

CE-351

26 = 100%

HASIB

SIR

Transportation planning: 06, 08, 04

Transportation planning consists of two terms:-

- (i) Transportation.
- (ii) Planning.

Transportation means the movement of goods and people. Planning means to find out a goal. If the goals are different, then the activities will be different. Planning must have alternatives. The goals of the roadway transportation is to produce a safe, efficient and convenient system. Set S R

Transportation planning is primarily the process of producing information that can be used by the decision makers to better understand the consequences of different course of actions.

→ planning is undertaken for four reasons:-

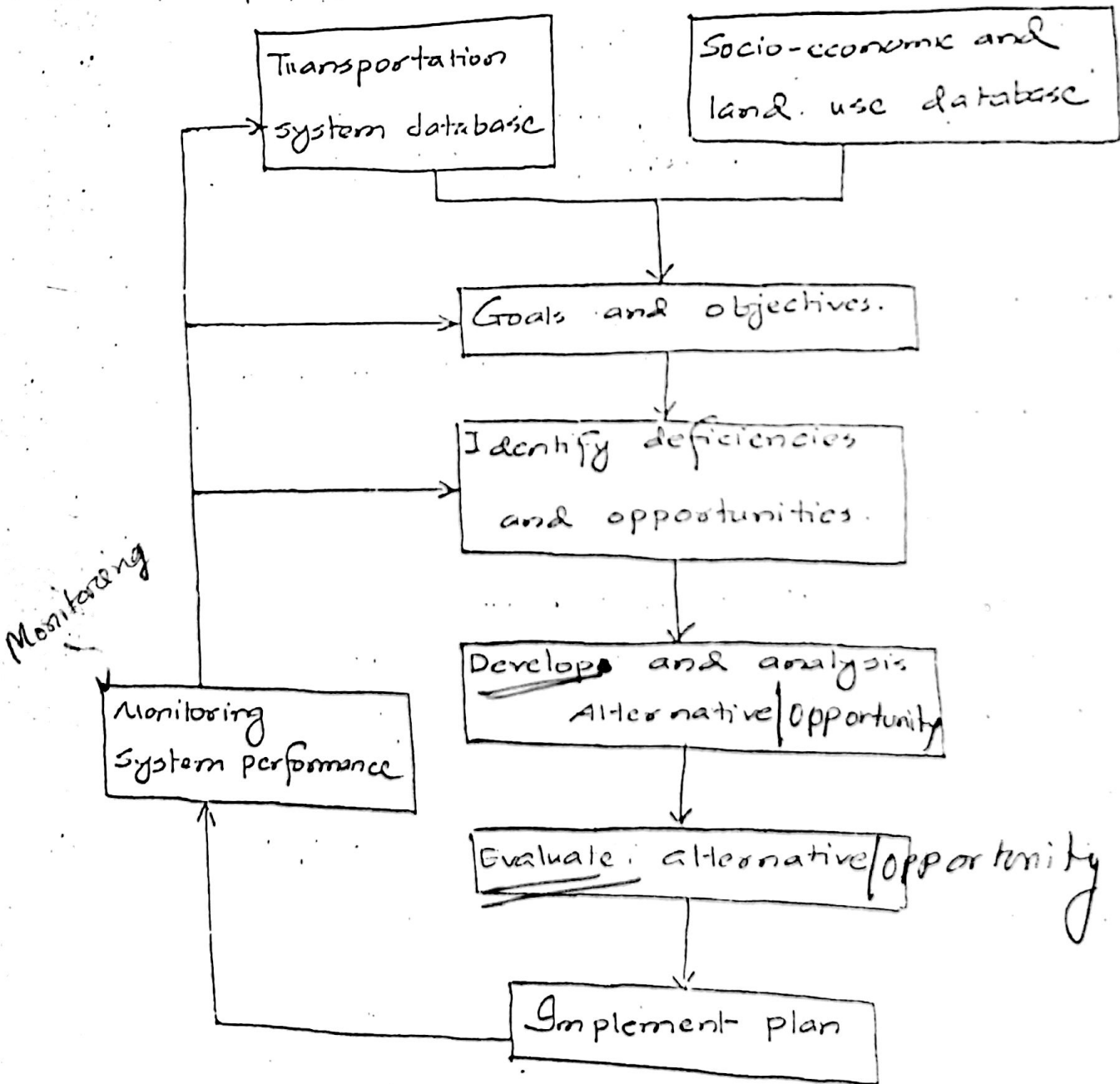
- (i) To ensure the efficient allocation of resources.
- (ii) To ensure the equitable allocation of resources.
- (iii) To widen the community choices by offering alternatives and options.
- (iv) To make possible prediction of future needs and the consequences of proposed scheme.

→ Equity may be two types:- (i) on the basis of needs.

(ii) on the basis of contribution.

Basic Elements of Transportation Planning:

Transportation planning is primarily a process of providing information that can be used by the decision-makers to understand the consequence of different course of action.

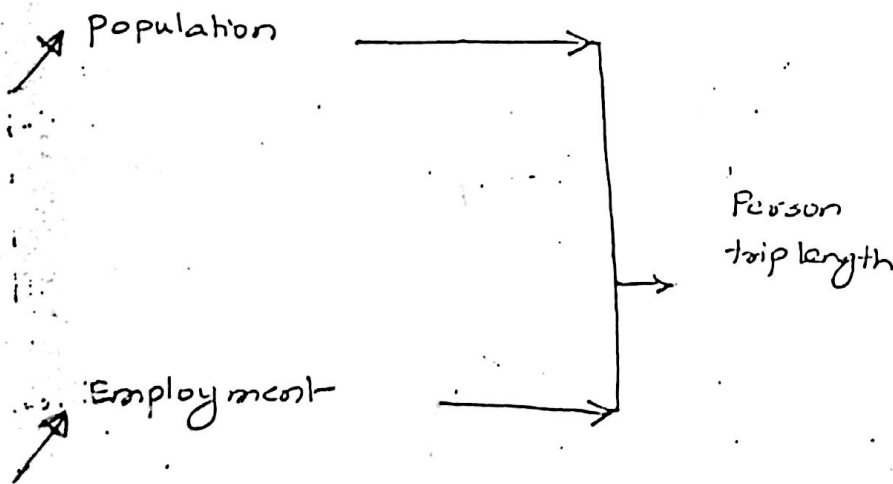


Transportation System Modeling:

Transportation planning is concerned with the future demand and putting in place the facilities and services that will accommodate these demands. The main challenge of the transportation planner is to make a reliable forecast that will effect the population social and economic condition as well as the change in traffic network. Factors affecting the growth of personal travel are given below:-

→ Trips per capita.

→ person / household



107 109

* Recognized components of traffic travel demand include:

'10-11, '08-09,

01) Existing traffic:

Traffic currently used in an existing highway that is to be improved.

02) Normal traffic Growth:

Traffic can be explained by:-

- (i) anticipated growth in state and regional population
- (ii) Area-wise change in land use.

03) Diverted traffic

Traffic that is switched to a new facility from nearby roadways.

04) Converted traffic:

Traffic changes resulting from change of mode.

05) change of destination traffic

Traffic has been changed to different destinations, where change is attributed to attractiveness of the improved transport and not to change in land use. attractive to transport

06) Development traffic

Traffic due to the improvement in adjacent land in addition to the development that have been taken place had the new or improved highway not been constructed.

07) Induced traffic

Traffic that represent the trips that would not have made but now are because of improved transportation have

END. C.C. D.T

Basic Concept In Transportation System Modeling:

Most of the transportation models are based on some fundamental assumptions and approaches that will heavily influence how these models are used by transportation planners and engineers. The most important are as follows:

Tripmaking is a function of land use

- The concept of derived demand is a critical point of departure in model development.

- Most of the models are developed on the assumption that tripmaking is related to types of land use at the origin and destination ends of the trip.

Trips are made for different Purpose:

Modeling is based on the type of activities and land use at the both ends of the trip. The modeling process often treats these trips separately and aggregates them at the end for estimation of total no. of trips in the network. Trips are made for accomplishing different activities.

Trips are made at different time at the day.

The basis of most of the modeling is to determine the origin and destination patterns in a study area. These patterns largely depend on time as different types of trip is made at different times of the day. Thus modeling is often done with origin-destination trip table to represent different trip period.

Travelers have different options available to them:

It is important not only to know the origin-destination pattern but the travelers have the option of trip making using different modes as well as different paths in the network.

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It is important not only to know the origin-destination pattern but the travelers have the option of trip making using different modes as well as different paths in the network.

Trips are made to minimize the level of inconvenience with the reaching a destination:

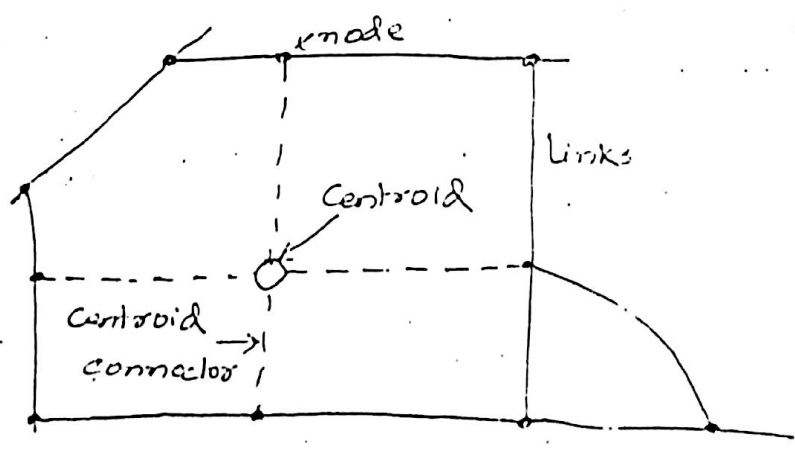
- The choice of modes depend on:-
 - ① trip time / travel time associated with reaching the destination
 - ② expense of the mode to reach destination

The choice of paths through a network is often based on the assumption that the minimum amount of time it takes to go to the destination.

Transport networks and traffic analysis zones are the of system modeling

Given numerous origins and destination in a study area and a given the different paths that can be used to reach destinations, transport models must be able to represent the land use pattern and network characteristics that are the fundamental of trip estimation.

The transport system often pictured as a network links and nodes. The conceptually represents the roads and intersection. The figure, given below represents the transport network



Steps of Transportation Modeling:

- Four steps :-
- (i) Trip generation :- no of trips produced in traffic analysis zone (T_i)
 - (ii) Trip Distribution :- no of trips generated @ zone i and attracted to zone j (T_{ij})
 - (iii) Mode split :- no of trip produced at zone i and attracted to zone j travelling by mode m (T_{ijm})
 - (iv) Trip assignment :- no. of trip produced in zone i and attracted to zone j travelling by mode m and by route n (T_{ijn})

* Trip generation:

Trip generation step in transportation modeling relates the number of trips that have been produced at zone by time period and to the land use and demographic characteristics of the location. A necessary step input into the trip generation is to have some identification of what the land use and demographic characteristics are likely to be. Special models are used to estimate population, number of dwelling units, auto ownership, income, employment and retirement zone. In many cases, specially in regional and stateside planning, these future estimates are provided by economic and demographic planners rather than transport professionals.

Studies have shown that the rate of trip making is closely related to three characteristics of land use :-

- (i) Intensity of land use.
- (ii) character of land use.
- (iii) location relative to major economic activities.

There are various methods to estimate the trip producing activities that are happened in a particular zone. Three of most common methods to estimate trip rate :-

- (i) Trip rates from National / local sources.
- (ii) Cross classification analysis.
- (iii) Regression analysis.

Trip rates from national / local sources:

planners often undertake studies to determine the number of trips that are made associating with the land use. For example, traffic counts at the driveways of store and restaurant to count the number of vehicles that are arriving to these place. The Institute of Transportation Engineers (ITE) publishes the trip generation Handbook - that compiles the trip rates that have been occurred throughout the USA. In absence of such local information, Handbook is often used as a source of information of trip rates.

Cross Classification analysis:

If good data are available to provide information on the relation between socioeconomic variables and trip making cross classification analysis can be used to develop relevant trip rates. Data about the population of a community, persons / household, the corresponding no. of trips can be known from census. By estimating the number of future households and applying the calculated trips, future number

of trips can be estimated in a zone.

Regression Analysis:

Given high correlations that typically varies and exists between trip rate and socioeconomic variables, many model use least-square regression to estimate trip production per zone. The ITE Handbook also provide regression equation to estimate no. of trip making based on certain types of land use.

→ Trip Distribution:

The major product of the trip distribution step is to provide a trip table, an origin-destination matrix that shows the no. of trips that are generated from a particular zone and where they are destined to. The major method of producing such trip table is the gravity model.

This gravity model is named because of its similarity with Newton's law of gravity. Employed first for sociological and marketing research but in 1950s the model is used to transportation modelling. Since then the model has been slightly modified and has become the predominant technique for transportation modeling.

Let total no of P_i trips are made from zone i and T_{ij} of them are destined to zone j for a specified purpose. The distance between zone i and j is D_{ij} . If A_j is a measure of attraction of zone j for this purpose and n is the exponent of that varies with trip purpose. The gravity

model can be written as :-

$$T_{ij} = \frac{\frac{A_j}{(D_{ij})^n}}{\sum_{j=1}^m \frac{A_j}{(D_{ij})^n}} P_i$$

The gravity model has been modified in recent years that reflects the research and experience with the model. It has been found that travel propensity decreases not only with travel length but travel-time to distance. It has been established that the exponent of travel time, n not only varies with travel purpose but also with trip length. Trip distribution analyses are therefore usually stratified according to the travel time t with calibrated value of exponent being determined for a given city and given trip. To facilitate efficient computer use of gravity model, it is now well practice to represent the effect of spatial separation in form of travel time factors,

$$F = \frac{c}{t^n}$$

Here c is a constant. Instead of surrogate measures of attractiveness such as commercial floor space, actual zonal trip attractions are used in the eqn. currently the recommended gravity model is :-

$$T_{ij} = \frac{A_j F_{ij} K_{ij}}{\sum_{\text{all zones}} A_j F_{+ij} K_{ij}} P_i$$

Where, F_{ij} = travel time factor for travel between zone i to

K_{ij} = Socio economic adjustment factor between i and j

A_j = total attraction at zone j

* Mode Split Model

Mode split model is to predict the number of travelers who choose one mode over another for making particular trip. A great deal of research have been undertaken to better specify models will correctly reflect the individual decision-making process making this choice. Empirical evidence indicates the following factors influence mode choice :-

(i) Type of trip.

(ii) characteristics of trip maker.

(iii) characteristics of transportation system.

The most recent advance in transportation modeling have on what are called individual choice models. which relate mode choice to the utility associated with each mode. The utility as attractiveness of the each mode can be defined as a variety variables relating to travel costs, travel time, reliability and Different mode formulation have been used to predict modal based on different modal utilities, but the most common formulation is called the logit model and is of following

$$P_{ti} = \frac{e^{U_{it}}}{\sum e^{U_{it}}}$$

P_{ti} = Probability of individual t choose mode i

U_{it} = utility of mode i to individual t .

Measures of effectiveness used in evaluation:

01. Transportation system performance:

- Number of trips by mode
- Vehicle miles traveled.
- Congestion
- Peak hour congestion.
- Transit boardings.
- Highway level of service

02. Mobility:

- mobility option
- Improved movement of people

03. Accessibility:

- percent within 30 minutes etc.
- Transit and highway speed.

01.

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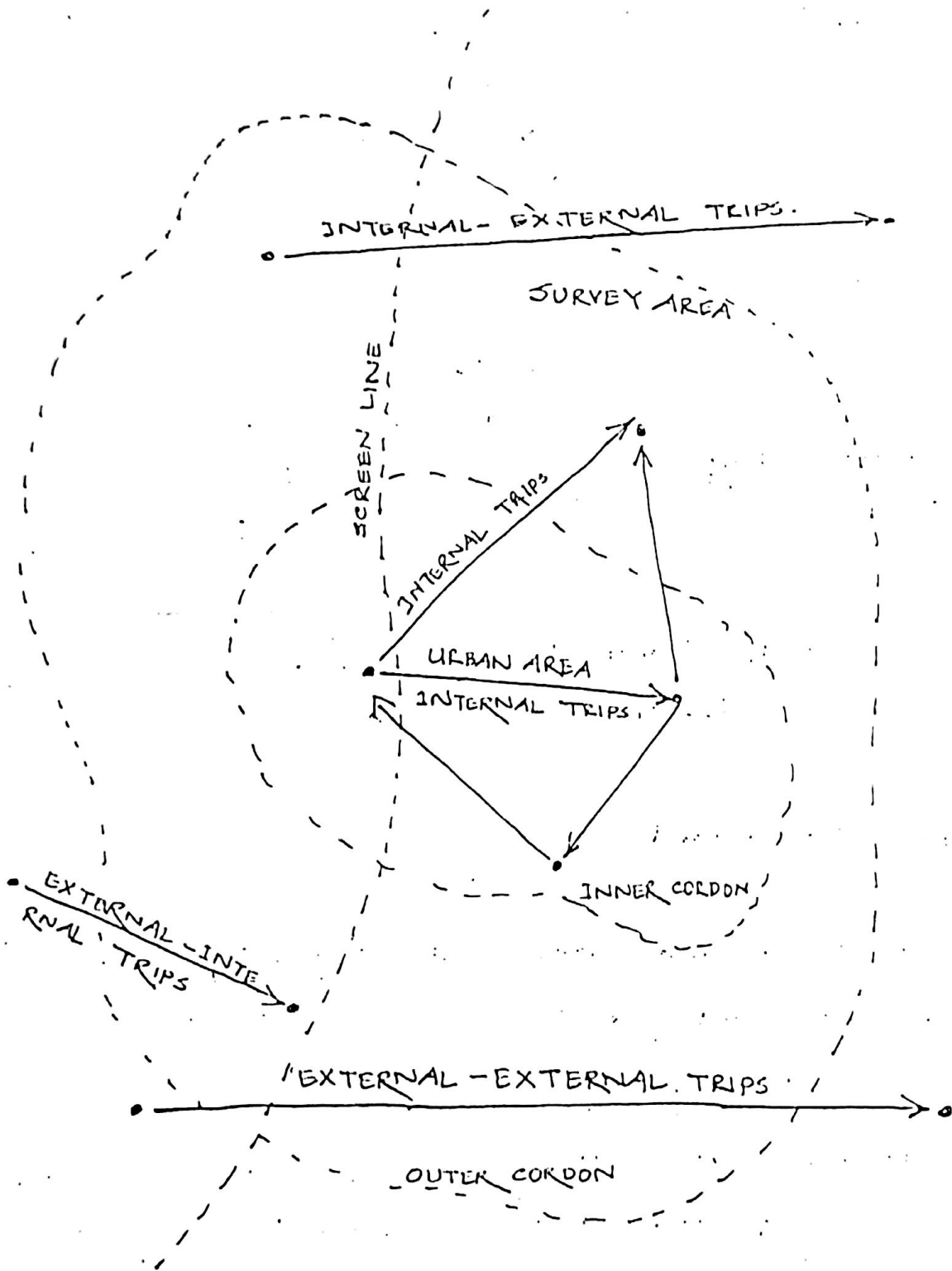
- mobility option
- Improved movement of people

03. Accessibility:

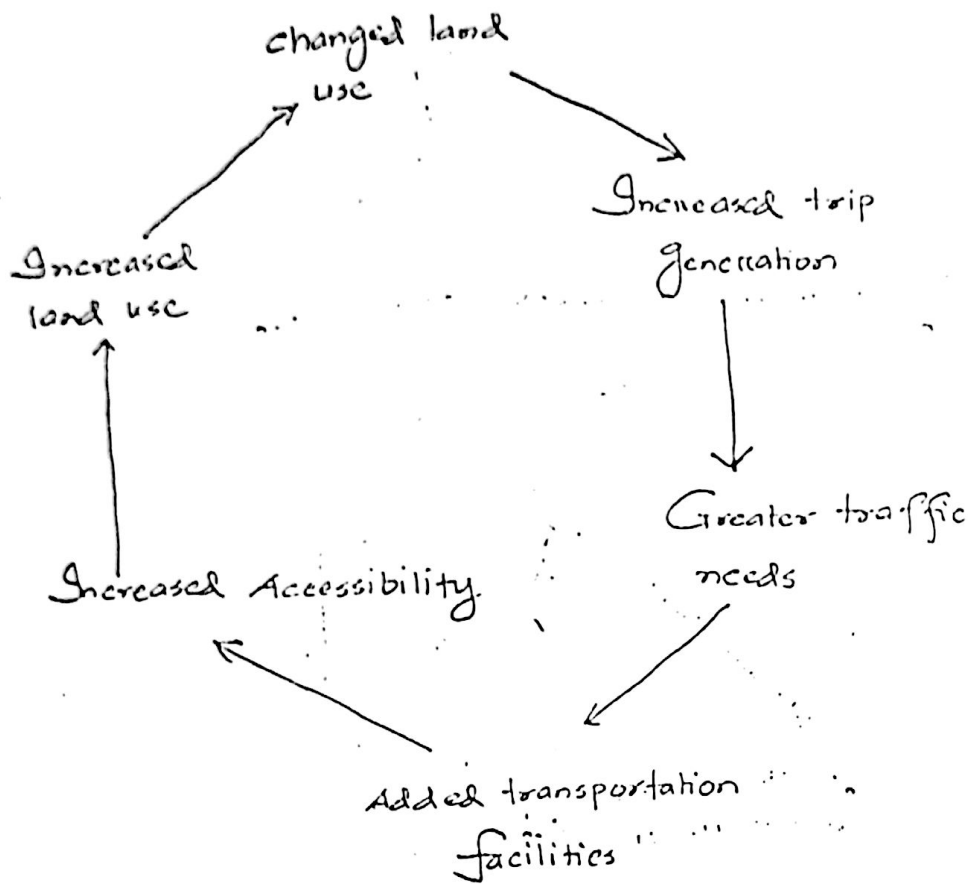
- percent within 30 minutes etc.
- Transit and highway speed.

04.

Ques 09-10: Explain diagrammatically the basic movement to categorize travel pattern in planning areas.



Ques - 10-11 = Explain land use - transportation cycle



Transportation - land use cycle

change in land use may improved the facilities in the area and attracted individual. So trip rate increase, eventually which requires greater number of traffics. So to change in land use may cause to change in transport facilities. Because of greater or improved in transport facilities greater no. of people makes to these destination which requires increasing land use and so do the facilities.

Ques 10-11:

50 Given, $U_k = a_k - 0.25x_1 - 0.032x_2 - 0.015x_3 - 0.002x_4$

Determine: the trips taken by bus and auto: by logit model

Soln According to logit model,

$$P_{it} = \frac{e^{U_{it}}}{\sum e^{U_{jt}}} = \frac{e^{U_{bas}}}{e^{U_{bas}} + e^{U_{auto}}} = P_{bas}$$

$$\text{and, } P_{auto} = \frac{e^{U_{auto}}}{e^{U_{bas}} + e^{U_{auto}}}$$

Now, $U_{auto} = -0.12 - 0.25 \times 5 - 0.032 \times 0 - 0.015 \times 20 - 0.002 \times 15$
 $= -1.87$

$$U_{bas} = -0.22 - 0.25 \times 10 - 0.032 \times 15 - 0.015 \times 10 - 0.002 \times 50$$
$$= -3.9$$

$$\text{So, } P_{bus} = \frac{e^{-3.9}}{e^{-3.9} + e^{-1.87}} = 0.116$$

$$P_{auto} = 0.884$$

No of trips taken by auto = 112

" " " " Bus = 58

Ans.

Ques - '09-10:

$T_i = 450$ work trips.

From Gravity model,

$$T_{ij} = \frac{A_j F_{+ij} K_{ij}}{\sum A_j F_{+ij} K_{ij}} P_i$$

Here, $F_{+ij} = \frac{c}{+ij^n}$.

$$\begin{aligned} \text{Now, } \sum A_j F_{+ij} K_{ij} &= \sum (A_j * K_{ij}) \frac{c}{+ij^n} \\ &= 750 \times \frac{c}{9^{0.6}} + 400 \times \frac{c}{5^{0.6}} + 300 \times \frac{c}{7^{0.6}} \\ &= 446.32c \end{aligned}$$

$$\text{So, } T_{i1} = \frac{200.69c}{446.32c} \times 450 = 202.34 \approx 202$$

$$T_{i2} = \frac{152.29c}{446.32c} \times 450 = 153.55 \approx 154$$

$$T_{i3} = \frac{93.34c}{446.32c} \times 450 = 94.1 \approx 94$$

Ans.

Question 08-09

(c) Trips no. that will increase in future

Trips/Household obtained from current trip making

House hold size		1	2	3+
Household sex - incom	Low	3.1	4.1	4.6
	Medium	3.7	4.5	4.9
	High	3.9	4.8	5.1

Forecasted no of trips in zone determined by multiplying the rate by no. of households

House hold size		1	2	3+
House hold Incom	Low	357	1127.5	1978
	medium	814	5499	11318.8
	High	351	576	1350

Total = 23871.3

≈ 23872

QWY

Ques 105-06

5(c) Given, no of trips = 33

Shopping Center	Floor space	distance
1	184 ft ² x 1000	8
2	215 ft ² x 1000	4
3	86000 ft ²	5

Trips from zone i to shopping center 1 = $\frac{184000}{8^2} + \frac{215000}{4^2} + \frac{86000}{5^2}$
 $= 1.3 \approx 5$ trips

" " " i to " " 2 = $\frac{215/4^2}{\frac{184}{8^2} + \frac{215}{4^2} + \frac{86}{5^2}}$
 $= 22.45 \approx 22$ trips

" " " i to " " 3 = $\frac{86/5^2}{\frac{184}{8^2} + \frac{215}{4^2} + \frac{86}{5^2}}$
 $= 5.75 \approx 6$ trips

GEOMETRIC DESIGN OF HIGHWAYS

* Design controls and criteria:

The elements of design are largely influenced by the design controls and engineering criteria and project specific objectives. Such factors include the following:-

- Traffic safety consideration.
- functional classification and of the roadway
- Projected traffic volume and composition.
- Required design speed.
- Multimodal needs of surrounding community.
- Topography of surrounding land.
- Capital costs for construction
- Agency funding mechanism.
- Human sensory capacities of roadway users.
- Vehicle size and performance characteristics
- Public involvement, review and comment-
- Environmental consideration
- Right-of-way impacts and costs.

Design speed : 109-110

- The engineer should select a design speed for assuring the users travel at desired speed for specific facility.
- Other factors influencing the selection of desired speed including volume of traffic and composition, costs of right-of-way, and aesthetical consideration.
- Design speed ranges from 15 to 75 mph. and intermediate are chosen in increments of 5 mph.

- where change in terrain or other conditions dictate a substantial length of highway to assure the consistency of roadway features.

- where change in terrain or other conditions dictate to change the speed, these change should be made over a sufficient distance to permit the driver to change speed gradually.

* Design Vehicle : '09-10

- The dimensions of motor vehicle that utilize the proposed facility influence the design of roadway project. The width of the vehicle naturally affects the width of the lane. The vehicle length has a bearing on roadway capacity and affects the turning radius. The height of the vehicle affects the clearance of the structure. The weight of the vehicle affects the structural design of roadway.

- vehicle performance has a considerable impact on design of the highway. Acceleration and deceleration rates are important parameter to calculate the several numerical values. An obvious consideration when designing a two-lane road is an evaluation of slow-down speed. Factors affecting the slow-down speed is the steepness of the roadway upgrade and the total length of the upgrade.

* Limited Access - Highway:

Defn:

A limited access highway may be defined as a highway street especially designed from through traffic where motorist and owners of abutting properties have only restricted right of access.

Benefits:

- Improved highway capacity.
- Improved highway speed.
- Improve safety.

Controlling of limited access:

- limiting the number of connection to and from highway
- facilitating the flow of traffic by separating cross traffic with overpass and under pass.
- Eliminating or restricting direct access of by owners of the abutting properties using frontage roads.

Consideration:

The design of limited access should provide adequate right-of-way, adequate landscaping, prohibition of outdoor advertisement, control of abutting service facilities.

* Pavement Crowns:

Purpose:

- for drainage purpose

Limits:

- present days high type pavement with good quality control of drainage $\frac{1}{8}$ " per foot.
- Low crowns are satisfactory when:-
 - (I) little or no settlement in the pavement is expected
 - (II) Drainage system is of sufficient capacity to remove the water from the traffic lane.
- when four or more lanes are used, it is desirable to use higher crown on the outer lane to expedite the flow of water from the pavement to the gutters.

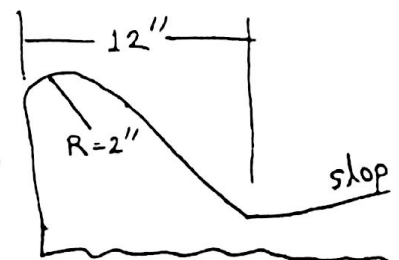
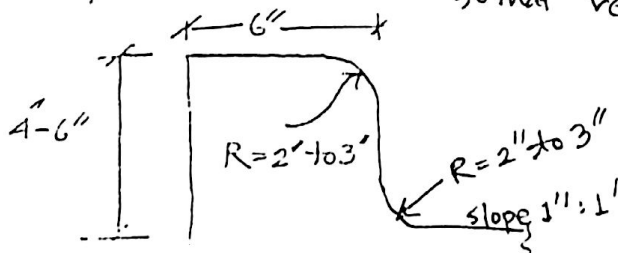
Curbs:

two major types:-

- (I) mountable curb.
- (II) vehicle barrier curb.

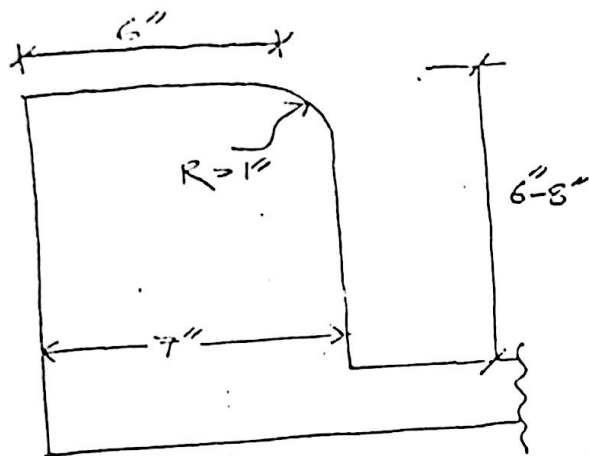
* Mountable curb:

- used when no sidewalks are provided
- Low in height
- constructed with a vehicle flatter angle
- face of the curb should not be steeper than 4° so that vehicle can mount them easily



Vertical Barrier Curb:

- curbs at parking and adjacent to side walk.
- Height - 6"-8"
- face of the curb nearly vertical.



- Factors affecting the height of the curb:

- height of the curb should be such that to open the doors of the passenger-cars with sufficient clearance and to facilitate car fenders and bumpers.

- storm water drainage and the to accommodate curb will also affect the shape and height of the curb.

- From a pedestrian's point of view the height should be limited to one step height.

07/09
Shoulders: 109-10-11

purpose:

- safe operation.
- develop full capacity of the road.
- ~~increase~~ increase the effective lane width as the drivers close to the edge of the roadway.
- Permit and encourage the traffic to leave travel lane while stopping.
- Greater the traffic volm, the greater is the likely to be used in emergency purpose.

limits:

<u>Highway type</u>	<u>desired value</u>	<u>limiting value</u>
(i) High speed highway	12'	10'
(ii) Low-type highway	6-8'	4'
(iii) In terrain when guard rails are required.	Additional 2'	—

* Guard rails:

Guard rails are provided :-

- where fills are about 8ft in height-
- sidewalks shoulder slopes are greater than 1V54H.
- location where there is a change in alignment.
- Where greater reduction in speed is necessary.
- locations with deep ditch, banks and other right-of-way limitation, it is necessary to provide steepness in side slope. and use guard rail.

Types of guard rail

- three types:-
- (I) W-Beam guard rail
 - (II) Cable guard rail
 - (III) Box beam guard rail.

* Right-of-way :

- The portion which is fully used.
- The portion which is required for us.

A wide section of right-of-way provides :-

- (I) Safer highway.
- (II) Gentle rounded slope
- (III) Low maintenance cost

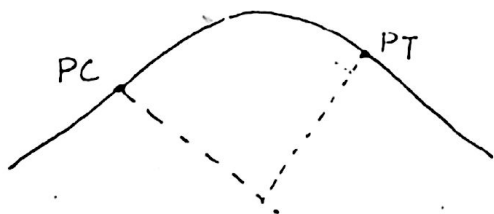
Sufficient right-of-way should be acquired to :-

- (I) avoid the expense of purchasing developed property
- (II) Removal of encroachment.

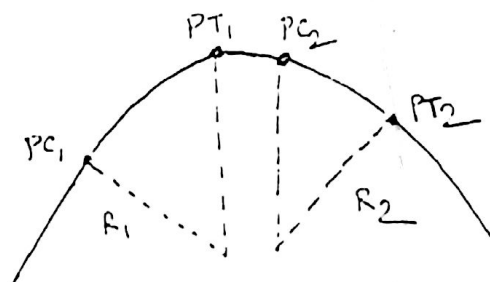
- Types of Super elevation Curves :

- four types :-
- (I) Simple Horizontal curve
 - (II) Broken back curve.
 - (III) compound curve

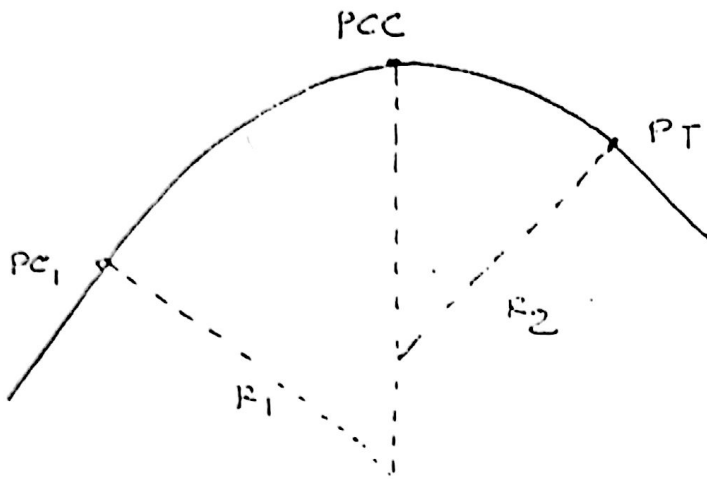
- (IV) Reverse curve :- (I) with tangent
Reverse (II) without tangent



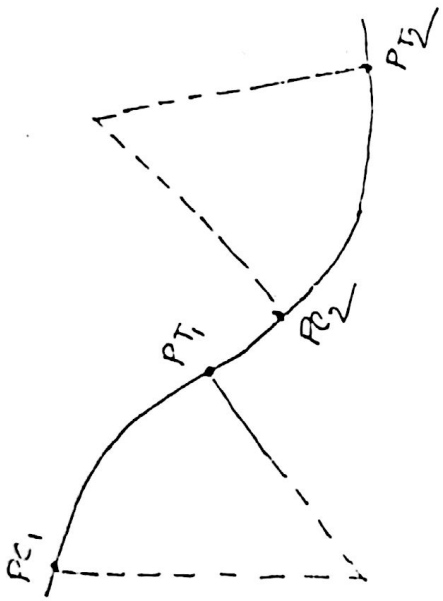
(a) Simple Horizontal Curve



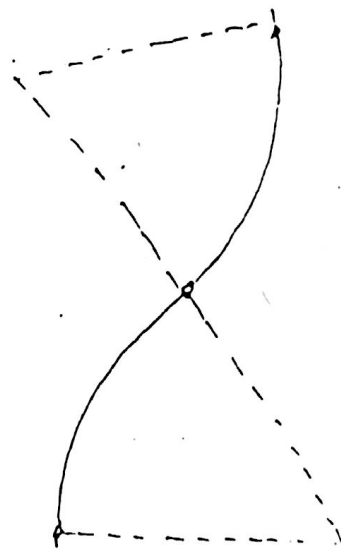
(b) Broken Back curve



(c) Compound curve.



(i) with tangent-



(ii) without tangent-

(d) Reverse curve

Importance of Super elevation:

- To reduce the centrifugal force
- To reduce the tendency of slip.
- To attain desired speed.

* Attainment of Super elevation ↗

The transition from tangent, normal crown section to a curve super elevation section must be accomplished without any appreciable reduction in speed and in such a manner to ensure the safety and comfort to the occupants of traveling the vehicle

In order to attain the change, the curved section should be tilted and banked as a whole to provide the super elevation at the required speed. Four methods are commonly used to attain super elevation transitions:-

01. The tilting usually is accomplished by rotating the cross section about the central line axis. The effect of this rotation is to lower the inside edge - at the same time to raise the outside edge without changing the central line grade.

02. Second method to rotate about the inner edge

03. Third method to rotate normal cross section about the outer edge.

04. Fourth method to rotate straight cross section about the outer edge.

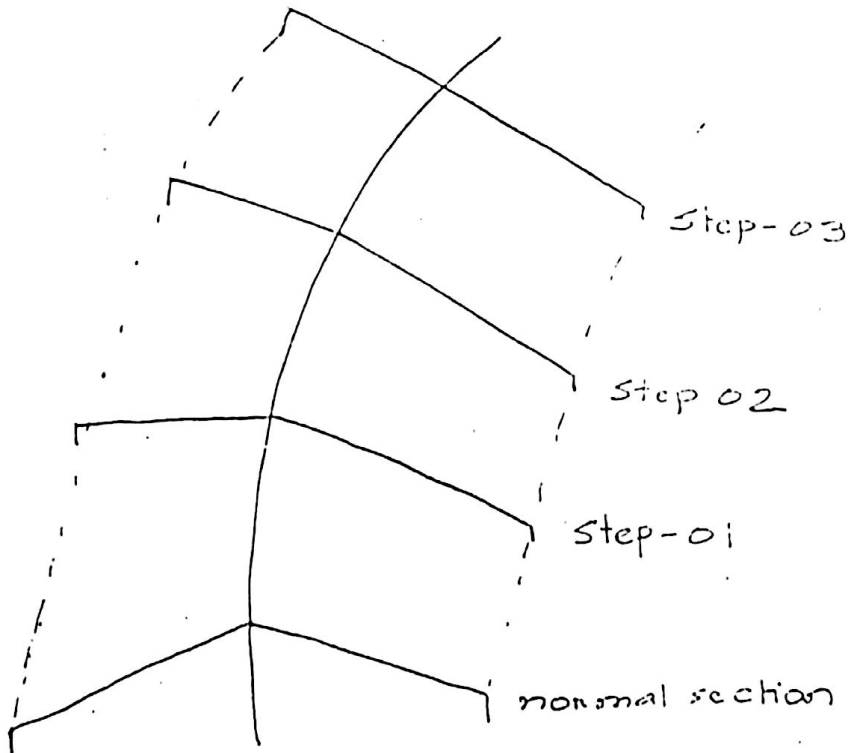
Process:-

The process to attain the super-elevation by first method given below:-

step 01: changing the section from CL to the outside edge

step 02: raise the outer edge to an amount half of desired super-elevation and at the same time, lower the inside edge and put the whole section in one inclined plane.

step 03: Continue the rotation about CL until the full super-elevation is reached.



100

* Diagrammatic representation of attaining super-elevation



* Widening of curves:

major reason:

As a vehicle turns at the curve the rear wheels follow the front wheel on shorter radius, and this effects increasing the width of the vehicle in relation to the lane width. Studies of drivers traversing curves have shown that the tendency to drive a curved path longer than the actual path curve, shifting the vehicles right at right-turning curves and left on left turning curves. Thus the tight turning curves the vehicle shifts toward inside edge on pvt. pavement. Thus the curves need widening. This widening are influenced by factors like:-

- (I) width of pvt on tangent
- (II) Speed of vehicle
- (III) Horizontal curve radius.

Minor Reasons:

Other factors affecting widening on curves:-

- To attain slip.
- more place is required at curve physically.

• rear wheel follow the front wheel on shorter radius.
• tendency to drive a curved path longer than the actual path curve.

• shift vehicle right at right turn
left at left turn

- width of pavement on tangent-
- speed of vehicle.
- Horizontal curve radius.
- to attain slip



- more place is reqd at curve physically.

* Sight Distance

Safe highways must be designed to give drivers a sufficient distance of clear vision ahead so that they can avoid hitting unexpected obstacles and passing the slower vehicle without danger.

Sight distance is length of highway that is visible ahead a vehicle of driver. In a roadway, three types of sight distance should be considered:-

01. Stopping sight Distance
02. Passing sight distance
03. Decision sight distance

Stopping Sight distance

Stopping sight distance is the minimum distance that a vehicle required to stop a vehicle travelling near the design speed before it reaches a stationary object in vehicle's path.

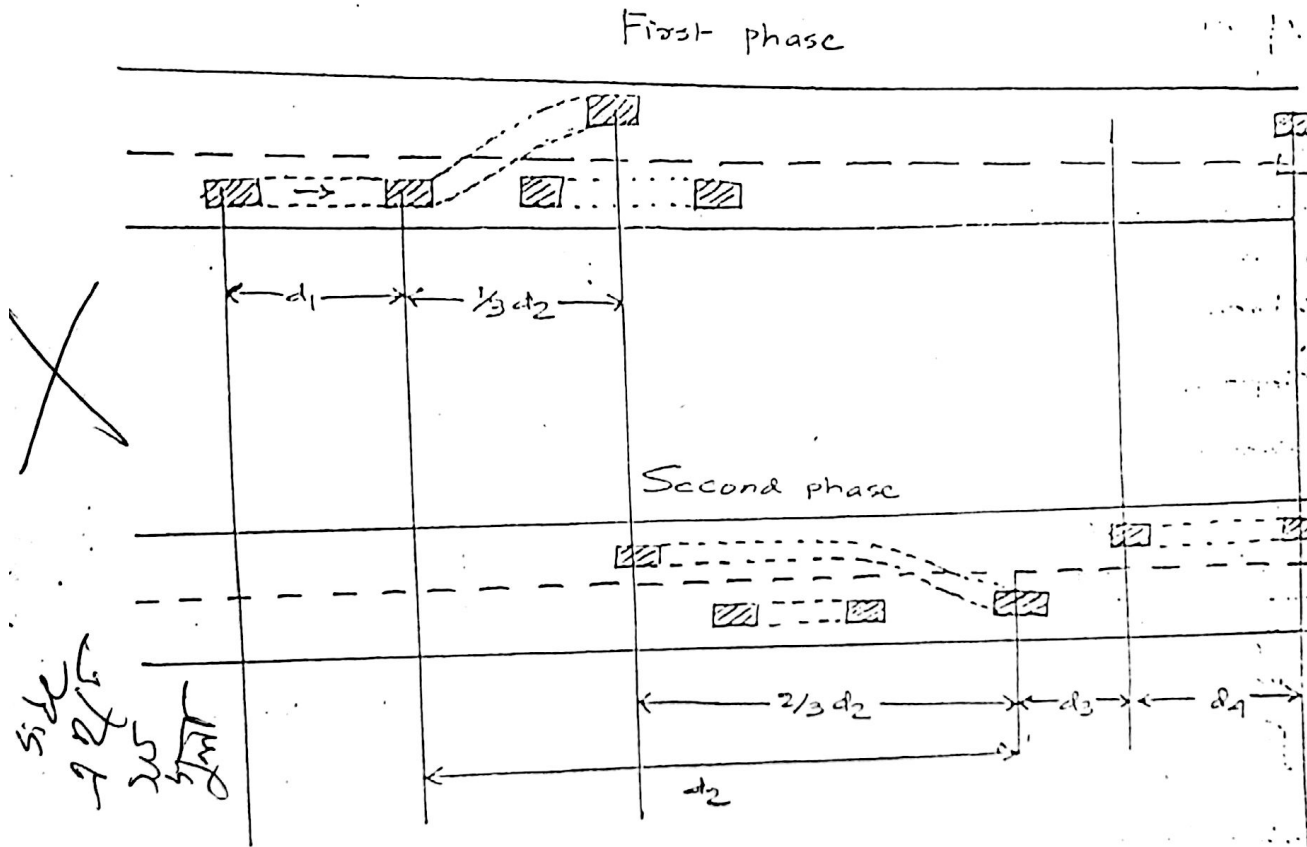
Passing Sight Distance:

When the sight is long enough to enable a vehicle to overtake and pass another vehicle in a two-lane highway without interference from opposite vehicles, this sight distance is known as passing sight distance.

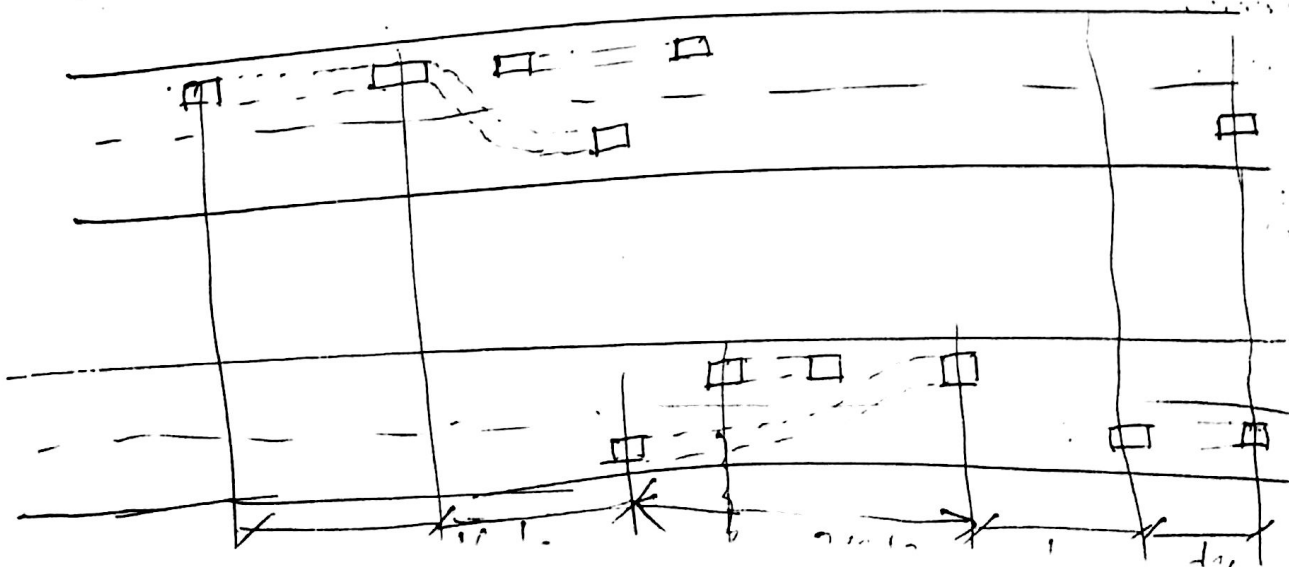
Decision Sight Distance

Sometimes the driver may encounter an unexpected road feature. The sight that is required for a driver to make decision and execute require maneuvers at complex location known as decision Sight Distance.

Passing Sight Distance



Minimum passing sight distance - distance traveled during perception and reaction time and during the initial acceleration to the point where the vehicle will turn into the opposite direction (d_1) + the distance traveled when the passing vehicle occupied the left lane + distance between the passing vehicle maneuver (d_3) + The distance traveled by the oncoming vehicle from opposite direction.



Mathematical Problem

6(c) Question ~~9-10~~ 10-11

Given, Avg. speed of the passing vehicle = 51 mph

Avg. speed of the passed vehicle = 41 mph

time of preliminary delay of passing vehicle = 4 sec

Avg. acceleration rate for passing vehicle = 1.43 mph/s

Time passing vehicle occupies the opposite lane = 10 sec

safe clearance distance = 180 ft

$$\begin{aligned} \text{Now, } d_1 &= 1.47 t_1 \left(v - m + \frac{at_1}{2} \right) \\ &= 1.47 \times 4 \left(51 - 10 + \frac{1.43 \times 4}{2} \right) \\ &= 257.8968 \text{ ft} \end{aligned}$$

$$m = (51 - 41) = 10 \text{ mph}$$

$$t_1 = 4 \text{ sec}$$

$$d_2 = 1.47 v_a t_2$$

$$= 1.47 \times 51 \times 10 = 749.7 \text{ ft}$$

$$d_3 = \text{safe clearance distance}$$

$$= 180 \text{ ft}$$

$$d_4 = \frac{2}{3} d_2 = \frac{2}{3} \times 749.7 = 499.8 \text{ ft}$$

$$\begin{aligned} \text{min}^m \text{ passing distance} &= (257.896 + 749.7 + 180 + 499.8) \text{ ft} \\ &= 1687.396 \text{ ft} \end{aligned}$$

Ques 107-08 7(c)

Given, v = velocity of vehicle in mph when brake is applied = 65 mph

g = percentage of grade = $\frac{g}{100}$

f = co-efficient of friction betⁿ tires and pavt = 0.29

$$\text{So, } d = \frac{v^2}{30(f+g)} = \frac{65^2}{30(0.29+0.03)} = 110.1 < 500'$$

Driver will be able to stop the car before hitting the obstruction.

see sheet