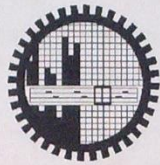


**Bangladesh University of Engineering
& Technology**

Department of Water Resources Engineering



Course No. :WRE 302

Course Title: Open-Channel Hydraulics Sessional

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Level: 3 ;Term: I

Hydraulics & River Engineering Laboratory.

Objective:

- To determine the total discharge and mean velocity of the flow.
- To measure Mannings 'n' and Chezy 'c'
- To plot 'n' and 'c' against depth of flow and observe the relationship.

Experimental Setup:

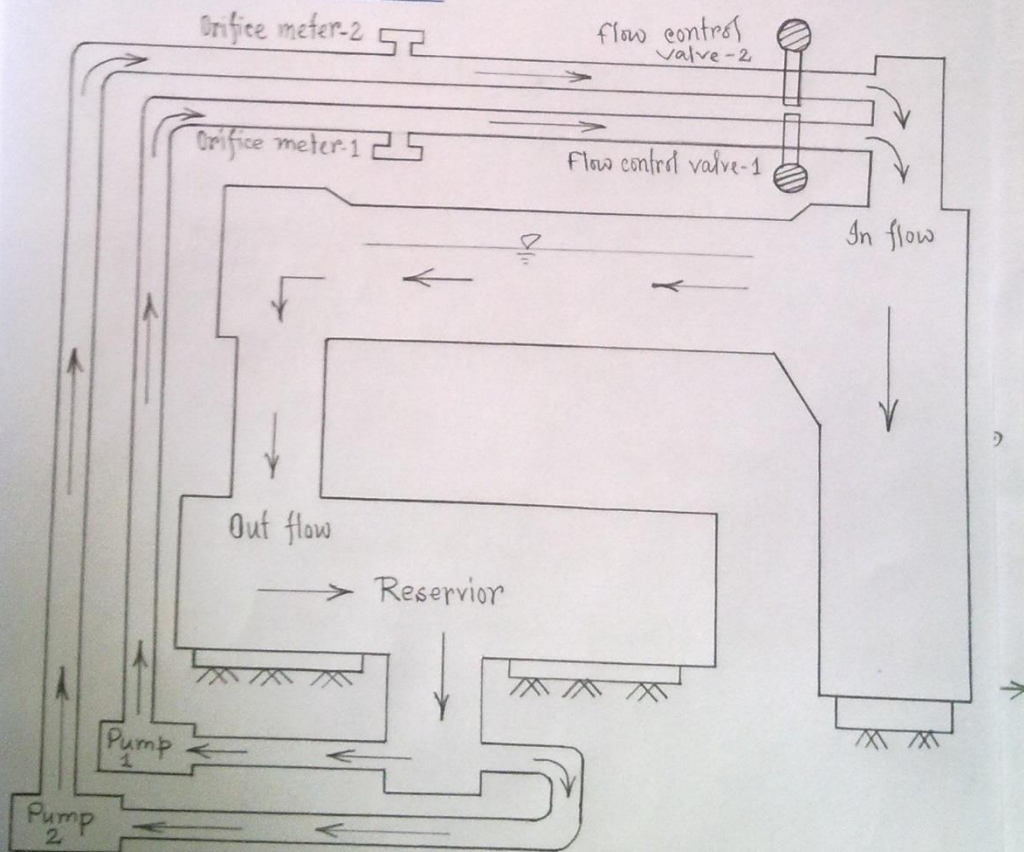


Figure: Setup for determination of discharge and mean velocity of an open channel.

Related formula:

* Depth mean velocity:

The velocity along a vertical varies from zero at the stream bed to maximum at or near the water surface. Normally when the depth of flow in a vertical is greater than 0.61 m, then the depth mean velocity is determined by averaging velocity at 0.2 and 0.8 depth otherwise at 0.6 depth taken as average velocity. So,

$$\text{Depth mean velocity, } V = \frac{v_{0.2} + v_{0.8}}{2} \text{ when, } y > 0.61 \text{ m}$$
$$= v_{0.6} ; \text{ when, } y \leq 0.61 \text{ m}$$

* Discharge of flow:

discharge through a strip is given by,

$$Q_i = v_i \Delta A$$

Where, v_i = mean velocity at i^{th} strip.

ΔA = area of the strip.

The sum of the discharges through all the strips is the total discharge i.e. \rightarrow

$$Q = \sum Q_i$$

* Mean velocity of the stream:

The mean velocity of the whole section is equal to the total discharge divided by the whole area and given by,

$$V = \frac{Q}{\sum \Delta A}$$

* Manning's and Chezy's roughness coefficient:

Manning's 'n' can be determined by the following equation given as,

$$n = \frac{(x-1) R^{1/6}}{6.78(x+0.95)}$$

where, $x = \frac{V_{0.2}}{V_{0.8}}$

R = hydraulic radius.

Using the above value of 'n' Chezy's c is determined by the following relation,

$$c = \frac{1}{n} R^{1/6}$$

Measurement total discharge

Flume Width,
m

$$B = 0.762 \text{ m} \quad \text{Width of each strip, } = B/3 = 0.254$$

Total depth of flow,
m

$$Y = 0.385 \text{ m} \quad \text{Area of each strip, } \Delta A = (B/3) \cdot Y = 0.0978 \text{ m}^2$$

Current meter constants,

$$a = 0.1334 \quad b = 0.029$$

Location of Current meter		Current meter reading			Point velocity v (m/s)	Depth mean velocity V_i (m/s)	Discharge through the strip $Q = \Delta A \cdot V_i$ (m ³ /s)	Total discharge $Q = \sum Q_i$ (m ³ /s)
Horizontal	Vertical	No of revolution N (rev)	Time of observation t (sec)	Revolution per second n (rev/sec)				
At middle of first strip	At 0.2 Y	83	14.7	5.646	0.782	0.800	0.078	
	At 0.6 Y	86	14.9	5.780	0.800			
	At 0.8 Y	82	14.9	5.503	0.763			
At middle of second strip	At 0.2 Y	83	14.7	5.646	0.782	0.803	0.079	0.235
	At 0.6 Y	87	15.6	5.800	0.803			
	At 0.8 Y	83	15.1	5.495	0.762			
At middle of third strip	At 0.2 Y	83	14.7	5.646	0.782	0.800	0.078	
	At 0.6 Y	86	14.9	5.780	0.800			
	At 0.8 Y	83	14.9	5.570	0.772			

$$\text{So, mean velocity of the stream} = v = \frac{Q}{\sum \Delta A} = \frac{0.235}{0.29337} = 0.801 \text{ m/s}$$

~~At mid 15.03~~

Calculation of n and C

Strip	Point velocity	x	Average x	Manning's n	Chezy's C
1	$V_{0.2}$	1.025	1.0213	1.21×10^{-3}	627.48
	$V_{0.8}$				
2	$V_{0.2}$	1.026	1.0213	1.21×10^{-3}	627.48
	$V_{0.8}$				
3	$V_{0.2}$	1.013	1.0213	1.21×10^{-3}	627.48
	$V_{0.8}$				

Plotting n and C vs depth of flow:

Depth of flow	Manning's n	Chezy's C
0.254	2.0×10^{-3}	375.053
0.34	3.0×10^{-3}	271.2
0.325	4.5×10^{-3}	166.07
0.385	1.21×10^{-3}	627.48
0.365	4.6×10^{-3}	164.25

Sample Calculation:

$$\text{Flume width, } B = 0.762 \text{ m}$$

$$\text{Width of each strip, } = B/3 = \frac{0.762}{3} = 0.254 \text{ m}$$

$$\text{Total depth of flow, } Y = 0.385 \text{ m}$$

$$\begin{aligned} \therefore \text{Area of each strip, } \Delta A &= (B/3) * Y \\ &= 0.254 * 0.385 \\ &= 0.0978 \text{ m}^2 \end{aligned}$$

$$\begin{aligned} \text{Current meter constants, } a &= 0.1334 \\ b &= 0.029 \end{aligned}$$

Calculation at middle of second strip (at $0.6Y$):

$$\text{No. of revolution, } N = 87$$

$$\text{Time elapsed, } t = 15.0 \text{ sec.}$$

$$\begin{aligned} \therefore \text{Revolution per second, } n &= \frac{N}{t} \\ &= \frac{87}{15} = 5.8 \text{ rev. sec}^{-1} \end{aligned}$$

$$\begin{aligned} \therefore \text{Point velocity, } V_{0.6} &= an + b \\ &= 0.1334 * 5.8 + 0.029 \\ &= 0.803 \text{ m/s} \end{aligned}$$

$$\begin{aligned} \text{Depth mean velocity, } V_i &= V_{0.6} \\ &= 0.803 \text{ m/s} \end{aligned}$$

Discharge through the middle strip, Q_i

$$= \Delta A * V_i$$

$$= 0.0978 * 0.803$$

$$= 0.079 \text{ m}^3/\text{sec.}$$

Total discharge, $Q = \sum Q_i$

$$= 0.078 + 0.079 + 0.078$$

$$= 0.235 \text{ m}^3/\text{sec.}$$

\therefore Mean velocity of the stream, $V = \frac{Q}{\sum \Delta A}$

$$= \frac{0.235}{0.762 * 0.385} = 0.801 \text{ m s}^{-1}$$

Verification of mean velocity with meter reading:

Total discharge, $Q = (320 + 457) \text{ m}^3/\text{hr}$

$$= 777 \text{ m}^3/\text{hr}$$

$$= 0.2158 \text{ m}^3/\text{sec.}$$

\therefore Mean velocity of the stream, $V = \frac{Q}{A}$

$$= \frac{0.2158}{0.29337}$$

$$= 0.736 \text{ m s}^{-1}$$

Hydraulic radius, $R = \frac{A}{T} = \frac{0.762 * 0.385}{0.762 + 2 * 0.385}$

$$= 0.1915 \text{ m}$$

Calculation of 'n' and 'c' :

for first strip - $v_{0.2} = 0.782 \text{ m/s}$

$$v_{0.8} = 0.763 \text{ m/s}$$

\therefore ratio of $v_{0.2}$ to $v_{0.8}$, $\chi = \frac{v_{0.2}}{v_{0.8}}$

$$= \frac{0.782}{0.763}$$

$$= 1.025$$

Average value of $\chi = \frac{1.025 + 1.026 + 1.013}{3}$

$$= 1.0213$$

\therefore Manning's 'n' = $\frac{(\chi - 1) R^{1/6}}{6.78(\chi + 0.95)}$

$$= \frac{(1.0213 - 1) * (0.1915)^{1/6}}{6.78(1.0213 + 0.95)}$$

$$= 1.21 \times 10^{-3}$$

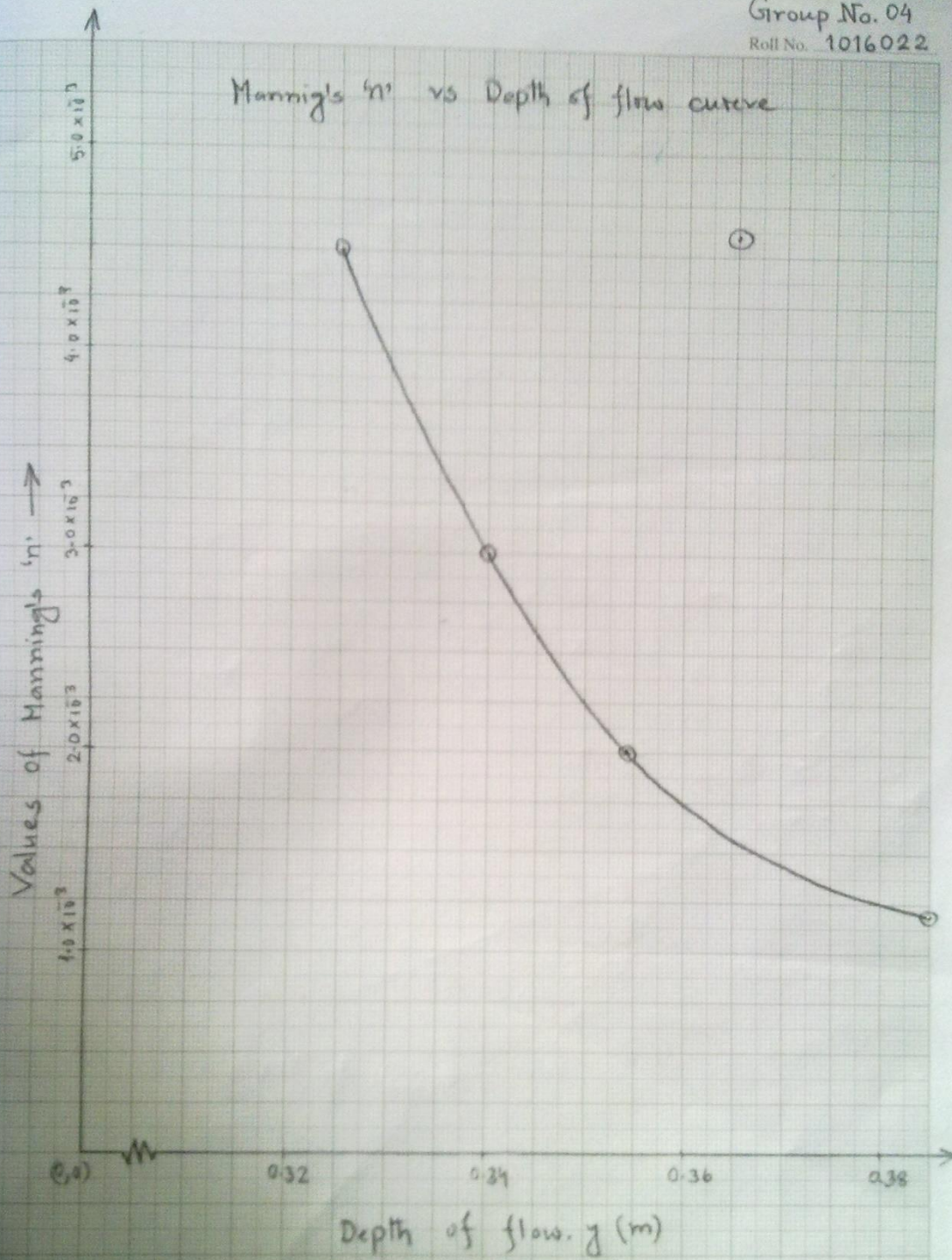
\therefore Chezy's c = $\frac{1}{n} R^{1/6}$

$$= \frac{1}{1.21 \times 10^{-3}} * (0.1915)^{1/6}$$

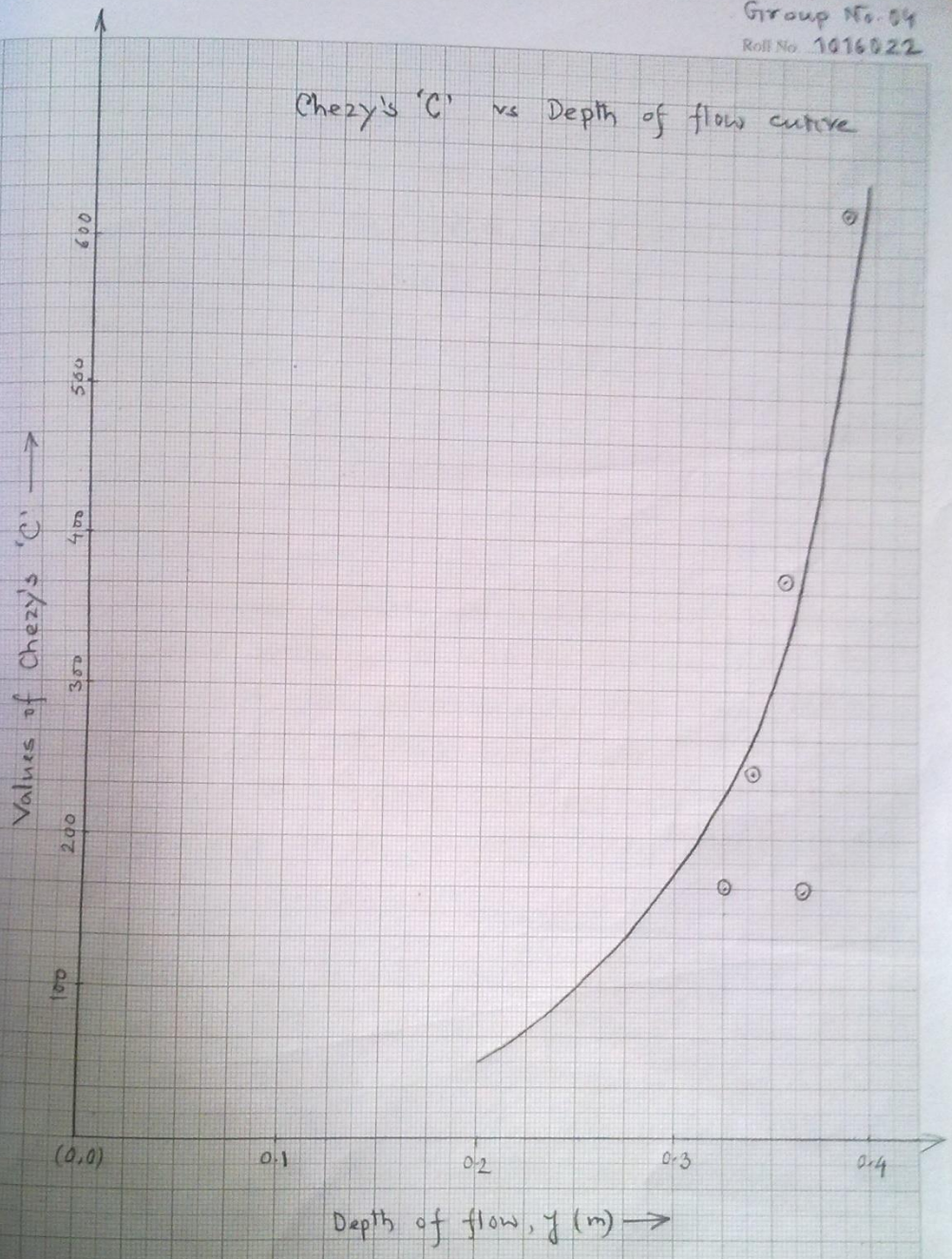
$$= 627.48$$

→

Manning's n vs Depth of flow curve



Chezy's 'C' vs Depth of flow curve



Result :

- Total discharge, $Q = 0.235 \text{ m}^3/\text{sec}$
- Mean velocity of the stream, $V = 0.801 \text{ m/s}$
- Manning's 'n' = 1.21×10^{-3}
- Chezy's 'C' = 627.48

Discussion :

- In open channel, velocity distribution along a vertical do not found uniform as velocity change along the depth increment. The point velocity was measured at 0.2y, 0.6y and 0.8y. As the total depth of flow was 0.385m (less than 0.61m) point velocity at 0.6y depth was considered to be as depth mean velocity of that strip.

- Total discharge is measured $0.235 \text{ m}^3/\text{sec}$ using area-velocity method. Again, meter reading was found $0.216 \text{ m}^3/\text{sec}$ as value of discharge. Therefore, applying this area-velocity method, the computed value of discharge is too close with the actual value. →

- Mean velocity of the laboratory flume for the given discharge is measured to be 0.801 m/s whereas using the meter reading of actual discharge mean velocity is computed to be 0.735 m/s .

- The value of Manning's 'n' is measured as 1.21×10^{-3} whereas the typical value of Manning's 'n' for open channel having bed surface to be concrete is measured 0.013 .

- The value of Chezy's C is found 627.48 . But the numerical value of Chezy's C varies from 30 to $80 \text{ m}^{1/2}/\text{sec}$.

- Manning's 'n' vs depth of flow curve is convex to the origin. It represents the value of 'n' decrease with the depth of flow increases.

- Chezy's C vs depth of flow curve is concave to the origin. With the increase of depth of flow the value of the depth C is also increase. Comparing these two curve these found an inverse relationship between Manning's 'n' and Chezy's C. →