

Book ⇒ 1) Water supply Engg. 2) Water safety plant  
H. A. Aziz

01.9.2015

CE-331

Environmental Engg-I

Delwar Sir (Tuesday)

\* অসুখ বনা ইত্য ⇒ প্রধান Water supply & sanitation  
হিসে  
i.e. ডিম্বাণী ইত্য  
সহজোদায়া অসুখ etc.

\* Topics :

- 1) Pumps & pumping machinaries
- 2) Water distribution systems, analysis, design of distribution network
- 3) Water safety plans.

Imp { \* first water treatment plan of Dhaka - 1876  
\* First of 18th century - Modern water treatment starts  
1939 ⇒ This plant is redefined

\* Water supply ⇒ Public health engg.

↓  
cz pure water prevents diseases  
& protects the health of the people.

\* Diseases (water borne) -

Tifoid, Diarrhoea, Pneumonia, अरत etc.

\* Objective of water supply system -

- 1) To supply safe & wholesome water to consumers
- 2) To supply water in adequate quantity
- 3) To make H<sub>2</sub>O easily available to consumer.

# Elements of H<sub>2</sub>O supply system -

- 1) Source of H<sub>2</sub>O supply - (i) ground H<sub>2</sub>O (ii) surface H<sub>2</sub>O.
- 2) Collection system - (i) intake (ii) intake main (iii) pump (iv) transmission main.
- 3) Treatment units
- 4) distribution system - (i) gravity system (ii) direct pumping (iii) pumping with storage reservoir.

अरत ←  
It is the best one  
Jeruzalem, Greece, Mesopotemia,  
Hortoppa, Mohen-jodaro,

↓  
both gravity & direct pumping

e.g. overhead reservoir

↓  
But it is costly

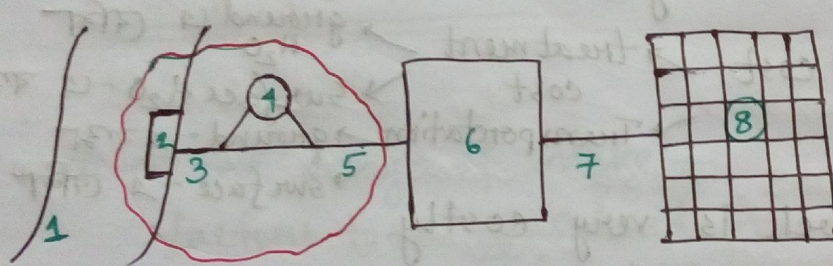
## # Intake velocity and depths -

1) Intake entrance should lie 10' to 15' below the  $H_2O$  surface but 4' to 6' above the river bottom

2) Entrance velocity 3' to 4" per second

3) Grating or screens of 2 to 8 mesh to an inch are provided at the intake entrance

Intake pipe are designed at velocity 3' to 4' per second.



Collection System

Fig: Flow diagram for  $H_2O$  supply system.

Here, 1. Source

2. Intake

3. Intake main

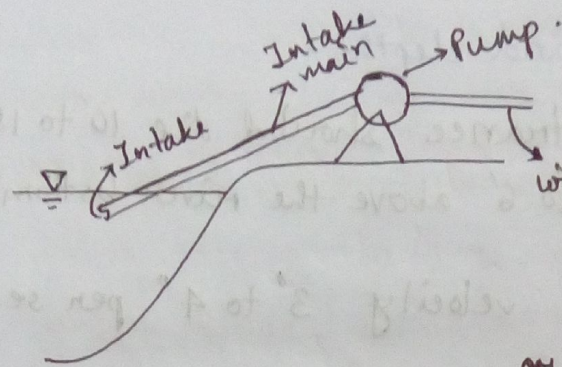
4. Pump

5. Transmission main

6. Treatment unit

7. Delivery main

8. Distribution system.



will be transmission main if it is attached to treatment plant

or it will be delivery main if it is attached to delivery unit.

• What will be the source will depend on three things—

1) Quantity  $\Rightarrow$  adequate quantity  $\text{সরবরাহ হবে।}$

2) Quality

3) cost  $\rightarrow$  treatment  $\rightarrow$  ground  $\text{এ বেশি}$   
 $\text{cost}$   $\rightarrow$  surface  $\text{H}_2\text{O}$   $\text{এ কম}$   
 $\rightarrow$  Transportation  $\rightarrow$  ground  $\text{—এ কম}$   
 $\rightarrow$  surface  $\text{—এ বেশি}$

\* tubewell is very costly

$\downarrow$   
 ez it needs a strainer

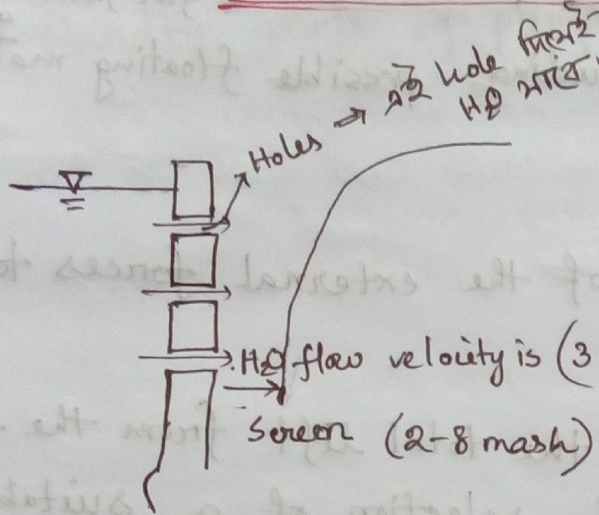
$\downarrow$   
 Then gravel  $\text{ফেলতে হয়।}$  gravel means coarser sand i.e. sand coarser than sylhet sand

$\downarrow$   
 but  $\text{সিঁকড় গুণ্ডা ফেলতে না পারলে}$  segregation  $\text{হবে।}$

$\downarrow$   
 so  $\text{কিন্তু পাম্প চিরকাল না।}$

## Chapter-4

### Surface Water Collection & Transportation



Intake pipe filter  $H_2O$  पावर  
जस velocity is 3-4 ft/s.

$$Q = AV$$

$$3-4 \text{ ft/s.}$$

A जानव

So Q पावर.

Fig 4.1 & 4.2 (from book).

#### • Location and design of intake —

factors to be considered in locating and design of intake —

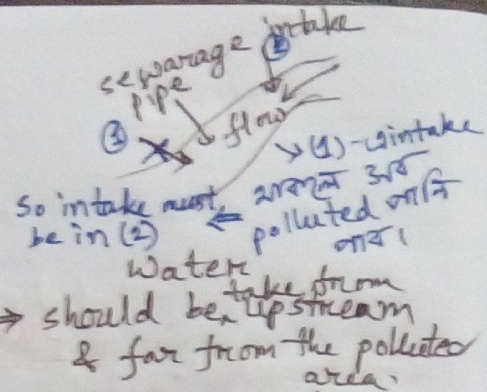
(previous Intake velocity & depths)

- 1) Location of the best quality of  $H_2O$
- 2) Possibility of wide fluctuations of  $H_2O$  level.
- 3) Characteristics of intake surrounding i.e. depth of  $H_2O$ , character of the river bottom, navigation, effects of waves, currents, floods and scouring the river bed & banks.

4) Formation of shoals & bars

↓  
↓  
↓

5) Possible sources of pollution →



6) Provision for excluding possible floating materials

like logs & vegetations

7) The magnitude of the external forces to be resisted by the intake

8) Consideration of the total lift from the source to the treatment plant & selection of a suitable pump unit.

9) Determination of the total length of suction & delivery mains, head losses due to friction & small bends, enlargement & reduction.

10) Selection of suitable screen to provide around the intake

11) Installation of intake valves or port holes at 2/3 diff. levels

12) Determination of cost benefit ratio.

21.01.20

13) Assurance of the safety of the intake structure.

14) Provision for future extension and installation of standby units of pumps → alternate ways

শাক্তি হবে  
মহি লান একই নক্টে হয়।

But লানডায়েই H<sub>2</sub>O উর্ডিস

বকা মাঝে না।

\* pipes should be corrosion free so materials must be corrosion resistant.

Effect of corrosion

1. It brings corrosion products into the water.

2. Acidity / Alkalinity

3. Sulphur compound

4. Temperature → corrosion

\* Two types of pipes for transmission —

1. Gravity Pipe  $\Rightarrow$  Sewerage line

2. Pressure Pipe  $\Rightarrow$  (मत head loss कम है।)

$\downarrow$

Water Supply line

Concrete pipe

Steel pipe

Cast iron pipe

PVC / HDPE pipe

$\downarrow$

High Density Poly Ethylene.

\* Pipes should be corrosion free i.e. materials must be corrosion resistive.

# Effects  
# Causes of corrosion —

1. Pitting  $\Rightarrow$  anode & cathode क्षेत्र है। So localized e<sup>-</sup> pass है।

2. Acidity / Alkaline

3. Sulphur compound

4. Temperature  $\rightarrow$   $\uparrow$  corrosion  $\uparrow$ .

5. Organic Particle (Biological Action)

6. Velocity of flow

7. Cavitation

### # Corrosion Control -

# Sealing  $\Rightarrow$  hard water  $\Rightarrow$  scaling  $\Rightarrow$  scaling  $\Rightarrow$  scaling

### # Forces acting on pipes -

1. Internal forces due to static head

2. " " " " water hammer

3. Forces at bends and changes in cross-section

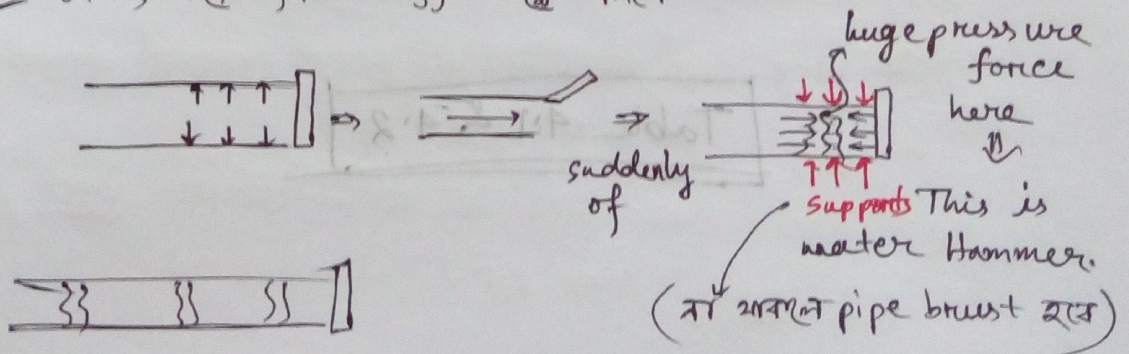
4. Forces due to temperature changes

5. External forces in the form of backfill, traffic, own weights.

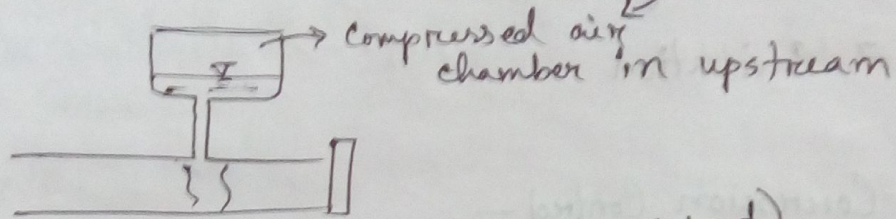
1. hoop stress  $\Rightarrow \sigma_h = \frac{PD}{2tD}$

longitudinal "  $\Rightarrow \sigma_L = \frac{PD}{4t}$

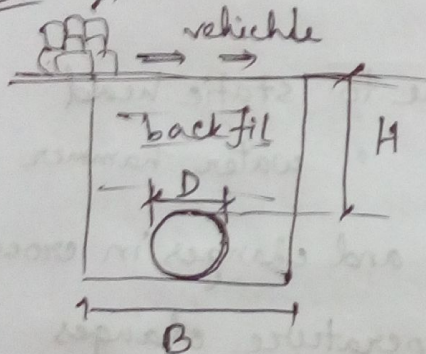
2.  $\Rightarrow$   $\Rightarrow$  flow off  $\Rightarrow$  pipe



237) bulb jointed pipes off water tap, but off slowly  
 off tap or prevent preventive measure



5. extra UDL / point load. (superimposed load)



Shallow height.

1. Flexible pipe  $\Rightarrow$  load  $\Rightarrow$   $\frac{1}{4}$  (PVC)

2. Rigid pipe  $\Rightarrow$  load  $\Rightarrow$   $\frac{1}{4}$  (steel)

The pressure,

$$\left\{ \begin{array}{l} P = c \gamma B^2 \Rightarrow \text{rigid pipe} \\ P = c \gamma B D \Rightarrow \text{flexible pipe} \end{array} \right.$$

trenched width

if trench width  $\Rightarrow$   $\frac{1}{2}$  width  $\Rightarrow$  rigid pipe then

$$P = c \gamma D^2$$

\* normally trenched width dia  $\Rightarrow$  1'-1.5' (rigid)  
 $\Rightarrow$   $\frac{1}{2}$  dia

Table 4.1 & 4.2





# Pipe joints —

Requisites for joints — 8 points.

# Pipe Laying —

Some steps for this —

(All these type of works i.e. water lines/gas lines must be done on government Land)

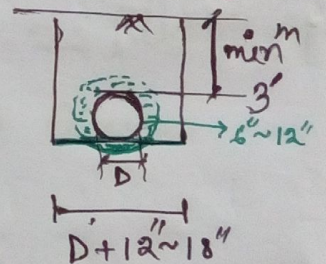
- 1) Prep. of detailed maps of roads & streets.
- 2) Locating the proposed alignment of the ground ⇒
  - curved शत 25'/50' नर नर trench लि
  - straight " 100' " " " "
- 3) Excavating trenches. & (9) Back filling —

⊕ pipe नर चरु नरुन 6"~12" cushion materials / sand लिने लेने देनर. शरु.



⊕ pipe लेन नरुनरुन नर नरुन अरुनरुन ground ले groove नर अरु नरुन शरु।

⊕ groove लेन लेने नर नरुन अरुनरुन sand / CC layer लेने शरु।



↓  
3' नरुनरुन शरु  
ले Traffic नरुन  
impact load  
लेन नरुन लेने  
नरुन लेने  
pipe नरुन  
लेने

6) Laying — normally slope is given 3:1 but  
given 5:1

8) Anchoring — ee block given anchoring 2:1

10) Pipe testing — Apprx. 70psi pressure given  
pipe — 2 marks

Class test → Chapter - 4 (full)

& Chapter - 1 (only location of intake)

5Q & fill in the blanks

# Water Distribution System

## Chapter - 9

- 3 objectives —

1) to supply adequate quantity of water at adequate pressure.

↓  
pressure & supply must be equal

50-60 psi → normal of supply system

100-120 psi → भारत भारत इत-भारत- (special)

80-120 psi → for fire resistance → diff. line भारत in developed countries

↓  
main H<sub>2</sub>O supply 23 भारत  
parallel line भारत high pressure  
↳ water supply भारत।

\* normally hilly area in developed countries

↓  
भारत water supply system create भार tough.

so 200 psi भारक दिहें भारत।

⇒ शीतप्रवर्धन भारत always tap भारत  
भारत भूलें भारत है। जे नो भारत  
freezing है & vol<sup>m</sup> बढ़े भारत  
H<sub>2</sub>O-भार।

2) It should be thoroughly reliable.

3) It " " economical.

Q. Write down / Discuss the advantages or disadvantages of H<sub>2</sub>O supply system.

3 systems —

1. Gravity system
2. Direct pumping
3. Pumping with storage reservoir

# Gravity system — (in Scotland) এ পর্যাপ্ত পরিমাণের জল/area

- It is economic
- It is not dependent on pump or electricity.
- No available source

# Direct pumping —

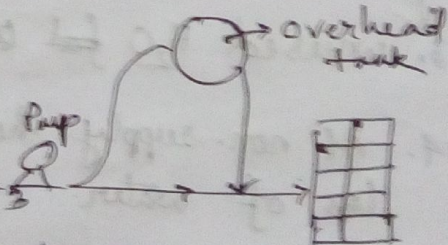
- Dependent on electricity.  
pump কে হলে পানি নেই



- peak — এ সময় বেশি পানি দেওয়ার সময় বেশি পানি লাগে।

# Pumping

- Overhead tank — এ first — এ water তুলে রাখা হয়। So pump কে হলে by gravity overhead tank থেকে water supply পাওয়া যায়।



- overhead reservoir is costly & to take water to this needs huge power.

no. (3) system modify कर Undergroud  
reservoir / Roof tank use कर 25!

• Method of H<sub>2</sub>O supply —

1. Continuous Method

2. Intermittent → भाकर भाके Water supply  
पाठना 25।  
• storage कर 25 for non-supply  
hours.

1. storage is  
economic loss  
& also a 2. greater chance  
of contamination.

Continuous → 24 hrs गानि शकवा At any  
time tap खुलले गानि पाव,  
• storage कर 25 ना.

Q. Continuous method is always better than Intermittent  
Method — Justify the statement.

↳ दोनो method - 25 advantage & dis advantage

दुनो कर 25 या 25 कर 25।

3. unused H<sub>2</sub>O फल खाल देस। So direct wastage of water

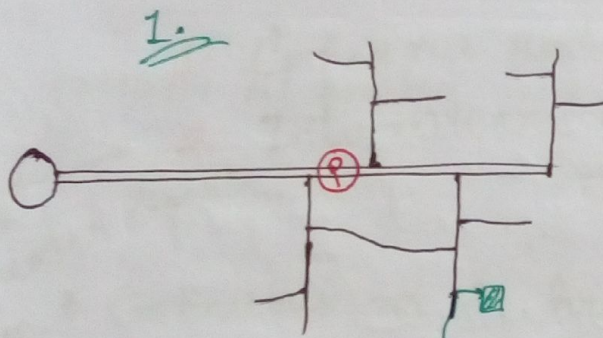
4. At non-supply hour, the tap is being open. So direct  
loss of water

5. If fire breaks in non-supply hour, tremendous damage  
is there than continuous method.



• Layout of water distribution system -

1. Branch or Tree or deadend system
2. Gridiron or Looped system
3. Ring or Circular system
4. Radial system



always deadend system i.e. water staking system

$Q = VA$  generally 3-4 ft/s.

$h_f = f \cdot \frac{L}{D} \cdot \frac{V^2}{2g}$

এখানে কোনও eq<sup>n</sup> দ্বারা solve করা দিা যায়। এ দিা যায়

কারণ rounding করা হয়

- 4"
- 6"
- 8"
- 10"
- 12" ⇒ এর পর 3" এর interval upto 24' - - - -

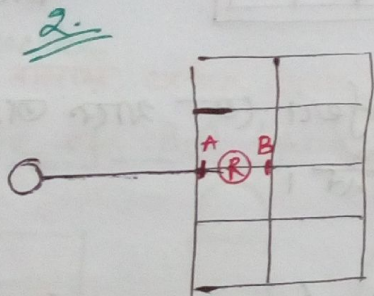
\* different pressure - ১ সিস্টেম।

Normally 50-60 psi but not < 20 psi

Disadvantages :

1. Suppose P to repair valve & valve operation work cannot be done. So system will be in trouble - water supply not.

2. Dead end - waste disposal is not so dead end after 15 days clean cannot be done. Water pressure uniform not, so this waste water - water contaminate.



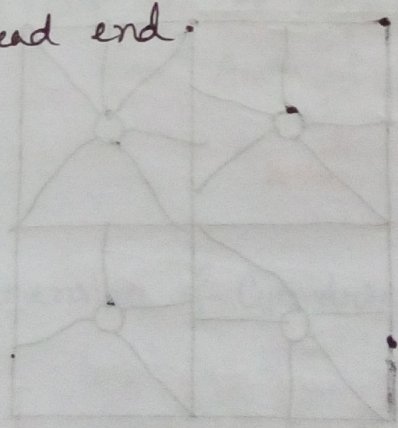
→ If there is a dead end, then repair work cannot be done. A and B are two valves. On and off will be a problem. Water problem will not.

Loop System

↓  
No. of pipes & valves will be greater than dead end.

↓  
This is better

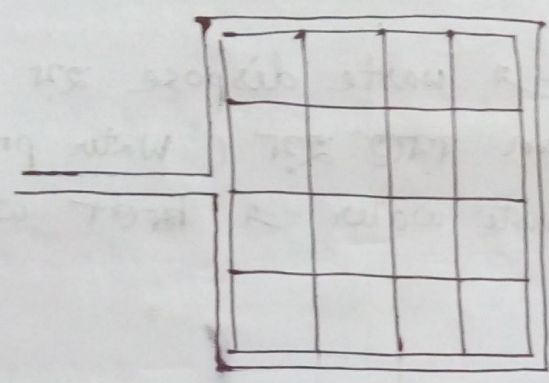
↓  
can be formed in planned city.



(is preferable)

↓  
This is analysed by Hardy cross method

3.

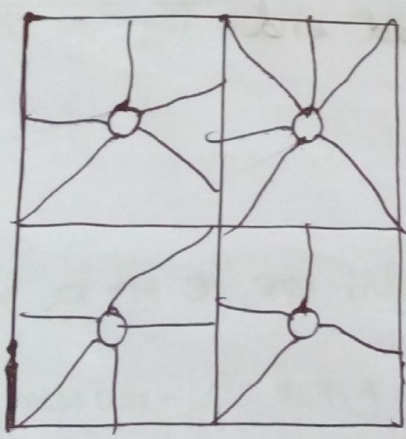


\* (যদি ব্যাসার্ধ width কম হয় তাহলে larger dia - এর pipe রাখাটা ভাল না।)

Main road যদি periferi থেকে থাকে তাহলে বড় dia pipe periphery তে দিবে।

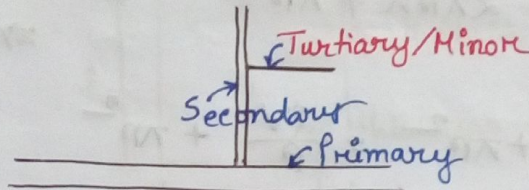
↓  
uniform velo. / pressure - এ water ভাল & so head loss কম থাকবে।

4.



→ খুবই quick গতি চলে থাকে।  
↓  
তালেক similarity with ring system

# In water supply systems  $\Rightarrow$  3 types of lines -



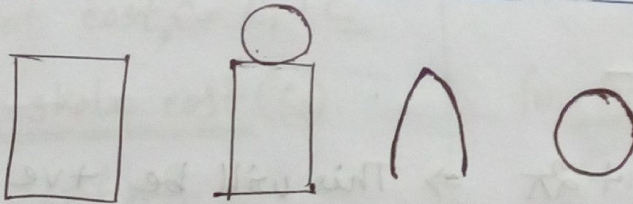
~~1 gal~~

$$1 \text{ ft}^3 = 7.48 \text{ gal}$$

$$1 \text{ gal} = 3.785 \text{ litre}$$

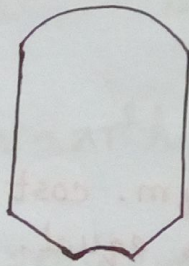
Usual Practice

- असहज वासाय extra connection निचे हलत Minor लाके निचे रह।
- Shape of H<sub>2</sub>O tank / Overhead Reservoir -



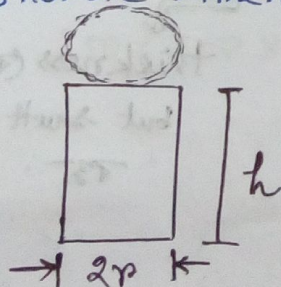
1. Rectangular
2. Cylindrical
3. Oval
4. Circular

5. Intze tank



$\Rightarrow$  { असाधारण design बना रह  
 एत corner effect ना रह  
 for Arch & Shale  $\Rightarrow$  mod अरुत रह  
 in base  $\downarrow$  so economical.  
 bottom 2 sides

# Economic Dimension of Cylindrical Type Water Tank -



$$V = \pi r^2 h$$

Lets assume, per unit cost for base & shale is same.

$$\left. \begin{array}{l} \text{Area for base} = \pi r^2 = A_2 \\ \text{" " " shale} = 2\pi r h = A_1 \end{array} \right\} \text{Vol}^m = \text{const.}$$

$$\therefore A = A_1 + A_2 = 2\pi r h + \pi r^2 \quad \text{but, } V = \pi r^2 h$$

$$\Rightarrow h = \frac{V}{\pi r^2}$$

$$\Rightarrow A = 2\pi r \cdot \frac{V}{\pi r^2} + \pi r^2 = \frac{2V}{r} + \pi r^2$$

first derivative -

$$\frac{dA}{dr} = -\frac{2V}{r^2} + 2\pi r = 0 \quad \leftarrow (1)$$

2nd derivative -

$$\frac{d^2A}{dr^2} = \frac{4V}{r^3} + 2\pi \Rightarrow \text{This will be +ve for } \underline{\underline{\text{min}^m}}$$

from (1)  $\Rightarrow$   
 Now,  $2\pi r = \frac{2V}{r^2}$

$$\Rightarrow 2\pi r = \frac{2\pi r^2 h}{r^2}$$

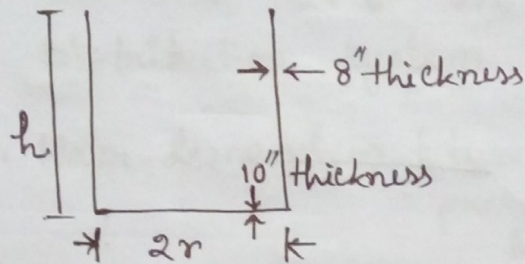
$\therefore \boxed{r = h} \Rightarrow$  when per sq.m. cost for base & shale is equal.

but normally shale cost > base cost

↓  
 shuttering (ब्रि) but thickness (ब्रि)

↓  
 thickness (ब्रि) but shuttering (ब्रि)

- for Diff. base & Shuttering cost —



base cost = 200 TK/cft, shale cost = 250 TK/cft.

$$\therefore \text{Total cost, } C = C_1 + C_2$$

for shale cost ( $C_1$ )

$$\text{Vol}^m = 2\pi r h \times \frac{8}{12} =$$

$$\therefore C_1 = 2\pi r h \times \frac{8}{12} \times 250$$

$$\text{Now, } V = \pi r^2 h, h = \frac{V}{\pi r^2}$$

$$\therefore C_1 = 250 \times 2\pi r \times \frac{V}{\pi r^2} \times \frac{8}{12}$$

for base cost ( $C_2$ )

$$\text{Vol}^m = \pi r^2 \times \frac{10}{12} =$$

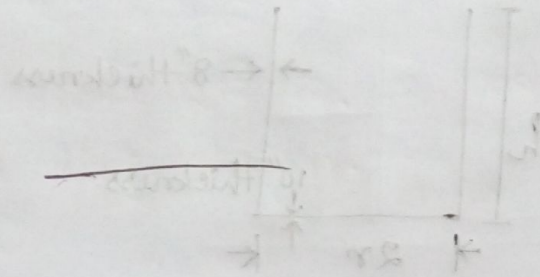
$$C_2 = \pi r^2 \times \frac{10}{12} \times 200$$

$$C_2 = \pi r^2 \times \frac{10}{12} \times 200$$

$$\therefore C = 250 \times 2\pi r \times \frac{V}{\pi r^2} \times \frac{8}{12} + \pi r^2 \times \frac{10}{12} \times 200 = \frac{1000}{3} \frac{V}{r} + \frac{500}{3} r^2$$

$$\frac{dC}{dr} = 0$$

Class test  $\Rightarrow$  Next Tuesday - 17.11.2015  
Only on this class



base cost =  $250 \times L$  / cost of slab =  $250 \times L$  / cost

Total cost (C)

$$C = \frac{10}{12} \times 250 \times L$$

$$C_1 = 250 \times \frac{10}{12} \times 250$$

$$C_2 = 250 \times \frac{10}{12} \times 250$$

Total cost = C<sub>1</sub> + C<sub>2</sub>

cost of slab (C<sub>1</sub>)

$$C_1 = 250 \times \frac{8}{12} \times 250$$

$$C_2 = 250 \times \frac{8}{12} \times 250$$

$$V = 250 \times \frac{8}{12} \times 250$$

$$C_1 = 250 \times \frac{8}{12} \times 250$$

$$C = 250 \times \frac{8}{12} \times 250 + 250 \times \frac{10}{12} \times 250 = \frac{250^2}{12} \times \frac{8}{12} + \frac{250^2}{12} \times \frac{10}{12}$$

$$\frac{dC}{dV} = 0$$

## \* Water Supply network Design -

1. Method of supply
2. Distribution system
3. Water demand  $\Rightarrow$  future water demands, not present  
 $\Downarrow$   
 at the rate of water usage  
 or design period.
4. Normally design flow =  $3 \times \frac{A_v \text{ flow}}{\text{Average flow}}$

## 5. Formula used

### • Factors of design

1. Contour map of the area
2. Detail plan  $\Rightarrow$  mainly street location
3. Future demand
4. Flow = 3 - 4 ft/s  $\Rightarrow$  1st assumption  $\Rightarrow$  mainly diameter selection  
 $\Downarrow$   
 On the basis of this, pipe dia is determined.

Then, head loss =  $f \cdot \frac{L}{D} \cdot \frac{v^2}{2g}$   
 is calculated

5. Design life  $\Rightarrow$  depends on the material used and cost  
 $\downarrow$   
 it is a prime factor

- Normally design life = 30 years  
but SS शत life बहुत कम है।
- Only design का लक्ष्य है कि, ये cost must be recovered by the design.

### \* Network Analysis —

Different methods are there —

1. Hardy Cross Method  $\Rightarrow$  mostly used

This method depends on two factors —

- a)  $\Sigma \text{inflow} = \Sigma \text{outflow}$  at any section/loop.
- and b)  $\Sigma \text{headloss} = 0$  must satisfy in any loop.

- But main drawback is — without any loop, this method can't be used.

### Derivation —

According to Hardy Cross —

$$H \propto Q$$

$$\therefore \underline{\underline{H = K Q^x}}$$

In any flow/loop, according to Hardy Cross

method,  $\boxed{\Sigma H = \Sigma K Q^x = 0}$

Accurate flow/discharge -

$$Q = \underbrace{Q_a}_{Q \text{ assumed}} + \underbrace{\Delta}_{\text{correction}}$$

and  $\Sigma H = \Sigma K (Q_a + \Delta)^x = 0$

$$\Rightarrow \Sigma K Q_a^x + \Sigma x K Q_a^{x-1} \Delta + \frac{x(x-1)}{2} \Sigma K Q_a^{x-2} \Delta^2 + \dots = 0$$

As  $\Delta \approx$  very small, so higher power of  $\Delta \approx 0$ .

$$\therefore \Sigma K Q_a^x = - \Sigma x K Q_a^{x-1} \Delta$$

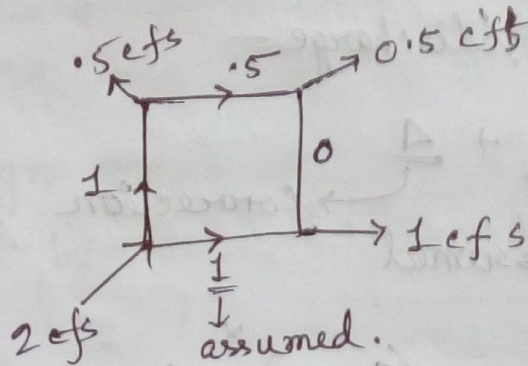
$$\Rightarrow \boxed{\Delta = - \frac{\Sigma K Q_a^x}{\Sigma x K Q_a^{x-1}}}$$

OR,

$$\boxed{\Delta = - \frac{\Sigma H}{x \Sigma H/Q}}$$

Sup. Mathematical Problem —

\* Suppose a closed loop —



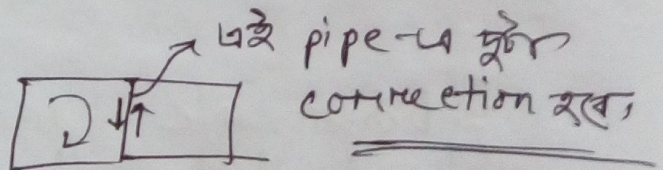
then either clockwise / anticlockwise loop flow direction is assumed. Then head loss is determined.

• According to Hazen William —

$\alpha = 1.85 \rightarrow$  According to Hazen William normally,  
 or  $\alpha \approx 1.7 - 2.2$

The method is repeated for successive trials and the accurate discharge is determined when  $\Delta \approx 0$ .

If two loops  $\Rightarrow$



\_\_\_\_\_

Use of pump:

- Overhead reservoir
- Distribution system
- প্রদেয় নদী line হলে pumping boosting করতে হবে।
- low head - এ কখনও কখনও লাগি উঠে।
- কখনও High head - এ উঠে (tall building)

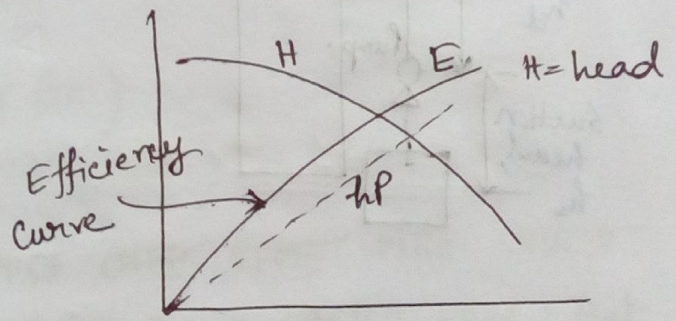
এসবের উন্নয়ন দিওঁ করে pump use করা হয়।

Centrifugal pump ⇒ mostly used

২ কাজের Head কমান

⇒ for designing a pump

$$hp = \frac{HQ}{3960}$$



H = total head (ft)  
Q = flow in gpm

↓  
Habibur Sir - এর  
বইয়ে উল্লিখিত আছে

Use of pump:-

- কাজে underground reservoir থেকে  
overhead tank - এ নিচে

- fire fighting ⇒ আলাদা tank ও থাকতে পারে

প্রদেয় ক্ষেত্রে fire fighting এর জন্য আলাদা reservoir -ও থাকে।

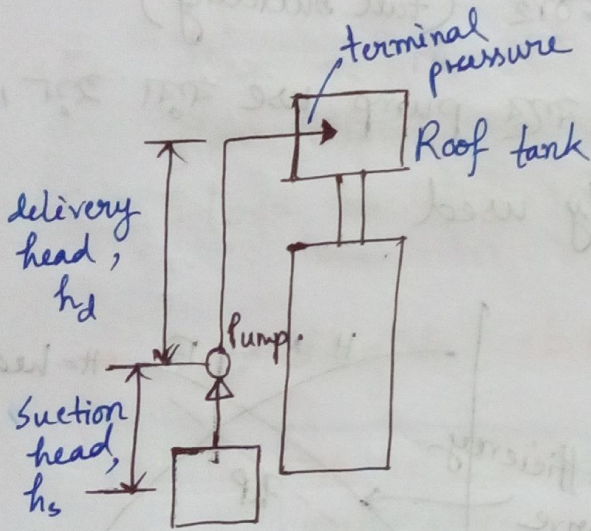
০৮২ = ১০৭ +

— It should be ensured that →

- flow बिबरन मात्र (velocity of flow)
- pressure

- 60 person, perhead consumption 60 gpcd.

↓  
can vary place to place



- ⊛ Generally मात्र/वाहित २ मात्र मात्र डिजाइन & प्रणियत्र 1 hr मात्र डिजाइन मात्र i.e. सकाल & बिकाल

- २ मात्र terminal pressure मात्र (min<sup>m</sup> 5 psi) → प्रत्येक head-१ मात्र मात्र

Let,  $h_m = 2 \text{ psi}$

So,  $H = h_s + h_d + h_f + (h_m + h_t)$   
psi → feet-१ मात्र मात्र

$$h_f = f \cdot \frac{L}{D} \cdot \frac{v^2}{2g}$$

↓  
head loss मात्र मात्र मात्र

- ⊛ किन्तु minor head loss मात्र मात्र as we use pump.

- ⊛ due to — 1) bent & 2) Pump

- ⊛ Normally atm. pressure-१ मात्र head मात्र — 13.82 psi  
↓  
34' of H<sub>2</sub>O

$$1 \text{ psi} = 2.46'$$

\* Now, total gallons per day =  $60 \times 60 = 3600$  gpd.

1 pumping hour = 1 hr/day

$$\therefore Q = \frac{3600}{60} \text{ gpm}$$

If we get the value of  $h_p = 5$  hp.

- কাজের থেকে 5 hp exactly আনা যাবে না।  
as pump 100% efficiency তে চলে না।

$$\text{BHP} = \frac{h_p}{E} = \frac{HQ}{3960E}$$

So graph থেকে Head (H)  
& Efficiency (E) থেকে  
 $h_p$  বের করে নিতে হবে।

(60 to 70% efficiency বিবেচনা করি)

\* আমরা অনেক টা থেকে i.e. পানির থেকে টাকার পানি আনতে  
অনেক বেশি hp i.e. 1000 hp এর pump-ও লাগতে পারে। But  
যতটা pump আমরা দরকার তত at least  $\frac{1}{3}$  standby রাখবে।

as ২টা লাগলে 1 টা standby

↓  
by rotation চালাতে হবে

\* Last year  $\Rightarrow$  আকস্মিক ঘটনা

- math করতে হবে (exam এ থাকবে)
- কিছু type এর pump এর def<sup>n</sup> (exam)

Various formulas are used —

$$h_f = f \cdot \frac{L}{D} \cdot \frac{V^2}{2g}$$

$$V = \frac{1.486}{n} R^{2/3} S^{1/2} \quad (\text{Manning's formula})$$

$$V = C \sqrt{RS}$$

- Minor loss assume করে নিতে হবে (1-2 psi, > 5 psi)

\* Fig 6.1  $\Rightarrow$  monogram দেখা আছে (সিদ্ধান্ত হবে)  
 $\searrow$  Imp. for exam

- \* approximately  $\left\{ \begin{array}{l} \bullet f = 0.4 \\ \bullet f = 0.01 \text{ (if } 4f \text{ is used)} \end{array} \right.$

\*  $n = 0.013$

$\downarrow$   
old value roughness বেড়ে গেলে, friction বাড়ে, n বাড়ে।

(Next class  $\Rightarrow$  Water <sup>Safety</sup> ~~Septic~~ Plant  $\Rightarrow$  BUET বই আসবে)

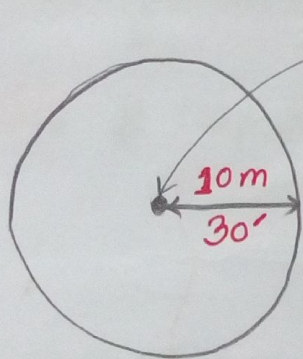
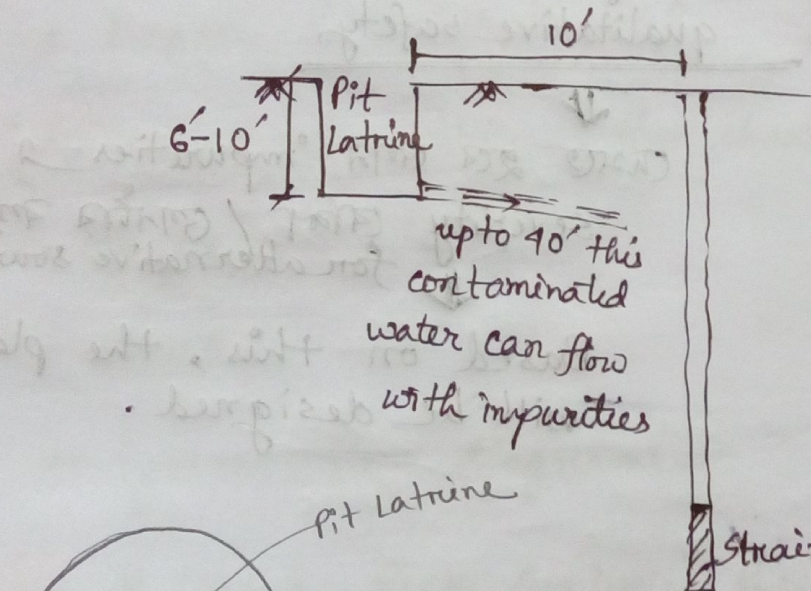
• Water supply system consists of -

- Source

- Collection System

- Treatment Units → Capacity এর অনেক বেশি H<sub>2</sub>O treat করতে হলে safety is a must.

- Distribution



↓  
 এই পুরা circle জুড়ে কোন tubwell থাকতে পারে না। এর অনেক দূরে i.e. এই circle এর বাইরে থাকতে হবে।

So for safety ←

Strainer থেকে নিচে  
 ↓  
 So normally contaminated H<sub>2</sub>O এর নিচে আসার কথা না, but কোন leakage থাকলে strainer - এ থেকে পারে।

- by keeping 10 m / 30' away from the latrine, Contamination from source can be protected.

⇓

But there may be other impurities i.e.

Fe, As etc.

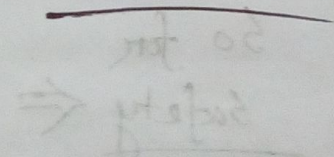
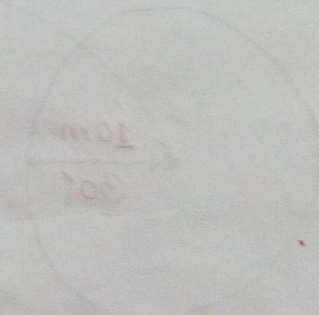
⇓

If they are beyond tolerable limit, then comes qualitative safety

⇓

कम से कम 'impurities' -  
severity level / tolerable limit  
for alternative sources.

Based on this, the plant will be designed.



## Water Safety Plan

- Background
- Concept of WSP
  - Identification
  - Monitoring
  - Overcome

### • Major Activities in WSP

### ~~Step 1~~ Developing a WSP (flow chart)

- Step 1:
- Step 2:

### ← System flow diagrams -

### ~~Step 2~~ • Step 3: Risk Analysis, Prioritization & Controls

- The major tasks -

- (A) Hazard identification & Risk Assessment
- (B) Identification of Controls
- (C)

# \* General approach to risk analysis

## Table

1. High (H)
2. Moderate (M)
3. Low (L)

Sup. for math

Semi quantitative approach

Risk Score = Likelihood x Impact.

- Estimation of "Risk Score" and "Risk Categories"

↓  
(Matrix)

v.v. says

	<u>Likelihood</u>	<u>Impact</u>	
Bacteria →	almost certain	Major	
Arsenic →	Severity varies	Major Minor	
	0.05 mg/L		
Fe →	Almost certain	Minor	⇒ This is because Fe does little harm to people.

Math

Pit latrine - এর কাছে / দূরে tubewell

↓

Then দেখতে হবে strainer সঠিক contamination  
যেতে পারবে কি না। আর অনুমানী likelihood &  
Impact বের করবে।

(<sup>↓</sup> Problem from book)

- Ex 1.(a), 1.(b), 2(a), 2(b) etc - - -