " crown ଉପରେ ଥିବା ଚାଞ୍ଚୁ

→ ① storm characteristic; wind speed, intensity

→ ② Vegetation:

→ ③ season: ଋତୁ ଉପରେ interception ମାନ ।

Estimation of interception:



Initial loss:

2. Depression storage

→ ठोस पदार्थ पर जल (कोई)

→ material पर जल

sand पर infiltrate (कोई)

clay " " " "

→ run-off (कोई) contribute (कोई)

" " " "

## Influencing factor :

1. Type of soil : sand, clay.

2. condition of surface : amount and nature

3. slope of catchment

4. soil moisture is  $\sigma_w$   $2\sigma_w$   $\sigma_{sk}$   $\sigma_{sk}$

Next Monday : CT (precipitation)

or is example.

D. Infiltration

→ certain depth तक (to) moisture water (water) free use करे (use) ET करे (use),

→ जो (the) water (water) infiltrate करे (use),

✱

✱✱✱ exam

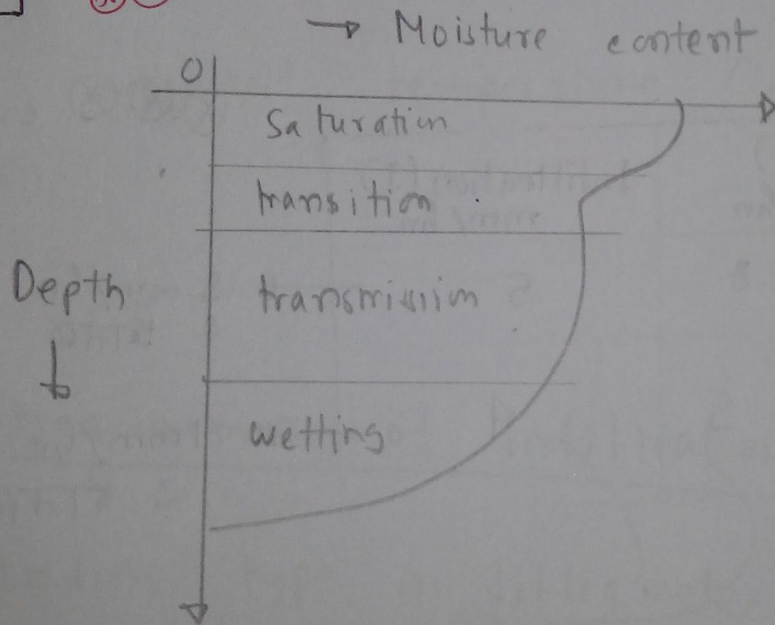


fig: Distribution of soil moisture in the infiltration process

max<sup>m</sup> rate at which a given soil at a given time can absorb water (cm/h)

$$I = I_e \text{ when } i \geq I_e$$

$$I = i \text{ when } i < I_e$$

Rainfall (i) mm/hr		Infiltration (I) mm/hr
cond <sup>n</sup> 1	3	5
cond <sup>n</sup> 2	5	4

⊗ ⊗ ⊗ exam

→ 3 mm/hr rate  
এ ঘটবে

→ 4 mm/hr rate  
এ ঘটবে।

→ 4 hr এ infiltration = 16 mm  
run off = 4 mm

\*] factors affecting infiltration:

→ soil type

→ soil surface (impervious → loss ↓  
pervious                      loss ↑)

→ vegetation cover (depends, <sup>root compact</sup> TN & rice or <sup>root loose</sup> paddy @ different)

→ moisture content

→ soil temperature (dry → temp ↑  
wet " " ↓)

more water → heat holding capacity ↑

\*] Measurement of infiltration:

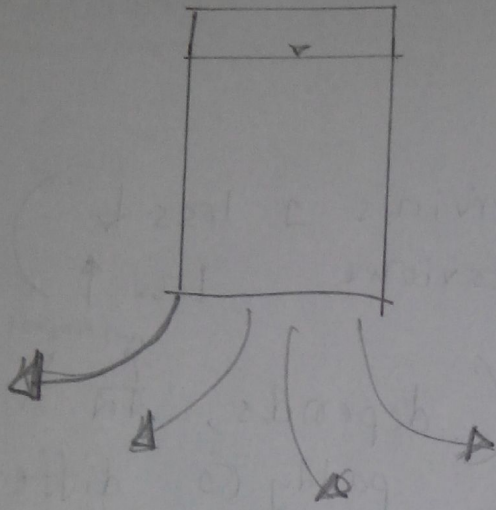
Flooding type in filtrometer

simple & double ring  
double ring

hollow cylinder માટે પૂરવું  
এ নানি মিত্ reading দিচ্ছি। আচ্ছ

আচ্ছ নানি সমস্যা। (যদি বৃষ্টি হয় then  
বড়ো পারে if rainfall rate > infiltration rate)

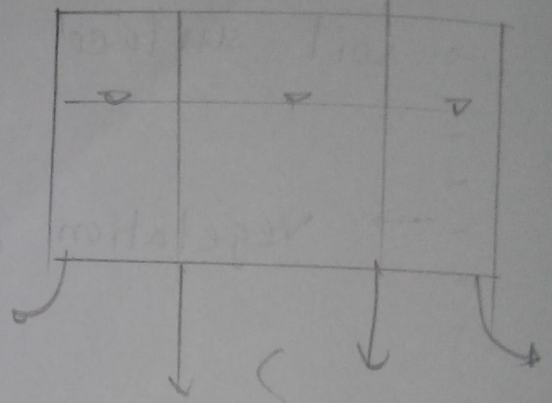
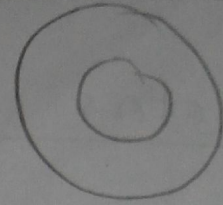
Simple (tube type)



radially

ଅକ୍ଷରା

Double ring



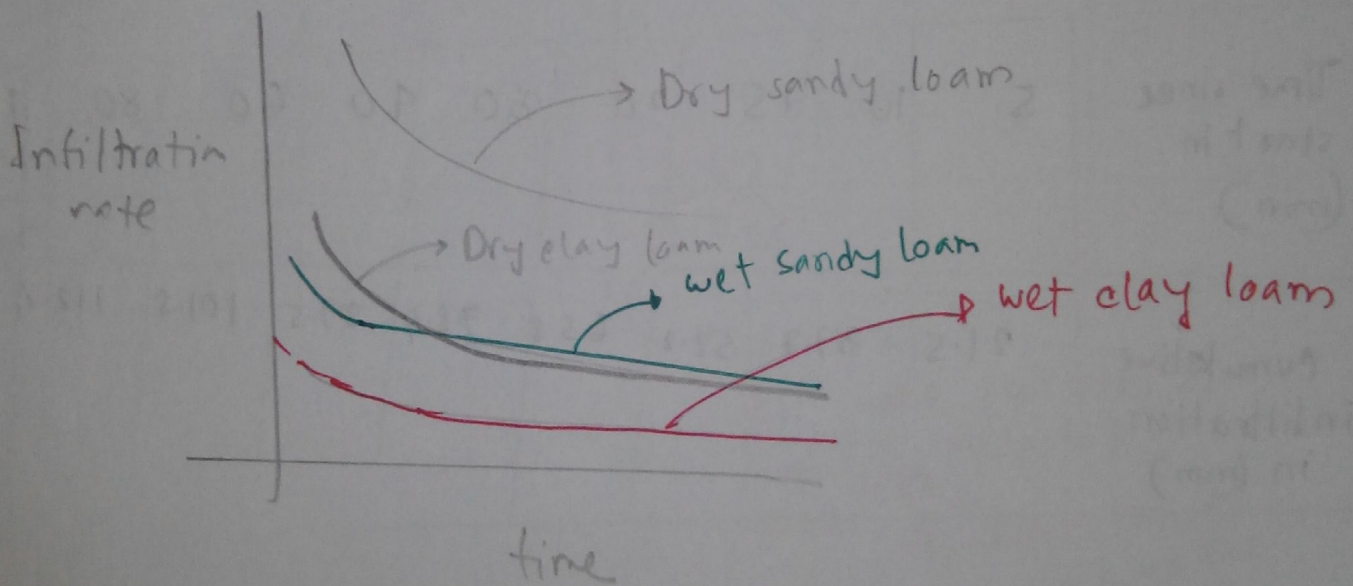
ଅକ୍ଷରା  
vertical reading  
ଅକ୍ଷରା

ଅକ୍ଷରା  
inner tube  
ଅକ୍ଷରା vertical  
ଅକ୍ଷରା radial  
ଅକ୍ଷରା

# ☒ Horton's Infiltration Model

$$f_p = f_c + (f_0 - f_c) e^{-k_h t} \quad \text{--- ①}$$

dry soil & infiltration capacity ~~constant~~   
 continuous  $\rightarrow$   $f_c$  constant



$$\text{①} \Rightarrow (f_p - f_c) = (f_0 - f_c) e^{-k_h t}$$

$$\Rightarrow \underbrace{\ln(f_p - f_c)}_y = \underbrace{\ln(f_0 - f_c)}_c \text{ (intercept)} - \underbrace{(k_h t)}_m \text{--- } x$$

$$\boxed{y = mx + c} \text{ type}$$

Problem

Results of an infiltrometer test on a soil are given below. Determine the best values of the parameters of Horton's infiltration capacity equation for this soil.

Time since start in (min)	5	10	15	20	30	40	60	80	100
Cumulative infiltration in (mm)	21.5	37.7	52.2	65.8	78.4	89.5	101.8	112.6	121.5

Time (min)  
0  
5  
10  
15  
20  
30  
40  
60  
80  
100

In

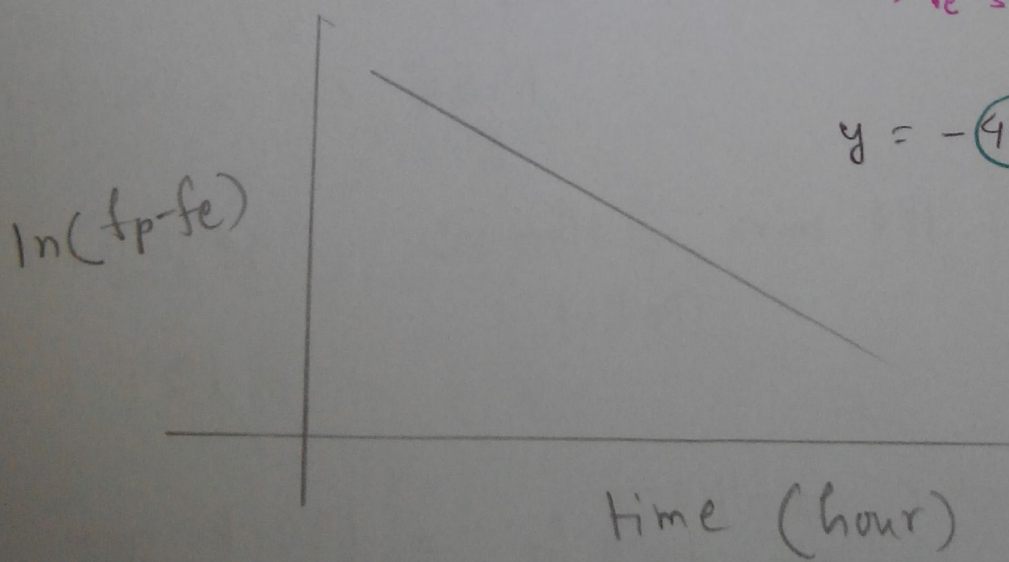
5	2.15	2.1	$\frac{2.1 \times 60}{5} = 25.8$	0.083
10	3.77	1.62	19.44	0.167
15	5.22	1.45	$\frac{1.45 \times 60}{5} = 17.4$	
20				
30				
40				
60				
80			3.69	
100			3.24	

$f_e = 3.24$

last value plot করা না  
 কারণ  $\ln(f_p - f_e) \neq \ln 0 = \text{undefined}$   
 হয়ে পারে।  
 অথবা,  $f_e$  slightly কমিয়ে করতে পারি,

$k = 4.1235$

$$y = -4.1235x + 3.6393$$



$\ln(f_0 - f_e)$   
 $C = f_e = 41.3$   
 $f_0 - f_e = 41.3 - 3.24 = 38.06$   
 $f_2 = 41.3$

$$f_p = 3.24 + \left( \frac{41.3}{-3.24} \right) e^{-4.1235 \times t}$$

\*] from that equ<sup>n</sup>,

$t = 2$  hour  $\rightarrow$  cumulative infiltration = ?

$$\text{infiltration rate } f_p = 3.24 + 41.3 e^{-4.1235 \times 2}$$

rate is variable

But integrate  $\rightarrow$  cumulative infiltration

when rate is variable

So, integration  $\rightarrow$

100 min  $\rightarrow$  const infiltration,  $(f_c = 3.24)$   
100 min  $\rightarrow$  cumulative = 123.3 mm

So, last 20 min  $\rightarrow$

$$\text{total} = 3.24 \times \frac{20}{60} = 1.08 \text{ cm} = 10.8 \text{ mm}$$

$$\therefore 120 \text{ min or } 2 \text{ hr } \rightarrow = 123.3 + 10.8$$

$$\text{Total} = 134.1 \text{ mm}$$

cumulative infiltration at  $t = 2$  hr = 134.1 mm

CT  $\rightarrow$   $\rightarrow$



Pulse number	Time	Cum rainfall	Incremental rainfall	Intensity (cm/hr)
1	0.5	0.25	0.25	0.5
2	0.5	0.5	0.25	0.5
3	0.5	1.1	0.6	1.2
			0.5	1
			1.0	2
			0.9	1.8
			0.2	4.4
			0.8	1.6
			0.8	1.6
			0.8	0.8

If the storm produced a direct runoff of 3.5 cm at the outlet of the watershed, estimate  $\phi$ -index of the storm and duration of R.F.

Time from start (hr)	Cumulative rainfall (cm)
0	0
0.5	0.25
1	0.5
1.5	1.1
2	1.6
2.5	2.6
3	3.5
3.5	5.7
4	6.5
4.5	7.3
5	7.7

let,  $n=7, m=8$   
 7th pulse 3 run-off लायति (1, 2, 10 3 पाएनि)

$$\sum_{i=1}^{10} (I_i - \phi) \Delta t = \text{run-off } (3.5 \text{ cm})$$

7th pulse लत,  $\sum_{i=1}^7 (I_i - \phi) \Delta t = 3.5 \text{ cm}$

$$\Rightarrow \sum_{i=1}^7 I_i \Delta t - n \phi \Delta t = 3.5 \text{ cm}$$

$$\Rightarrow \Delta t [15.4 - 7 \times \phi] = 3.5$$

$$\Rightarrow 15.4 - 7\phi = \frac{3.5}{0.5} = 7$$

$$\Rightarrow 7\phi = 15.4 - 7$$

$$\Rightarrow \phi = \boxed{0.943} \rightarrow 7 \text{ चिं. value एउ (एउ)$$

But  $n=8$  एउ वन]  $\phi = 0.925$ . [मिलेनि]

let,  $n=5$ . तम,  $\phi = 0.88$ . [matched]

[exam 1 n एउ trial 1-2 वार दिनेहरू]

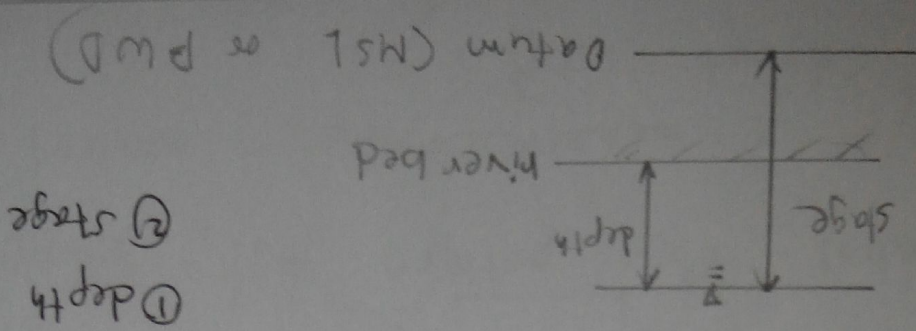
## W-index

→ math का जो मान निकले।

→ Def<sup>n</sup> से ज्ञात।

$$W = \frac{P - R - I_0}{I_e}$$

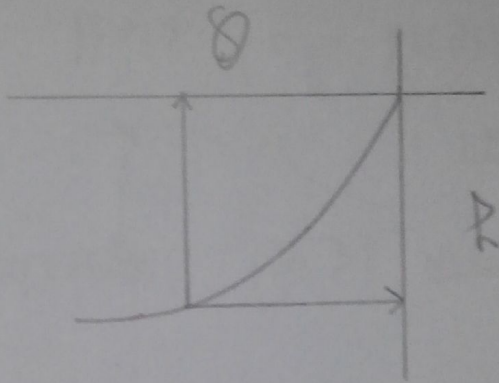
⊗ Difference bet<sup>n</sup> w-index and  $\phi$ -index.



- ① depth
- ② stage



100% data  
 S, M, R,  
 data is not  
 water  
 develop  
 graph



50 days data

→ depth vs discharge curve / rating curve

measure at tough in in!

→ high flow/current alarm currentmeter

flow measure

→ current meter bit 'v' measure  $Q = A \cdot v$

Stream Flow measurement

Chapter 4

lec 11





→ turbulence or effect or water d/s  
→ set station.

\* Advantage over the float-type : (or)

→ well not necessary,

→ inlet obstruct.

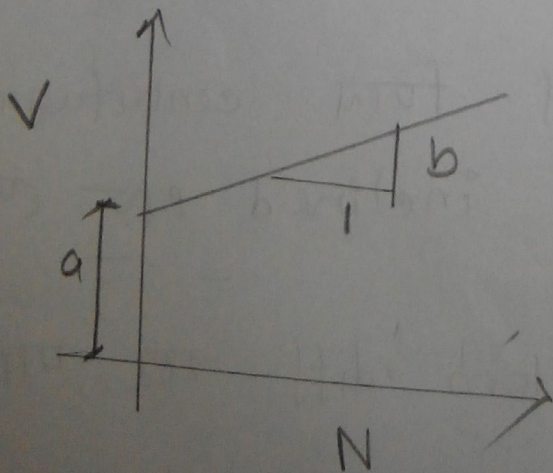
\* vel measurement:

→ vertical axis (or)

→ horizontal " (||)

→ current meter:

$$V = a + bN$$



## \*] Q measurement:

1. Area-velocity method

$$Q = V_{av} \times A$$

$$\checkmark V_{av} = V_{0.6} \quad \text{depth} < 3\text{m} \quad (\text{shallow depth})$$

$$\checkmark V_{av} = (V_{0.8} + V_{0.2}) / 2 \quad (\text{moderate depth})$$

$$\checkmark V_{av} = k \bar{V}_{0.5} \quad \text{for (deep stream-flood flow)}$$

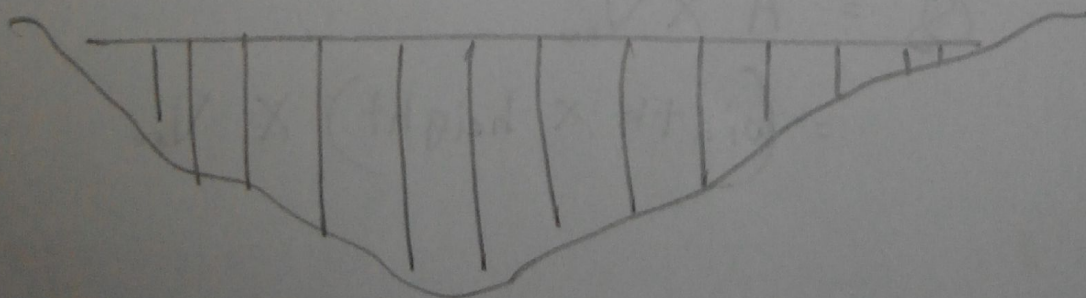
$$k = 0.85 - 0.95$$

vel measurement

1. vel of float  $\Rightarrow V_{av} = C V_f$

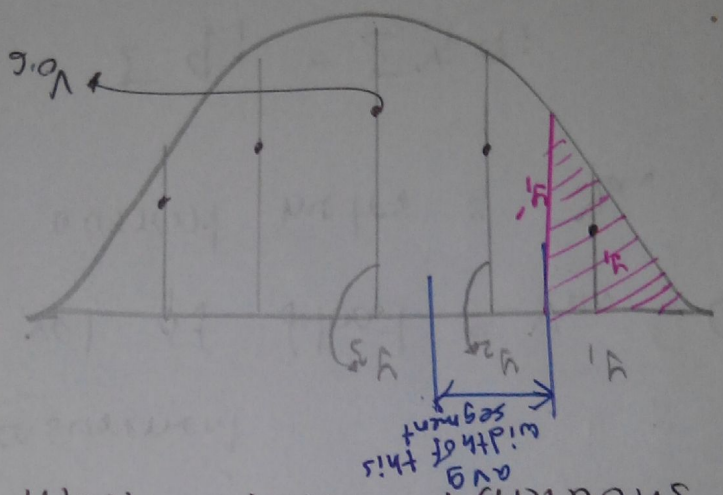
2. current meter  $\Rightarrow V_{av} = a + bN$

$$Q = \sum q_i = \sum v_i a_i$$



Guidelines of choosing sec:

- The segment width shouldn't be greater than  $1/15$  to  $1/20$  of the width of the river
- The  $Q$  in each segment shouldn't be less than 10% of the total  $Q$ .
- The diff of vel in adjacent segment shouldn't be more than 20%



$$Q = A \times V_{av} = (\text{width} \times \text{height}) \times V_{av}$$

Let - shallow depth

triangle ( $\Delta$ ) ବି.ସ. ଉପରେ

$$\frac{y_1'}{y_1} = \frac{w_1 + \frac{w_2}{2}}{w_1}$$

$$\Rightarrow y_1' = \frac{w_1 + \frac{w_2}{2}}{w_1} \times y_1$$

$$A = \frac{1}{2} \times y_1' \times \left( w_1 + \frac{w_2}{2} \right)$$

$$= \frac{1}{2} \times \frac{\left( w_1 + \frac{w_2}{2} \right)}{w_1} \times y_1 \times \left( w_1 + \frac{w_2}{2} \right)$$

$$= \frac{1}{2} \times \frac{\left( w_1 + \frac{w_2}{2} \right)^2}{w_1} \times y_1$$

← avg width

rectangular ( $\square$ ) ଉପରେ ଉପରେ,

$$A = y_2 \times \frac{w_2 + w_3}{2}$$

← avg width

end section ( $\Delta$ ) ଉପରେ ଉପରେ,

$$A = \frac{1}{2} \times \frac{\left( w_N + \frac{w_{N-1}}{2} \right)^2}{w_N} \times y_n$$

Problem: 0.6 d से निम्न गलत है → direct (सिद्धि)।

But यदि क्ला ना था तो  $(V_{0.2}, V_{0.6}, V_{0.8})$  का क्या

judgement apply करके  $V_{0.6}$  ही लेंगे

$$\frac{V_{0.2} + V_{0.8}}{2} \text{ ही लेंगे।}$$

(\*) A current meter with calibration eqn

$$V = (0.32N + 0.032) \text{ m/s} \quad \left| \begin{array}{l} N = \text{rev/sec at} \\ V_{0.6} \end{array} \right.$$

Using mid section method, calc Q

Distance from right bank (m)	0	2	4	6	9	12	15	18	20	22	23	24
Depth (m)	0	0.5	1.1	1.95	2.25	1.85	1.75	1.65	1.5	1.25	0.75	0
No. of rev	0	80	83	131	139	121	114	109	92	85	70	0
Obs. time (s)	0	180	120	120	120	120	120	120	120	120	150	0
avg width (m)	0	2.25	2	2.5	3	3	3	2.5	2	1.5	1.125	0
$N = \frac{V}{0.32}$	0	0.94	0.69	1.09	1.15	1.008	0.95	0.903	0.766	0.708	0.467	0

	0	2	4	6	9	12	15	18	20	22	23	24
*												
✓	0	0.174	0.253	0.381	0.403	0.36	0.336	0.323	0.277	0.258	0.22	0
q		0.196	5.566	1.85	2.72	1.998	1.764	1.33	0.831	0.484	0.186	0

$$\sum q =$$

$$Q = 11.895$$

OT - अविवरण (chap 5)

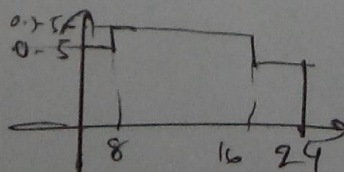
Ques: infiltration upto 15 hr →  
 " ~~15~~ after 15 hr →

E = given

loss = given

$$\text{runoff} = \boxed{\text{Precipitation}} - \text{loss}$$

hyetograph



$$\text{vol/m} = P.$$