

Taufiq Sir

- 1) 14 Feb
- 2) 28 Feb
- 3) 7 Mar
- 4) 21 Mar
- 5) 28 Mar
- 6) 25 Apr
- 7) 27 Apr
- 8) 2 May
- 9) 9 May
- 10) 11 May

Zakaria Sir

- 1) 23 Feb
- 2) 1 Mar
- 3) 2 Mar
- 4) 8 Mar
- 5) 15 Mar
- 6) 16 Mar
- 7) 18 Mar
- 8) 22 Mar
- 9) 29 Mar
- 10) 11 Apr
- 11) 12 Apr
- 12) 26 Apr
- 13) 4 May
- 14) 10 May
- 15) 16 May

Rouf Sir

- 1) 25 Feb
- 2) 4 Mar
- 3) 11 Mar
- 4) 25 Mar
- 5) 1 Apr
- 6) 22 Apr
- 7) 29 Apr
- 8) 6 May
- 9) 13 May

Taufiq Sir

11) 23 May

12) 30 May

Zakaria Sir

16) 18 May

Rouf Sir

10) 20 May

11) 27 May

12) 30 May

23 February 2015

Zakaria Sir-1

Statically Determinate Suspended Bridge

→ elastic theory of suspension bridge

✓ Dead Loads are entirely carried by cable

“ “ come from roadways

“ “ are assumed to be uniform per horizontal foot of span

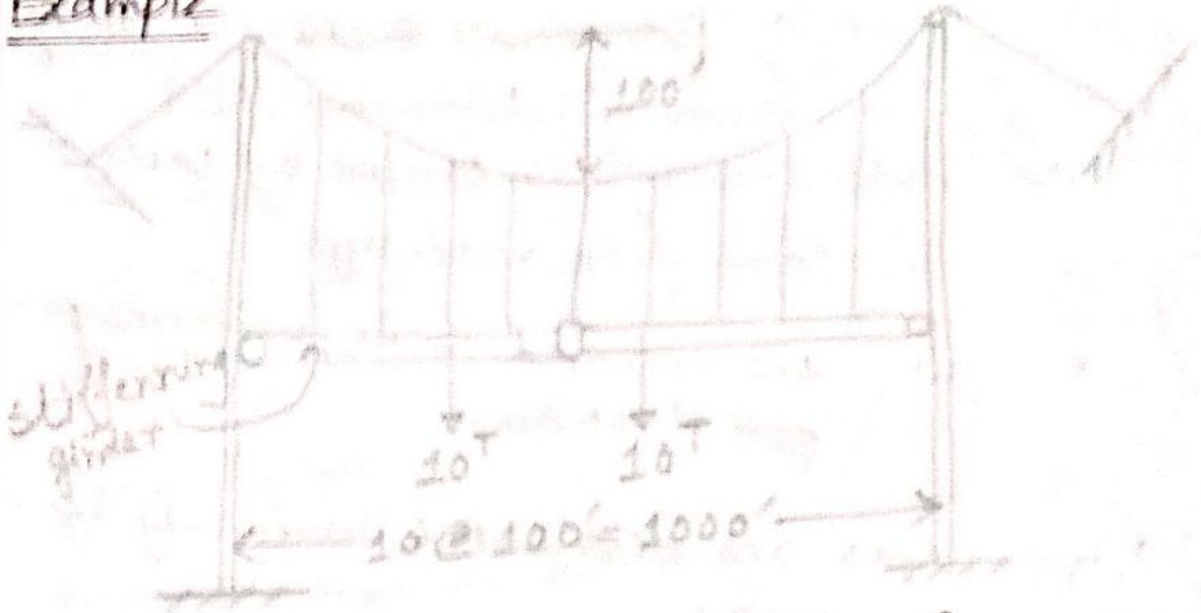
✓ Live Loads are distributed uniformly by stiffening Truss or Girders

✓ Total load is uniform per horizontal foot of this span

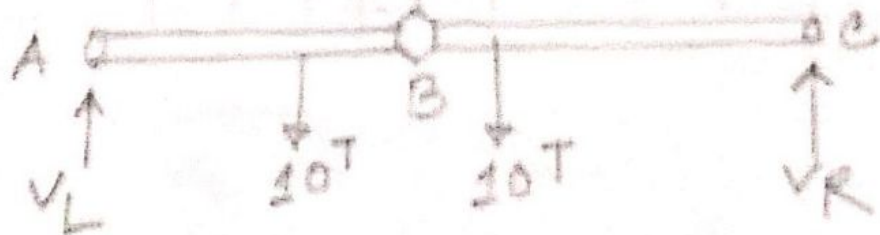
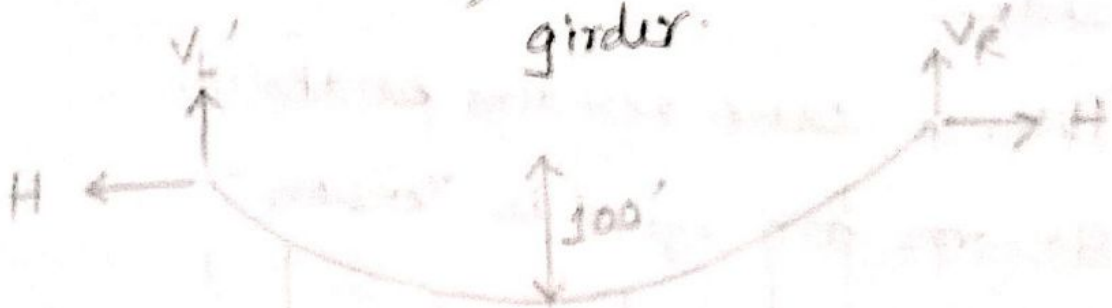
∴ Hence cable remains parabolic.

Hangers are equal in Tension

Example



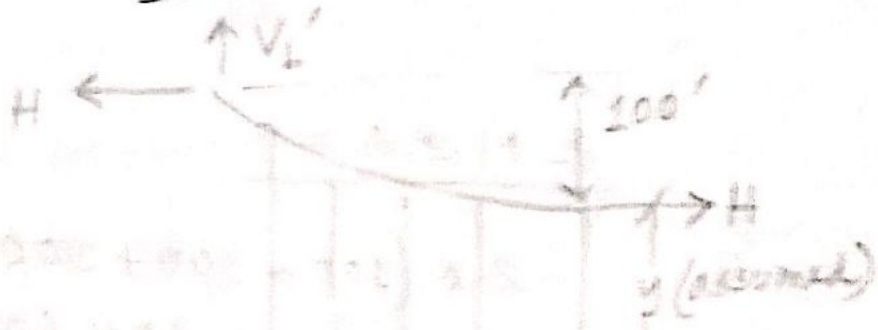
- 1) Find hanger force
- 2) Draw SFD & BMD of the girder.



$$\underline{\Sigma M_{ec} = 0 \quad (+ve \curvearrowright)}$$

$$(V_L + V_L') \times 1000 - 10 \times 700 - 10 \times 400 = 0$$

$$\therefore V_L + V_L' = \frac{11000}{1000} = 11^T$$



$$\underline{\Sigma M@B = 0 \quad (+ve \curvearrowright)}$$

$$(V_L + V_L') \times 500 - H(100 + y) + Hy - 10 \times 200 = 0$$

$$\Rightarrow 11 \times 500 - 100H$$

$$-Hy + Hy - 2000 = 0$$

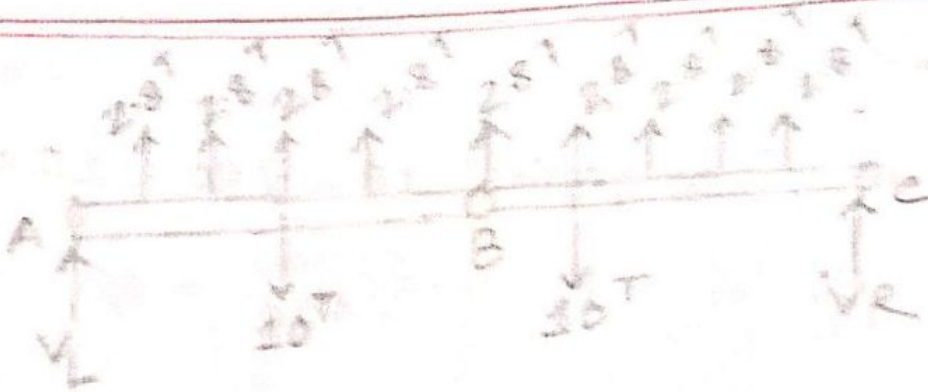
$$\therefore H = \frac{5500 - 2000}{100} = 35^T$$

From G.C.T, $H = \frac{\omega L^2}{8h}$

$$\Rightarrow \omega = \frac{8Hh}{L^2} = \frac{8 \times 35 \times 100}{(1000)^2}$$

$$= 0.028 \text{ ton/foot}$$

$$\begin{aligned} \text{Hanger Force} &= \omega \times \text{spacing of hanger} \\ &= 0.028 \times 100 \\ &= 2.8^T \end{aligned}$$



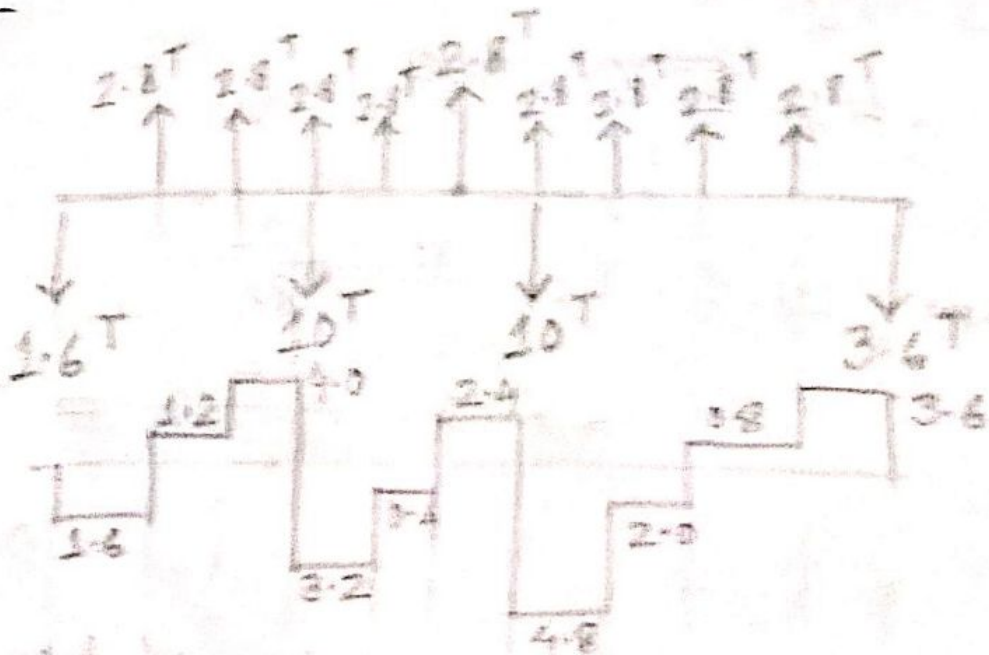
$$\underline{\Sigma M @ A = 0}$$

$$-2.8 (100 + 200 + 300 + \dots + 900) + 10 \times 300 + 10 \times 600 - V_R \times 1000 = 0$$

$$\Rightarrow V_R = -3.6 \text{ T } (\downarrow)$$

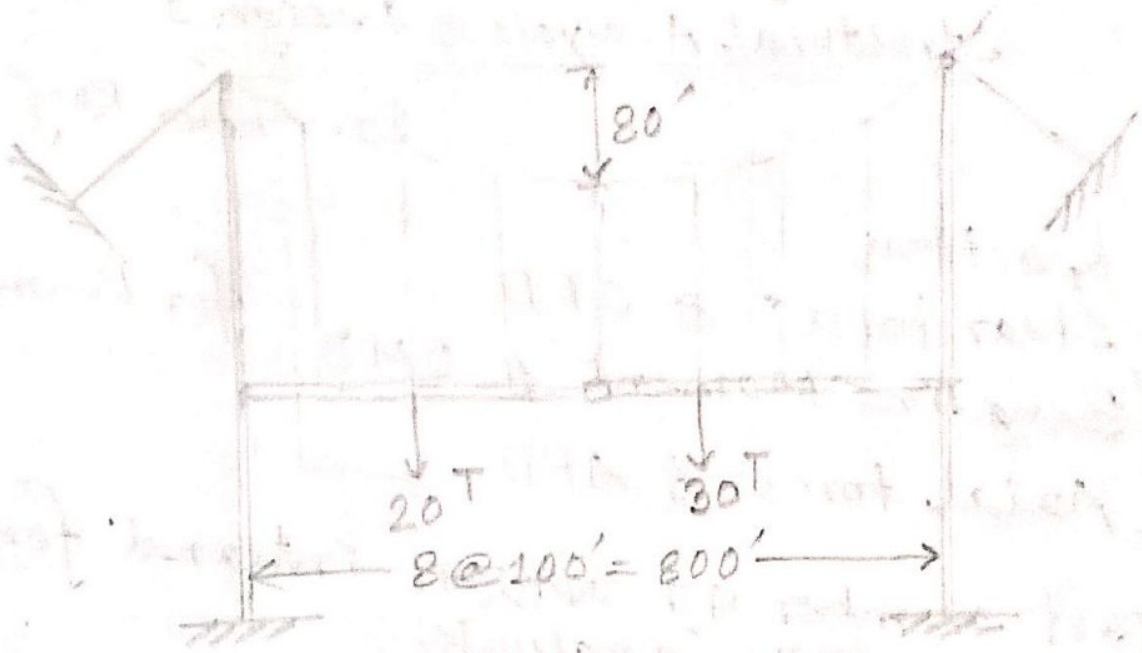
$$\underline{\Sigma F_y = 0} \Rightarrow (2.8 \times 9) - 3.6 - (10 \times 2) + V_L = 0$$

$$\Rightarrow V_L = -1.6 \text{ T } (\downarrow)$$



SFD

BMD



- (a) Hanger Force
- (b) Max^m cable Tension
- (c) SFD for girder
- (d) BMD for girder

Principle of Virtual Work

The external virtual work by a system of virtual Q forces acting on deformation by real force is equal to the internal virtual work by the internal virtual Q stresses on actual deformation.

$$W_s = W_d$$

Application - (1) to calculate deflection
(2) " " rotation etc.

$P \rightarrow$ real or Actual force system
 $Q \rightarrow$ virtual force system

deformation by actual force

$$W_s = \sum Q \delta$$

$$W_d = \int_0^L F_Q \epsilon_0 ds + \int_0^L \frac{M_Q M_P}{EI} ds$$

\uparrow axial force effect \uparrow bending effect

For truss member (member with axial force only)

$$\epsilon_0 = \frac{F_P}{AE} + \alpha_t t$$

\leftarrow member axial force \leftarrow coefficient of thermal expansion

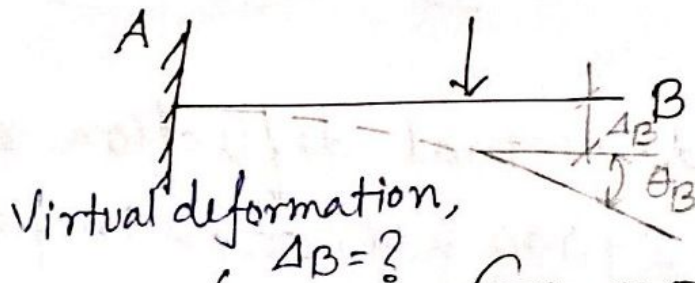
$$\left[\Delta L = \frac{PL}{AE} = \int \frac{F_P}{AE} ds \right]$$

For Truss Structures

$$W_d = \sum \frac{F_Q F_P L}{AE} + \sum F_Q \alpha_t L$$

1. point \rightarrow deflection
 2. point \rightarrow
 virtual force apply
 3.

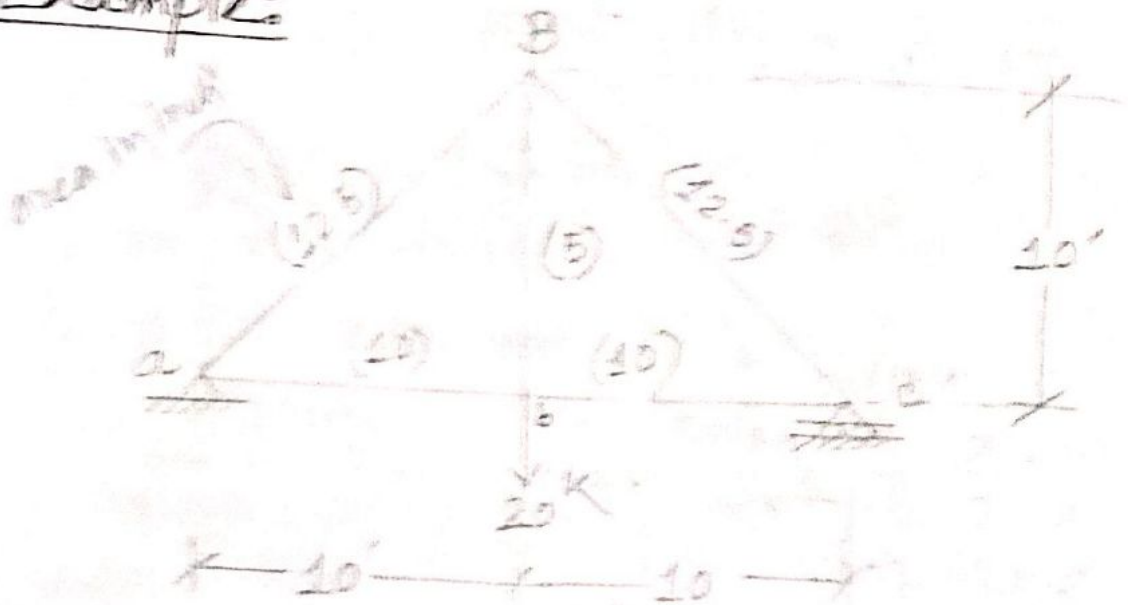
$Q = \text{unit force}$
 1. method
 unit force method - 3
 2.



Deflection \rightarrow 1 unit force
 Rotation \rightarrow 1 unit moment

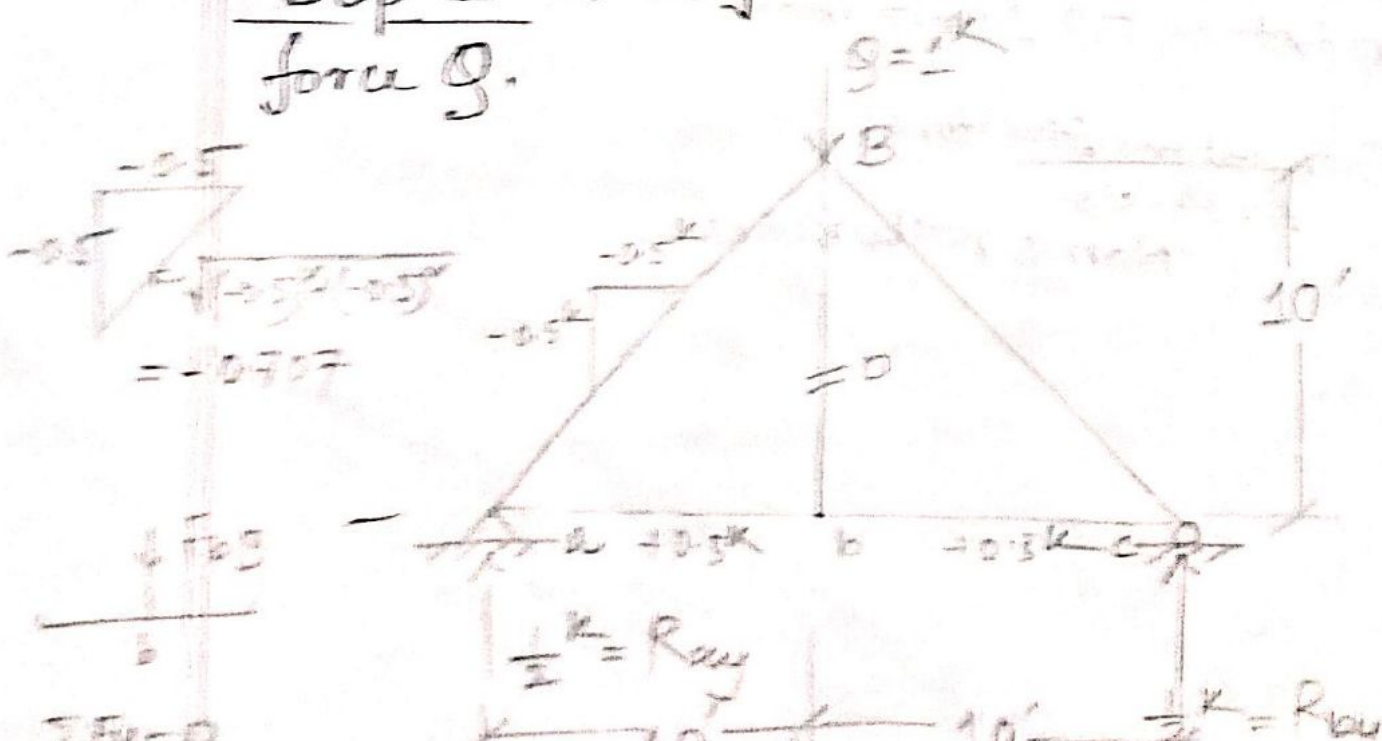
Elementary Structural Analysis { Solve: all problems in the book (examples)
 - Norris, Wilbur, Utku

Example:



Find virtual deflection at B. Given, $E = 29000 \text{ ksi}$. Area in parentheses (inch^2)

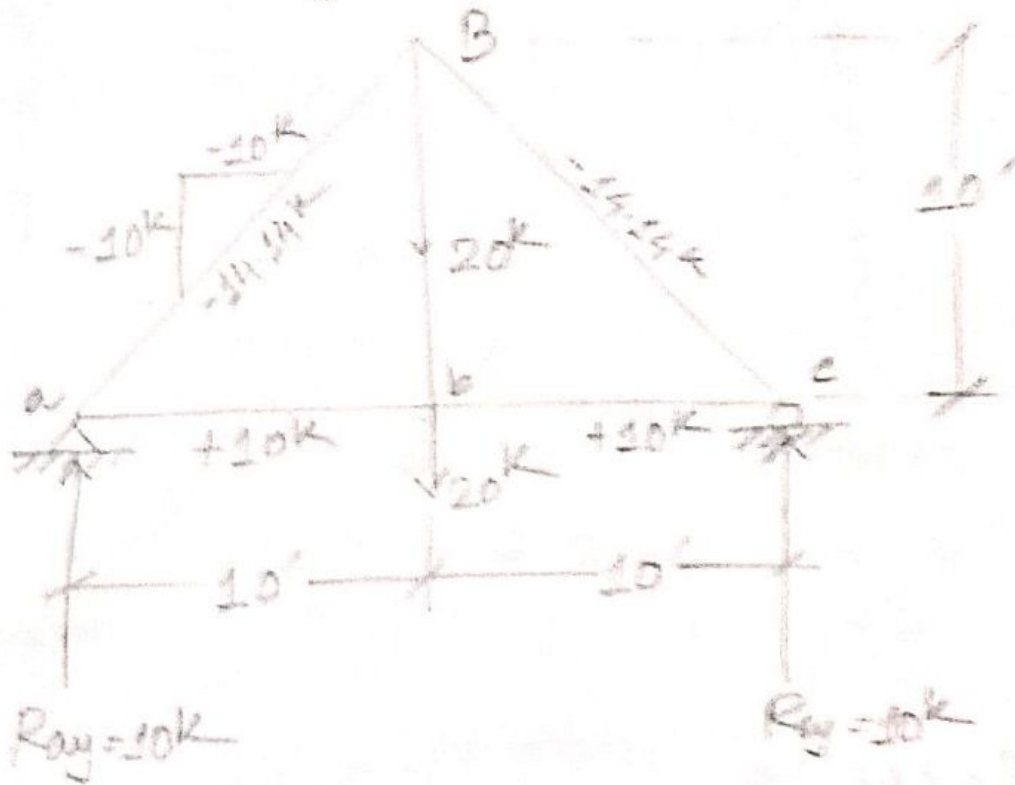
Step-1: Analyze structure for Virtual force Q .



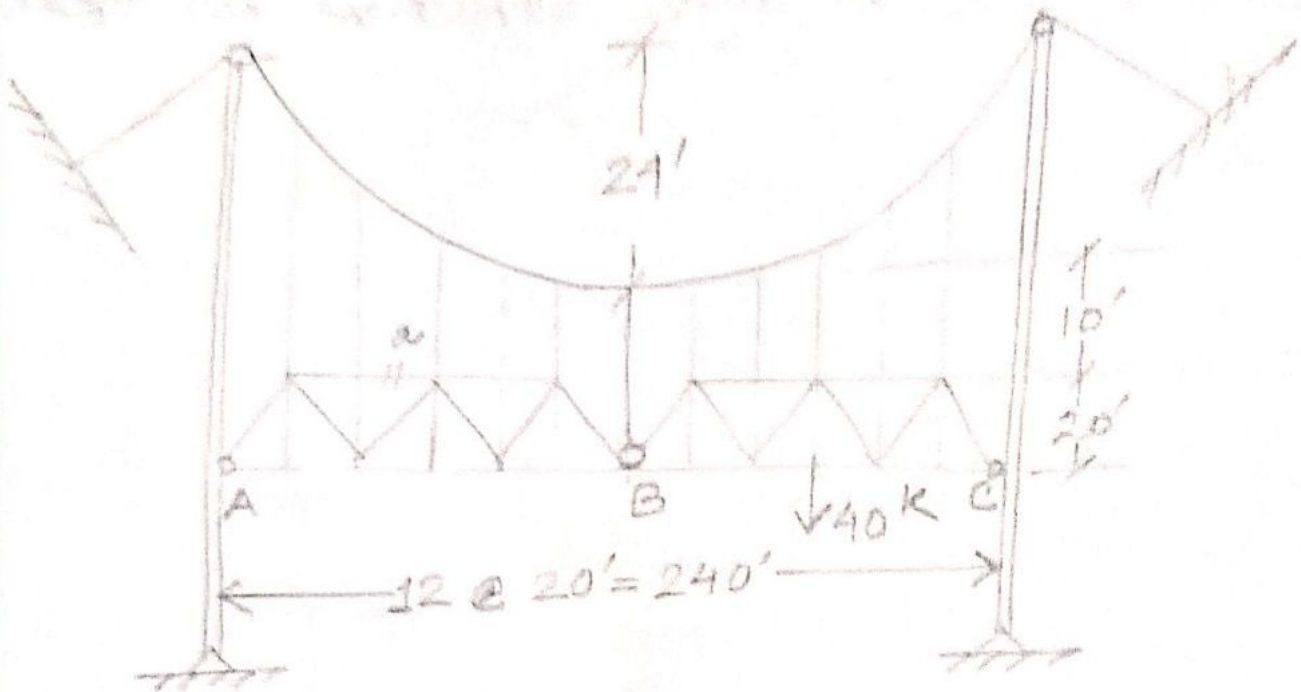
Bar Force for Q-force System

Step-2: Analyze structure for Real force

P.



Bar force for P-force system

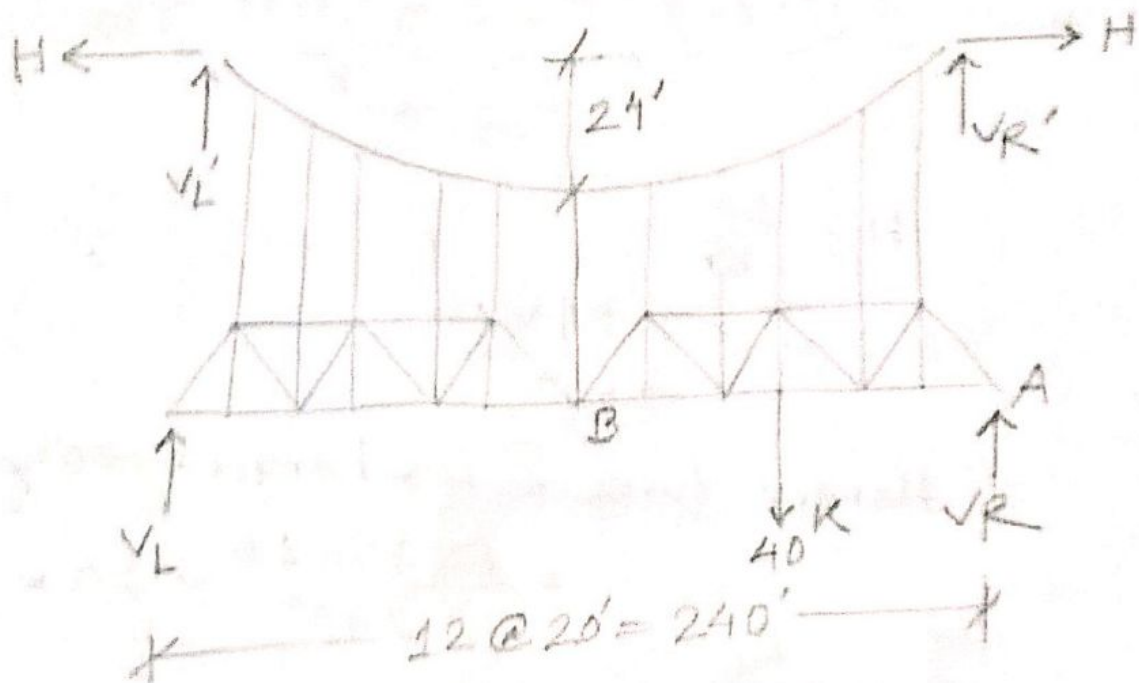


Question

(1) Hanger Force = 3.33^k

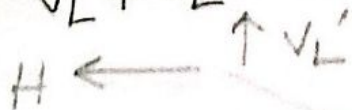
(2) $T_{max} = H [1 + 16\theta^2]^{\frac{1}{2}}$
 $= 53.85^k$

(c) Base Force $F_a = 13.33^k (+)$



$$(V_L + V_L') \times 240 - 40 \times 60 = 0$$

$$\therefore V_L + V_L' = 10 \text{ k}$$

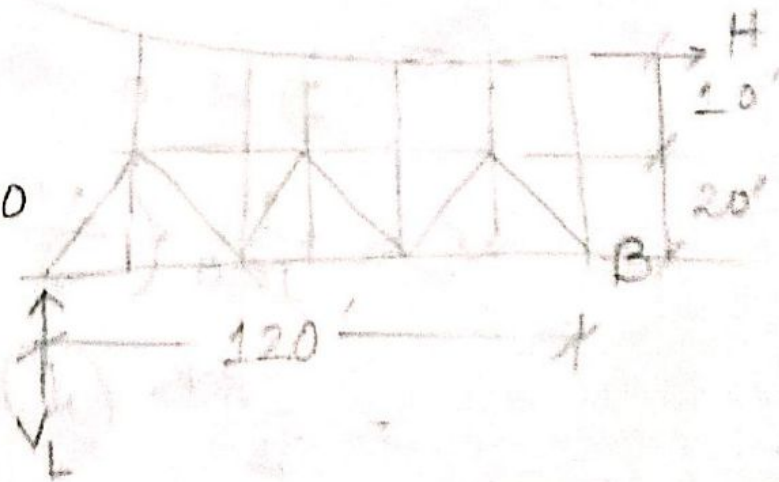


$$\Sigma M = 0$$

$$(V_L + V_L') \times 120 + H \times 30$$

$$- H \times 54 = 0$$

$$\therefore H = 50 \text{ k}$$



$$T_{\max} = H (1 + 16\theta^2)^{\frac{1}{2}} = 50 \left[1 + 16 \left(\frac{24}{240} \right)^2 \right]^{\frac{1}{2}}$$

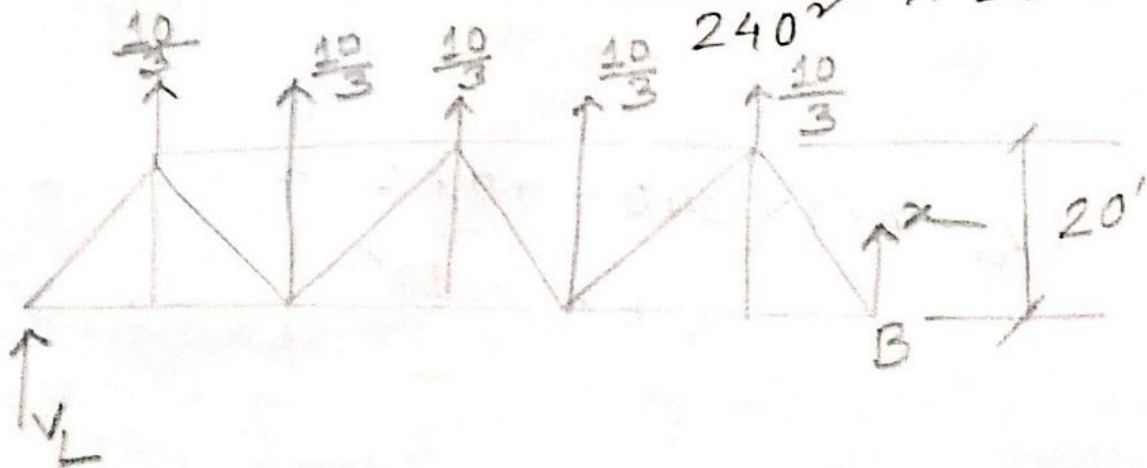
$$= 53.85 \text{ k}$$

$$H = \frac{\omega L^2}{8h}$$

$$\Rightarrow \omega = \frac{8 \times 24 \times 50}{240^2}$$

\therefore Hanger force = $\omega \times$ hanger spacing

$$= \frac{8 \times 24 \times 50}{240^2} \times 20 = \frac{10}{3} \text{ k}$$

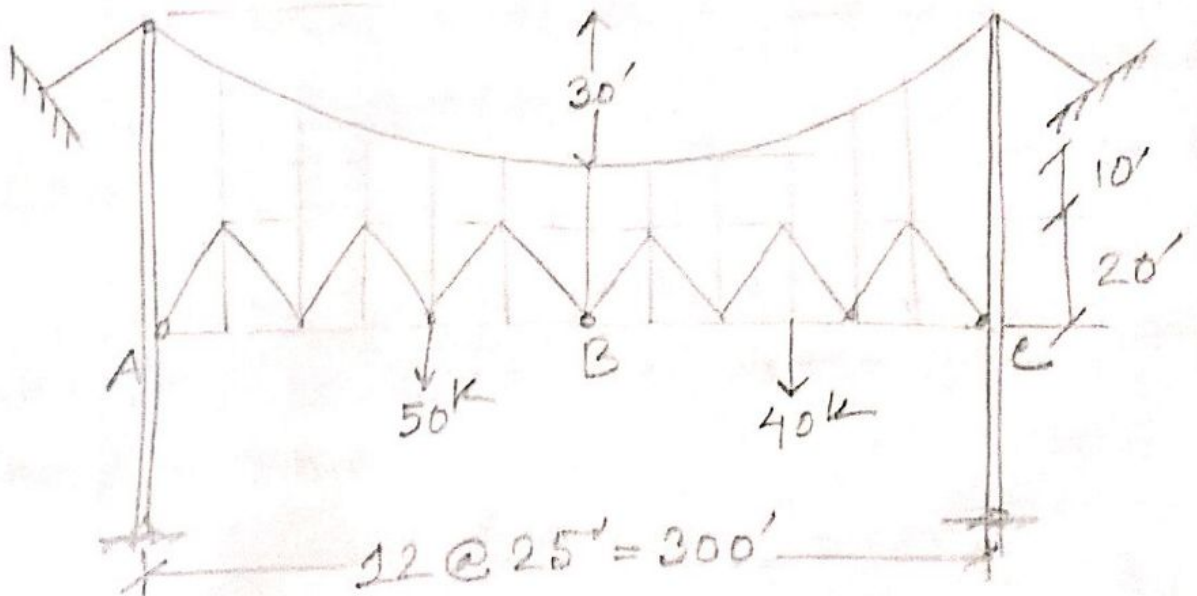


$$\sum M = 0$$

$$V_L = \frac{1}{120} \left\{ \frac{10}{3} \cdot (100 + 80 + 60 + 40 + 20) \right\}$$

$$= \frac{25}{3} \text{ k } (\downarrow)$$

Assignment



- Find:
- 1) Hanger Force
 - 2) T_{max}
 - 3) F_a, F_b, F_c

Bar	L (ft)	A (in ²)	L/A	F _g (k)	F _p (k)	F _g F _p L/A	t	F _g tL
Horizontal Bar	ab	10	10	1	+0.5	+10	+5	
	bc	10	10	1	+0.5	+10	+5	
Diagonal Bar	aB	14.14	12.5	1.13	-0.707	-14.14	+11.3	
	cB	14.14	12.5	1.13	-0.707	-14.14	+11.3	
Vertical Bar	bB	10	5	2	0	+20	0	
						Σ F _g F _p L/A		
						= 32.6		

AE is constant
W₃ = W_d

$$W_d = \sum \frac{F_g F_p L}{AE} + \sum F_g \alpha_e t L$$

$$W_3 = W_d$$

$$\sum \delta_B = \sum \frac{F_g F_p L}{AE} + \sum F_g \alpha_e t L$$

$$\Rightarrow 1. \delta_B = \frac{32.6}{E} = \frac{32.6}{29000}$$

$$\therefore \delta_B = 0.00112 \text{ ft} \quad (\text{downward})$$

Example 2:

Consider the problem in example 1 with 50°F temperature decrease in bottom chord member.

Given, $\alpha_t = \frac{1}{150,000} \text{ per } ^\circ\text{F}$

Bar	$\frac{L}{(\text{ft})}$	A (in^2)	$\frac{L}{A}$	F_Q (k)	F_P (k)	$F_Q F_P \frac{L}{A}$	t	$F_Q t L$
Horizontal Bar	ab	10	1	+0.5	+10	+5	-50	-25
	bc	10	1	+0.5	+10	+5	-50	-25
Diagonal Bar	aB							
	cB							
Vertical Bar	bB							

Using Principle of Virtual Work,

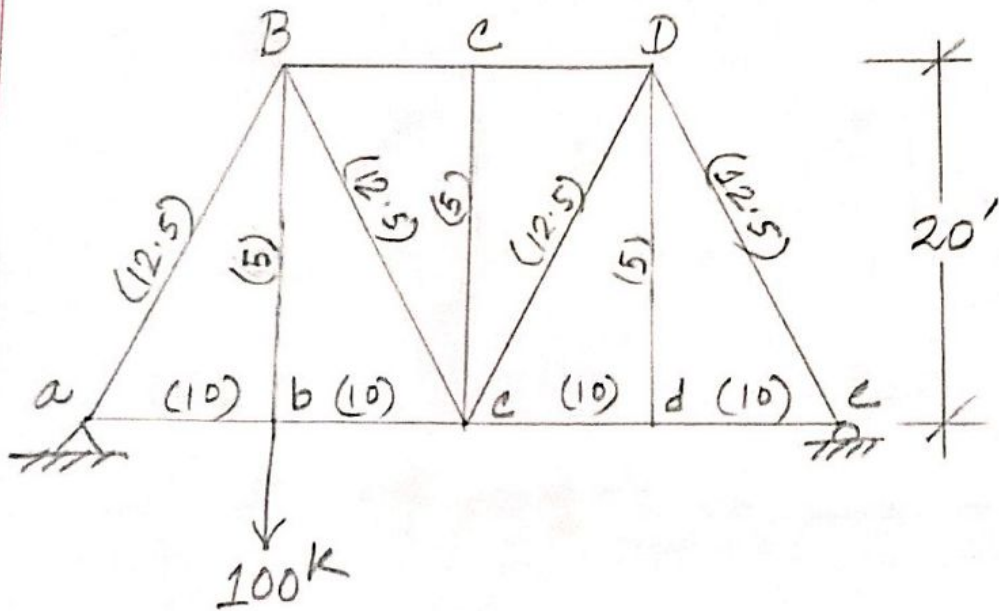
$$W_s = W_d$$

$$\sum Q \cdot \delta_B = \sum \frac{F_B F_P L}{AE} + \sum \frac{F_B \delta_t L}{E}$$
$$= \frac{32.6}{E} + -500 \times \delta_t$$

$$1 \cdot \delta_B = \frac{+32.6}{29000} - \frac{500 \times 1}{150000}$$

$$= -0.00221 \text{ ft (upward)}$$

Example 8.1:

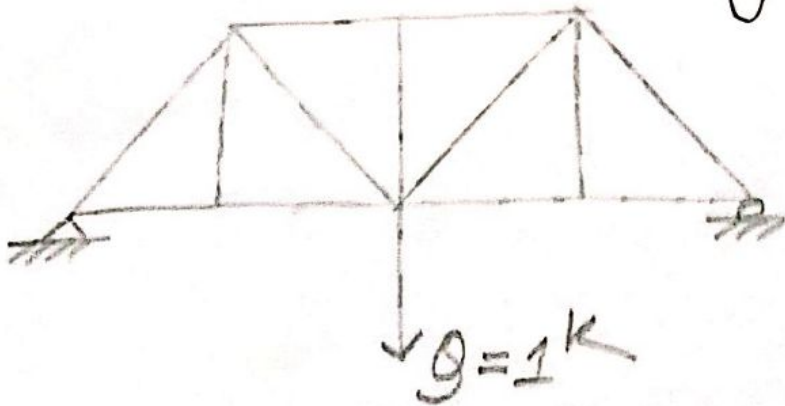


4 @ 15' = 60'

(a) Compute vertical deflection of joint e (bottom) chord. Given, $E = 300,000$ ksi

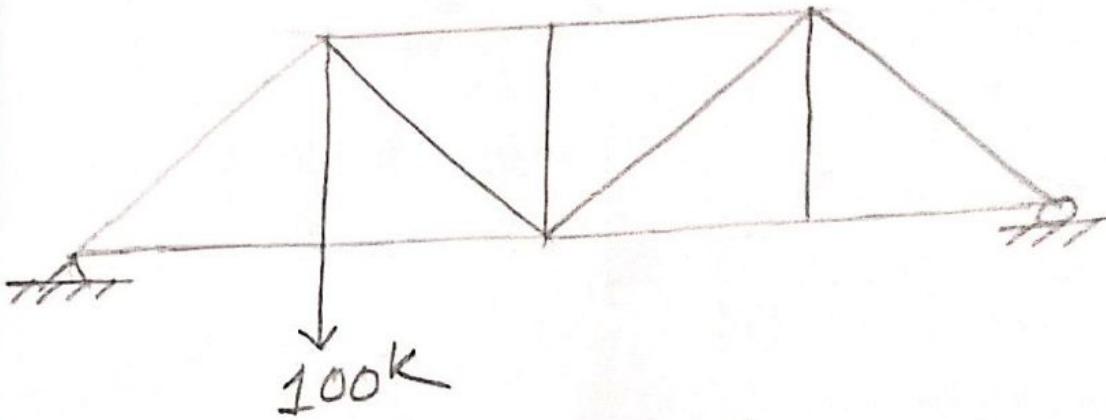
Step 1:

(b) Compute δ_c due to decrease in temperature of 50°F in the bottom chord only.



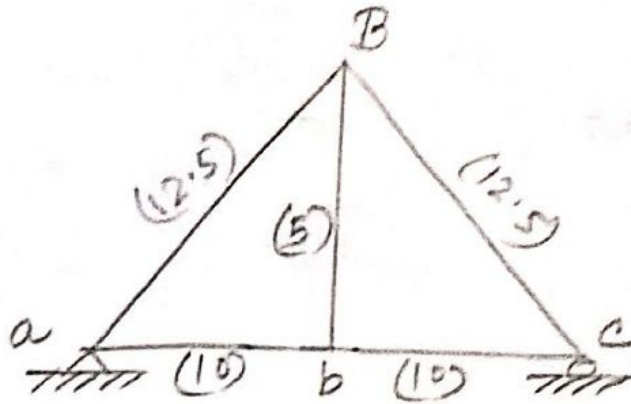
Bar Force for Q -Force system

Step - 2:



Bar Force for P-force system

Assignment 3

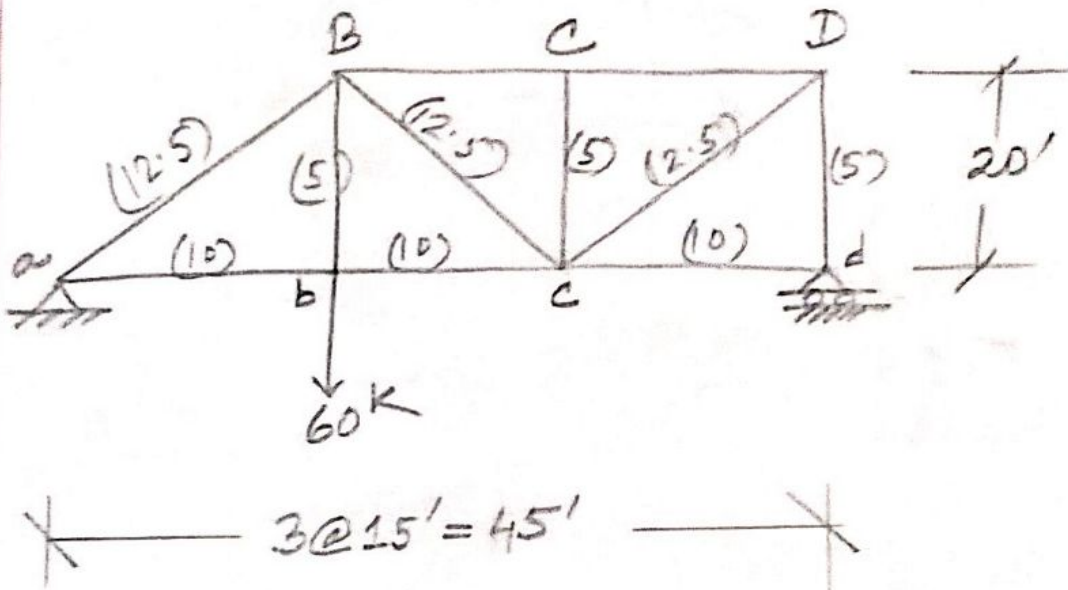


Temp. increase in Diagonal member = 100°F
Temp. decrease " horizontal " = 60°F

Compute vertical deflection at joint B of bottom chord.

$$\alpha_t = \frac{1}{150,000} \text{ per } ^{\circ}\text{F}$$

Assignment 4



(a) Compute vertical deflection of joint c (bottom chord) for the load shown and due to temp increase of 60°F in bottom chord & vertical members

$$E = 30000$$

$$\alpha_t = \frac{1}{150,000} \text{ per } ^{\circ}\text{F}$$

Truss with Support Movement

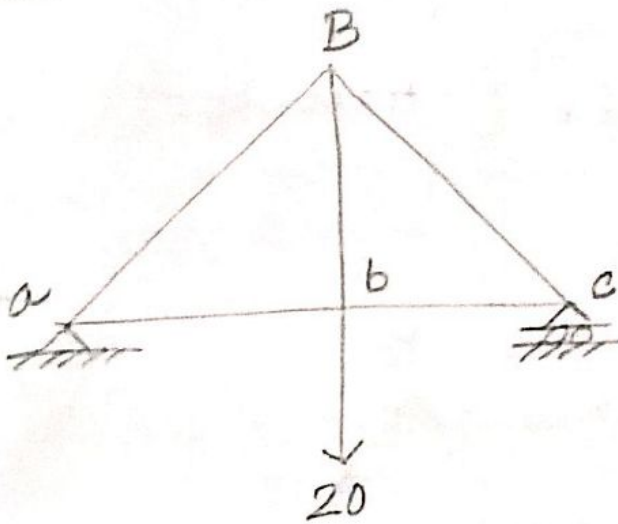
Principle of Work External Virtual Work by Virtual Reaction

$$\sum \delta S + W_R = \sum \frac{F_S F_P L}{AE} + \sum F_S \delta \left(\frac{\Delta L}{L} \right)$$

ΔL for FP

ΔL for temp change

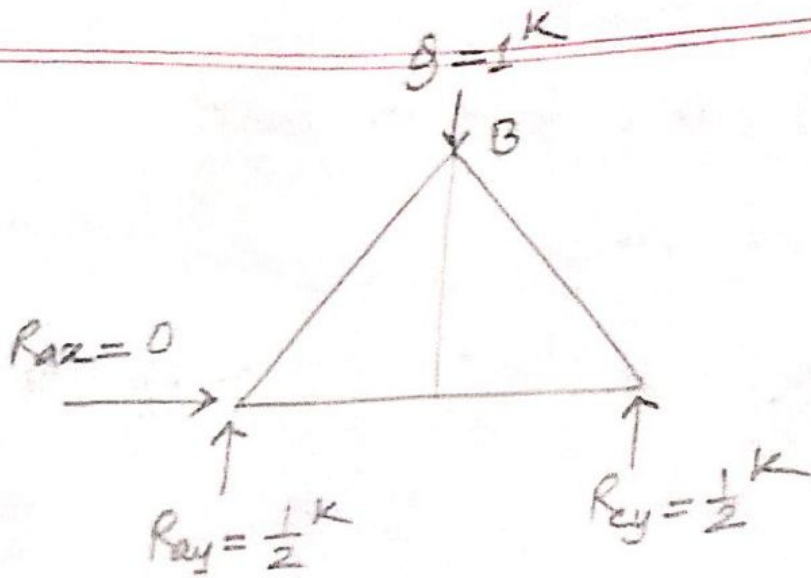
Problem



In addition to 20" load at "b".

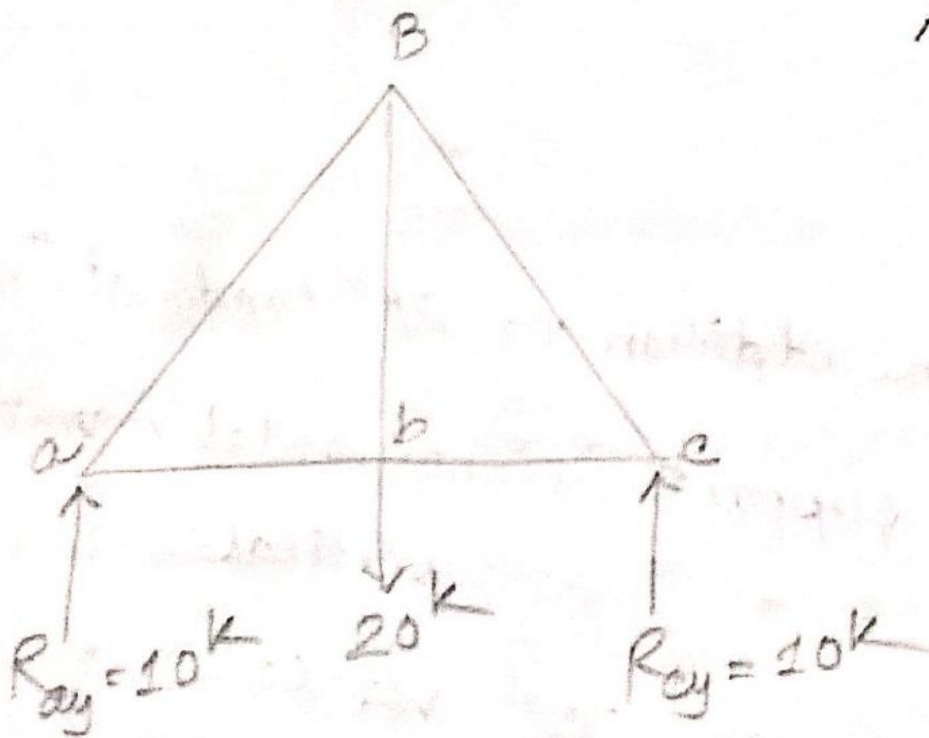
- At support "a" horizontal movement = 0.5" to left
- " " vertical " = 0.75" down
- " " "c" vertical " = 0.3" down

Calculate vertical deflection at B (Top chord)



Bar force for G -force system

$$\frac{\sum F_B F_P L}{A} = 32.6$$



Bar force for P -force system

Using principle of virtual work, (No temp change)

$$\sum 1 \cdot \delta_B + W_R = \sum \frac{F_x F_P L}{AE} + \sum F_B \delta_{x_i} \pm L$$

$$\Rightarrow \delta_B + R_{ax} \Delta_{ax} + R_{ay} \Delta_{ay} + R_{cy} \Delta_{cy} = \frac{32.6}{E}$$

$$\Rightarrow \delta_B + 0 \times \frac{0.5}{12} + \frac{1}{2} \times \left(-\frac{0.75}{12} \right) + \frac{1}{2} \times \left(-\frac{0.3}{12} \right) = \frac{32.6}{29000}$$

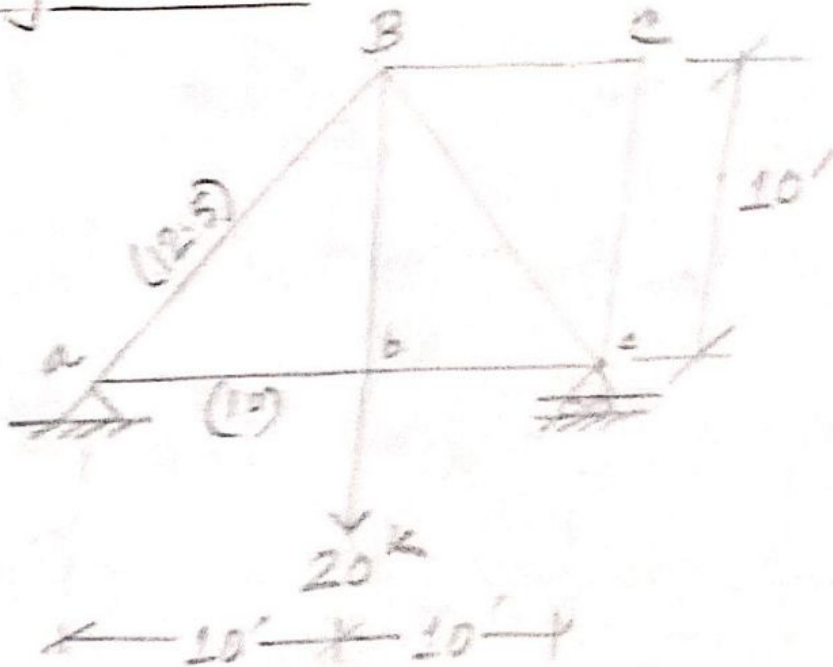
Reaction reaction
movement opposite
direction \rightarrow 2nd part (-)ve 2nd,

$$\delta_B = 0.00112 + 0.04375$$

Sign convention (movement at dirⁿ along reaction \rightarrow (+)
opposite of \rightarrow (-)

See Example Problems { 8.1
8.2
8.3
8.4

Assignment 5:



Load case #1 In addition to 20 k load at B

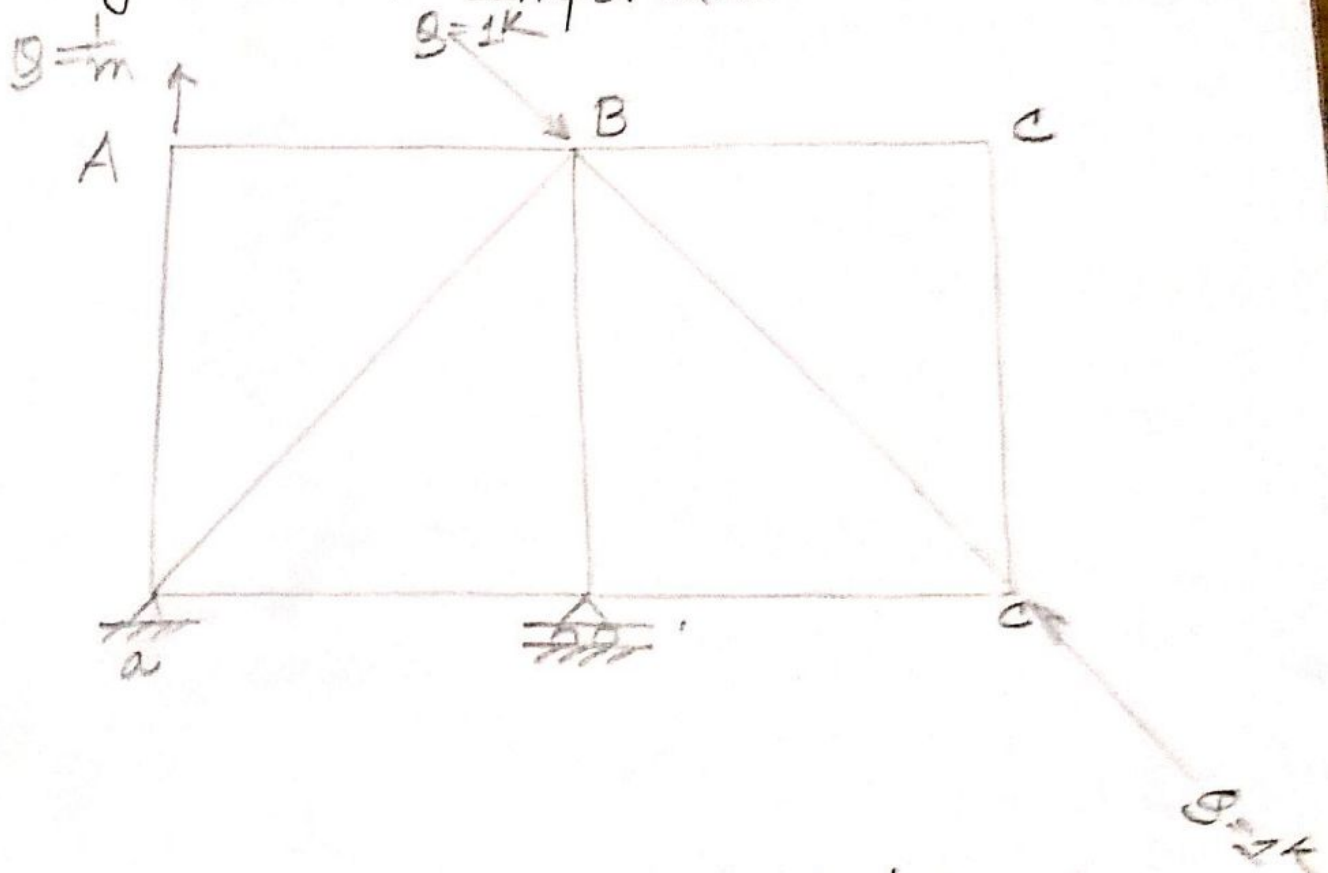
Load case #2 At support 'a' horizontal moment = 0.5" to Left
 for support vertical " = 0.75" down
 moment " 'c' " = 0.3" down

Load case #3 50° F temp rise in bottom chord members.

Given, $\alpha_t = \frac{1}{150,000}$ per °F

Find horizontal deflection at joint 'C' of top chord.

Fig 8.10 → Important



1) What is the relative movement of joint C & B?

2) What is the rotation of member AB?

$$\sum \theta_{C-B}^1$$

Deflection, Rotation for Beams & Frames

By virtual work (If support movement)

$$\sum \delta \delta + W_R = \sum \frac{F_Q F_P L}{AE} + \int \frac{M_Q M_P ds}{EI}$$

↑
support movement
at support is term zero.

$$+ \sum F_Q \alpha t L$$

↑
(for temperature change)

For beams with no axial force & temperature change and support movement

$$\sum \delta \delta = \int \frac{M_Q M_P}{EI} ds$$

virtual force at moment

real force at moment

Book Example for Truss

Example 8.1

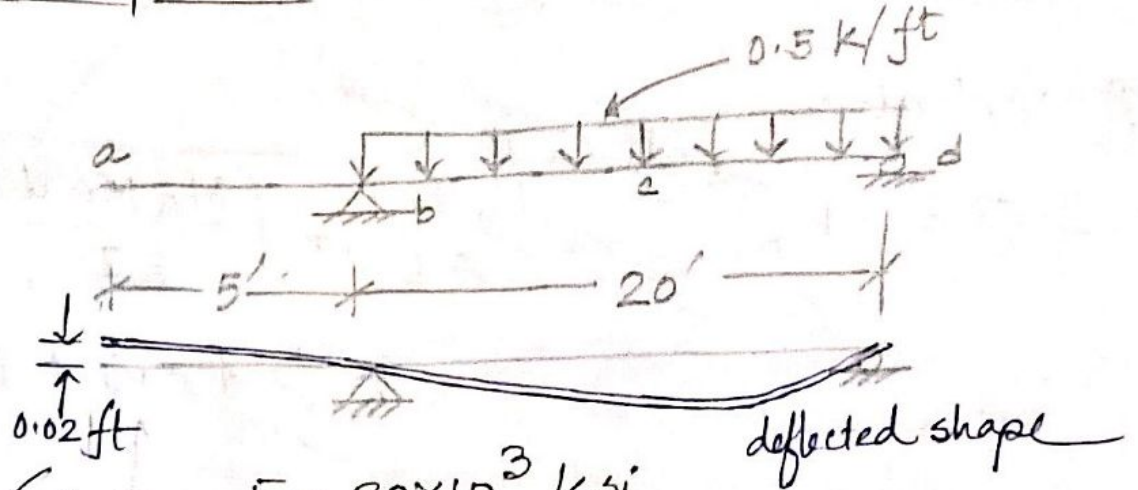
8.2

8.3

8.4

(very important for exam)

Example 8.5:



Given, $E = 30 \times 10^3$ ksi

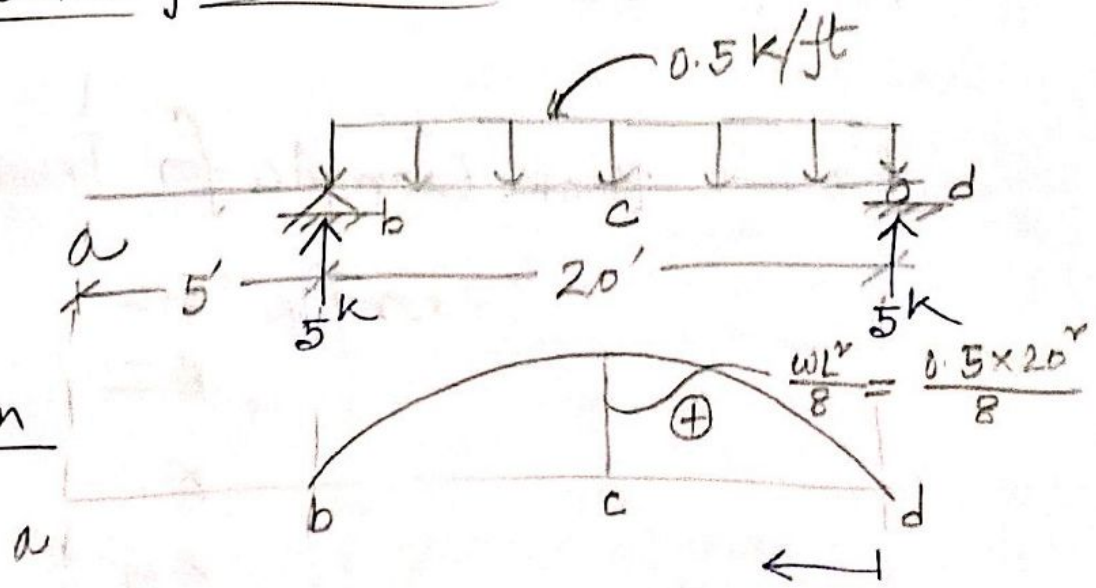
$I = 200 \text{ in}^4$

~~MP, MB
2100000
500000
400000~~

Compute vertical deflection at point a

Solve for P-Force

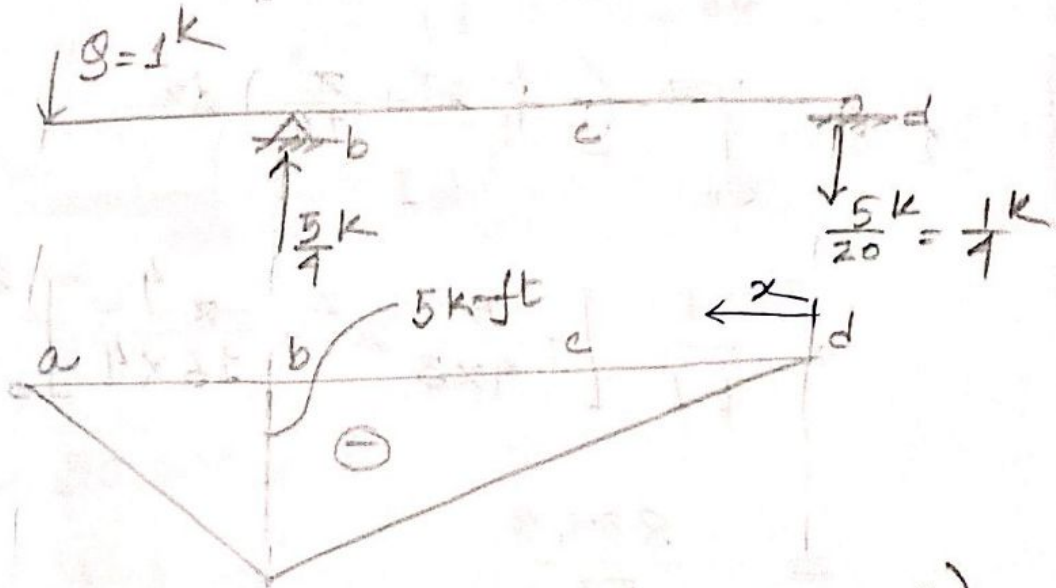
Mp diagram



Why from 'd' to 'b'?

For convenience of formulating the equation

Solve for Q-force



M_q diagram

By Virtual Work,

$$\sum \delta \mathcal{E} = \int_a^b \frac{M \delta M_p}{EI} dx + \int_d^b \frac{M \delta M_p}{EI} dx$$

0 ($\because M_p = 0$)

For segment 'ab' ($0 < x \leq 5'$)

$$M_p = 0$$

$$M_q = 1x \quad (0 < x \leq 20')$$

For segment 'db' ($d < x \leq b$)

$$M_p = 5x - 0.5x \times \frac{x}{2} = 5x - \frac{x^2}{4}$$

$$M_q = -\frac{1}{4}x$$

$$\Sigma \delta \cdot \delta = \int_0^{20} \frac{-\frac{1}{4}x \left(5x - \frac{x^2}{4}\right) dx}{EI}$$

$$1 \cdot \delta = \int_0^{20} \frac{\left(-\frac{5}{4}x^2 + \frac{x^3}{16}\right) dx}{EI}$$

$$= \frac{1}{EI} \left[-\frac{5}{4 \times 3} x^3 + \frac{x^4}{16 \times 4} \right]_0^{20}$$

$$= -\frac{833.3}{EI}$$

$$E = 30,000 \text{ ksi} \rightarrow (12)^2$$

$$= 30,000 \times 144 \text{ ksf}$$

$$I = 200 \text{ in}^4$$

$$= \frac{200}{144 \times 144} \text{ ft}^4$$

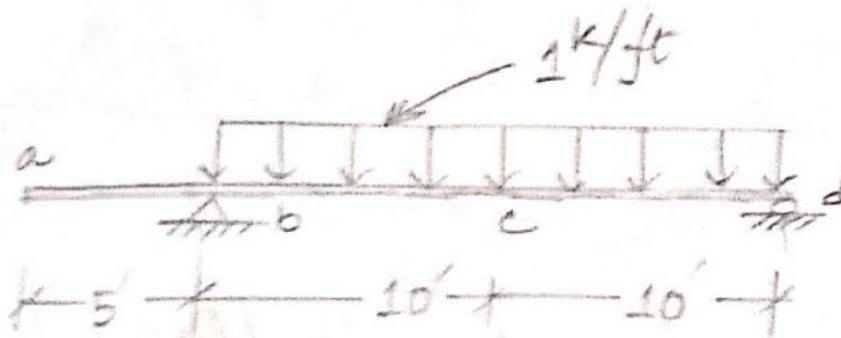
(12)⁴

$$= -\frac{833.3}{30,000 \times 144 \times \frac{200}{144 \times 144}}$$

$$= -0.02 \text{ ft (upward)}$$

↑ ২২ ফুট উল্লম্ব ↑
কমতায় - ২২ ফুট

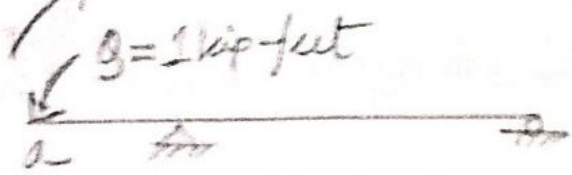
Assignment # 6



$E = 30,000 \text{ ksi}$
 $I = 300 \text{ in}^4$

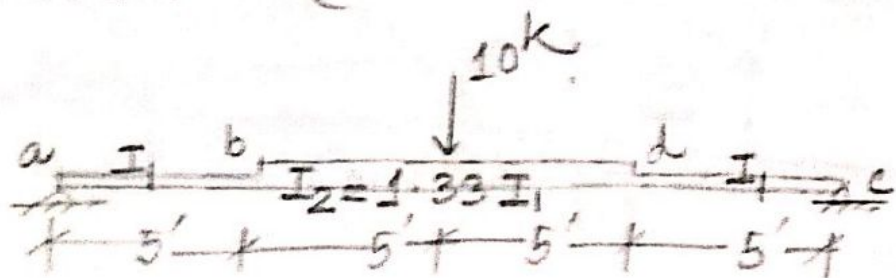
Compute:

- (a) vertical deflection at "c" (b to c & d to c)
- (b) rotation or slope at point "a".



* See example 8.6

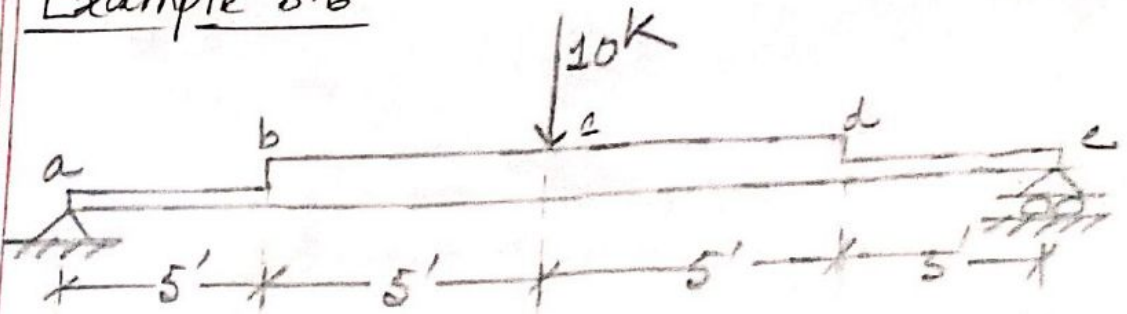
Or as answer in radians



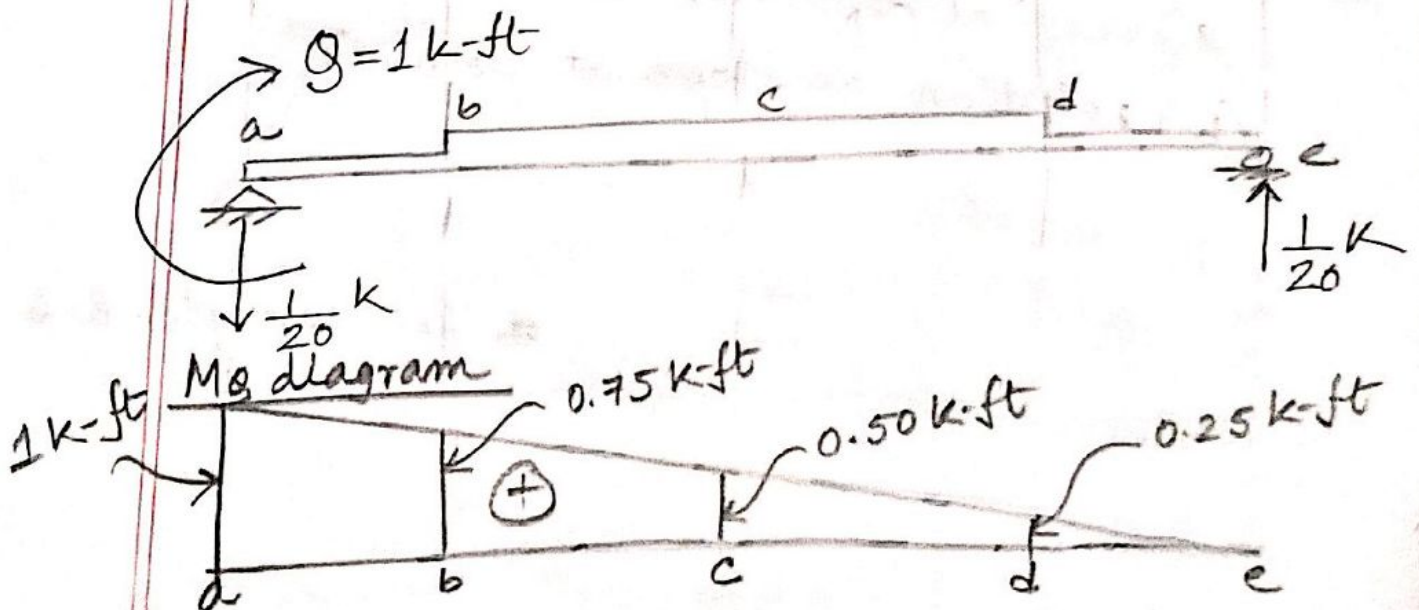
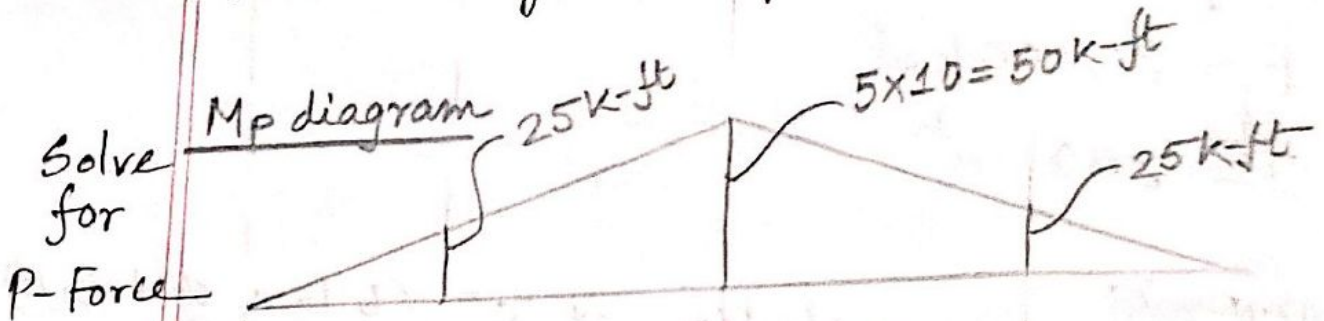
Given, $E = 30,000 \text{ ksi}$
 $I_1 = 150 \text{ in}^4$

Compute change in slope at point 'a'

Example 8.6



Find change in slope 'a' ($\theta_a = ?$)



By Virtual Work Method,

θ_a , change in slope \rightarrow

$$\sum \delta \theta_a = \int_a^b \frac{M_B M_p}{EI_1} dx + \int_b^c \frac{1}{EI_2} dx + \int_d^c \frac{1}{EI_2} dx + \int_e^d \frac{1}{EI_1} dx$$

Segment 'ab' ($0 < x \leq 5$)

$$M_p = +5x$$

$$M_B = +\left(1 - \frac{1}{20}x\right)$$

Segment 'bc' ($0 < x \leq 5$)

$$M_p = + (25 + 5x)$$

$$M_B = + \left(0.75 - \frac{1}{20}x\right)$$

Segment 'dc' ($0 < x \leq 5$)

$$M_p = + (25 + 5x)$$

$$M_B = + \left(0.25 + \frac{1}{20}x\right)$$

Segment 'ed' ($0 < x \leq 5$)

$$M_p = + 5x$$

$$M_B = + \frac{1}{20}x$$

$$\Sigma \theta_a = \int_a^b \frac{M_Q M_P}{EI_1} dx + \int_b^c \frac{1}{EI_2} dx$$

$$+ \int_d^c \frac{1}{EI_2} dx + \int_c^d \frac{1}{EI_1} dx$$

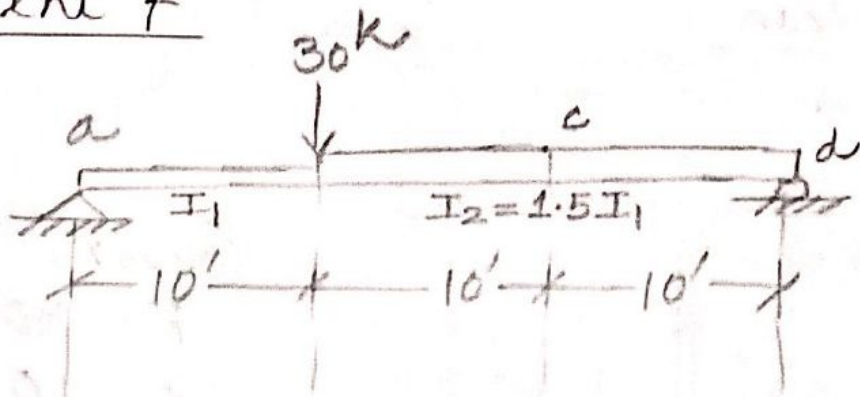
$$= \int_0^5 \frac{(5x) \left(1 - \frac{x}{20}\right)}{EI_1} dx + \int_0^5 \frac{1}{EI_2} dx$$

$$+ \int_0^5 \frac{1}{EI_2} dx$$

$$+ \int_0^5 \frac{1}{EI_1} dx$$

$$\Rightarrow 1 \cdot \theta_a = + 0.0065 \text{ radian (clockwise)}$$

Assignment 7

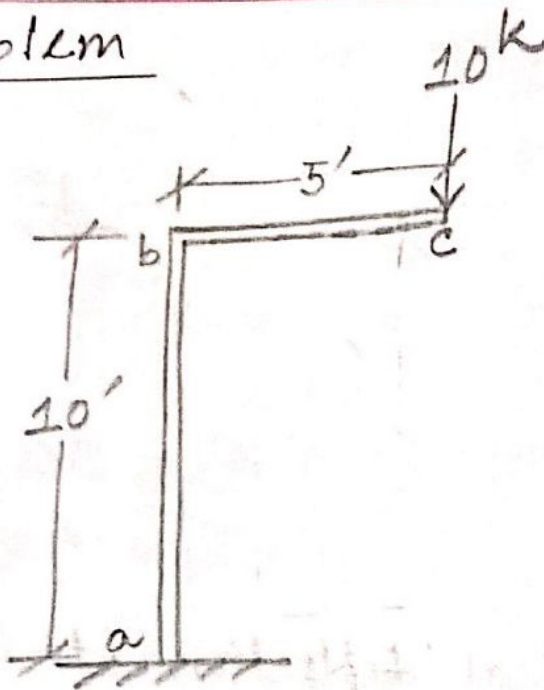


$$E = 30,000 \text{ ksi}$$

$$I_1 = 400 \text{ in}^4$$

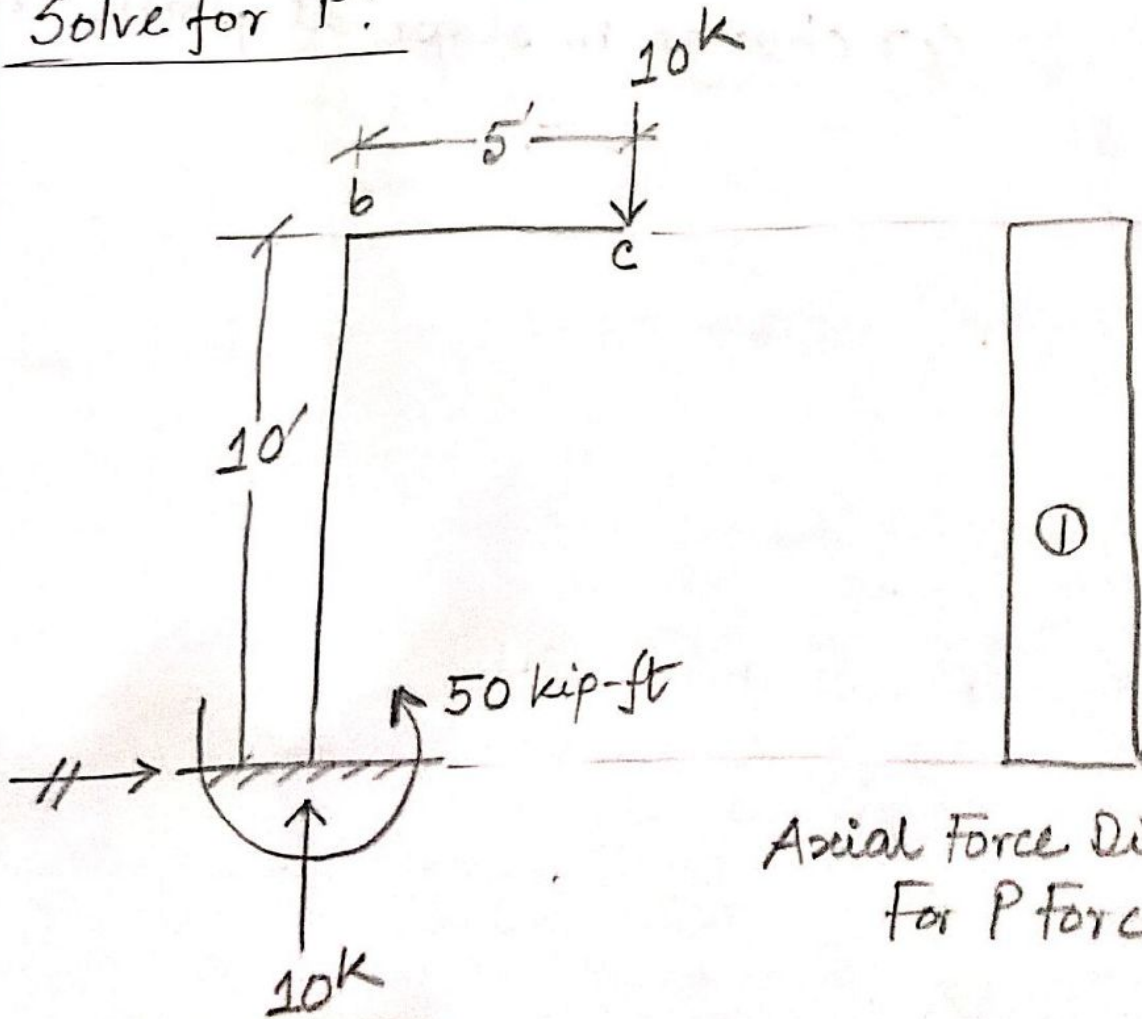
- Compute
- Vertical deflection at "c"
 - Vertical deflection at "b"
 - change in slope at point "d"

Problem

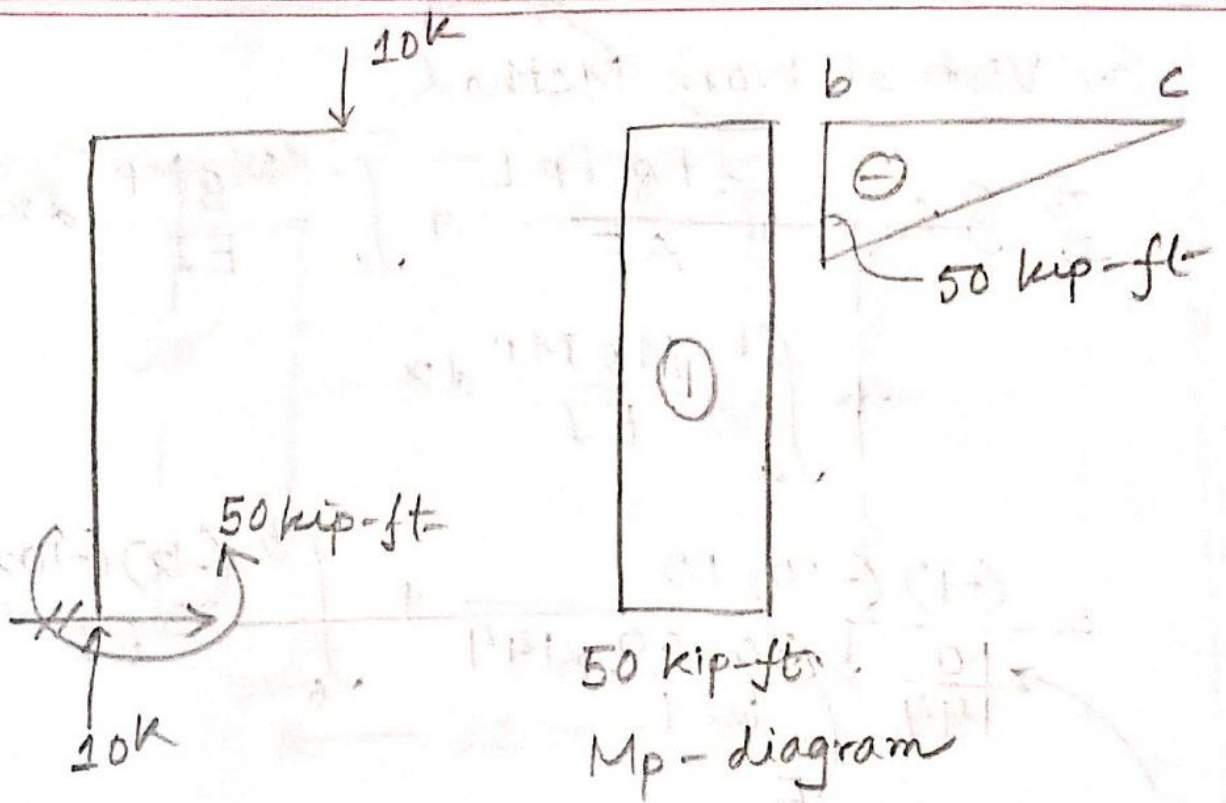


Given,
 $E = 30,000 \text{ ksi}$
 $A = 10 \text{ in}^2$
 $I = 200 \text{ in}^4$

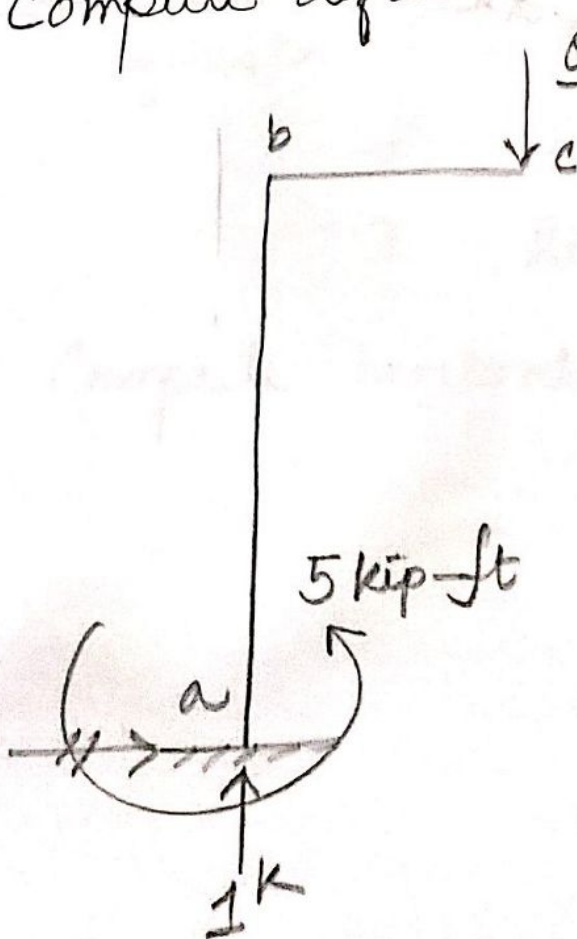
Solve for P:



Axial Force Diagram
For P force



Compute deflection at c.



By Virtual Work Method,

$$\sum \delta \delta_c = \frac{\sum F \delta F P L}{AE} + \int_c^b \frac{M \delta M P}{EI} dx$$

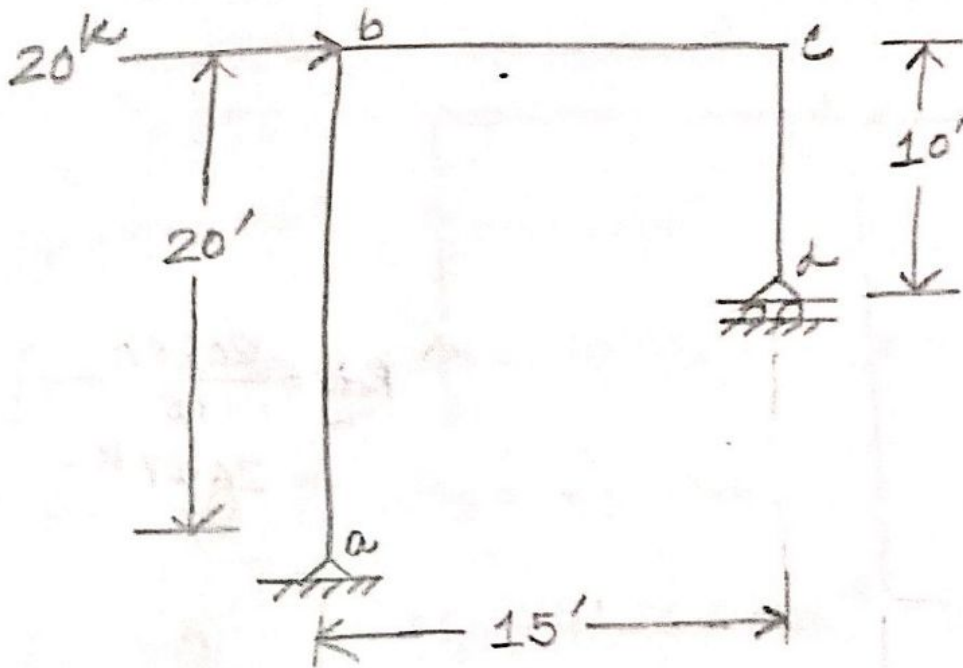
$$+ \int_a^b \frac{M \delta M P}{EI} dx$$

$$= \frac{(-1)(-10)10}{\frac{10}{144} \times \frac{30000}{144} \times 144} + \int_0^5 \frac{(-1x)(-10x)}{EI} dx$$

A

$$+ \int_0^{10} \frac{(-5)(-50)}{EI} dx$$

this expression may be wrong



Problem

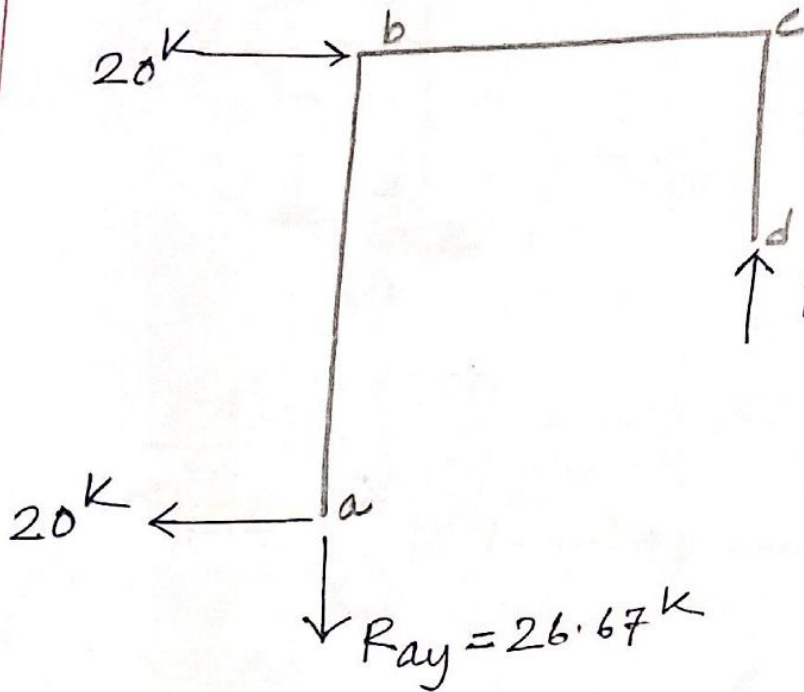
Given, $E = 30,000 \text{ ksi}$

$A = 10 \text{ in}^2$

$I = 200 \text{ in}^4$

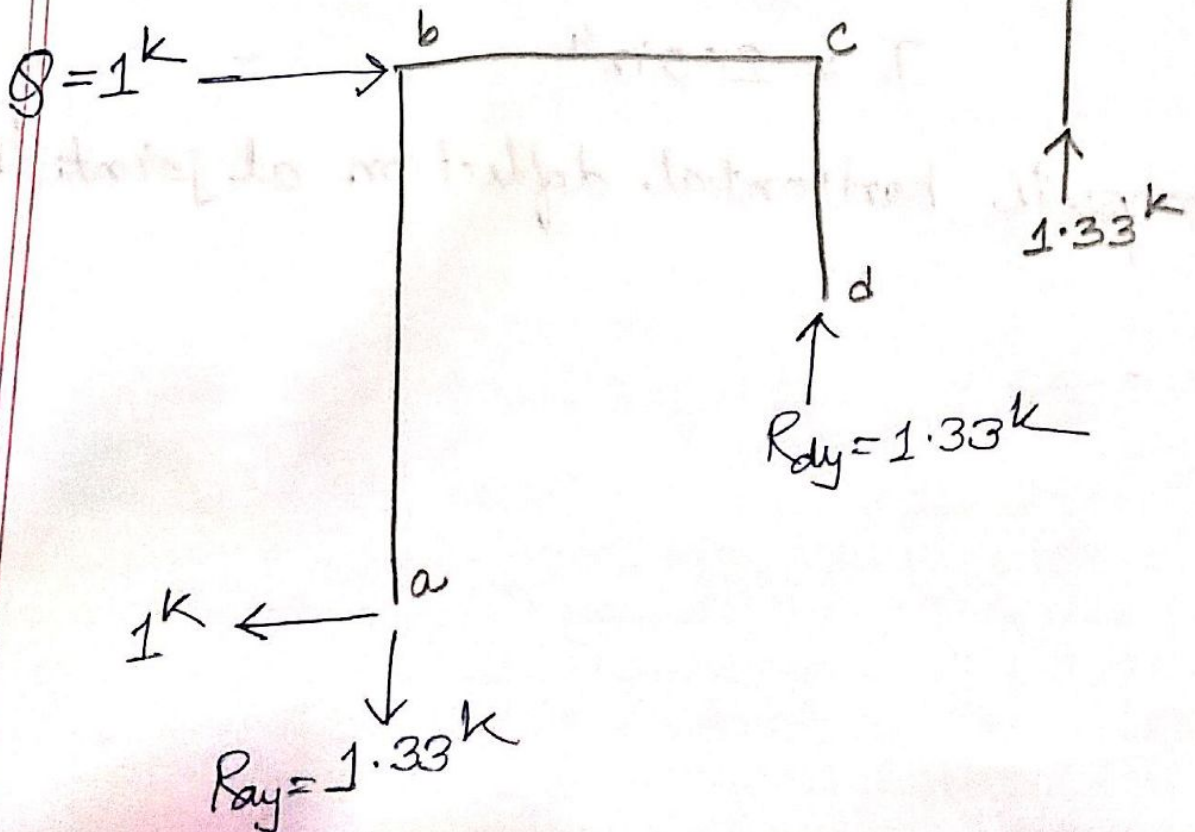
Compute horizontal deflection at joint 'b'

Solve for P-Force



$$R_{dy} = \frac{20 \times 20}{15} = 26.67\text{ k}$$

Solve for Q-Force



From a to b (segment ab) $0 < y \leq 20'$

$$\begin{array}{l|l} F_g = +1.33 \text{ k} & M_g = +1y \\ F_p = +26.67 \text{ k} & M_p = +20y \end{array}$$

From c to b (segment cb) $0 < x \leq 15'$

$$\begin{array}{l|l} F_g = 0 & M_g = +1.33x \\ F_p = 0 & M_p = +26.67x \end{array}$$

From d to c ($0 < y \leq 10'$)

$$\begin{array}{l|l} F_g = -1.33 \text{ k} & M_g = 0 \\ F_p = -26.67 \text{ k} & M_p = 0 \end{array}$$

By Virtual Work Method,

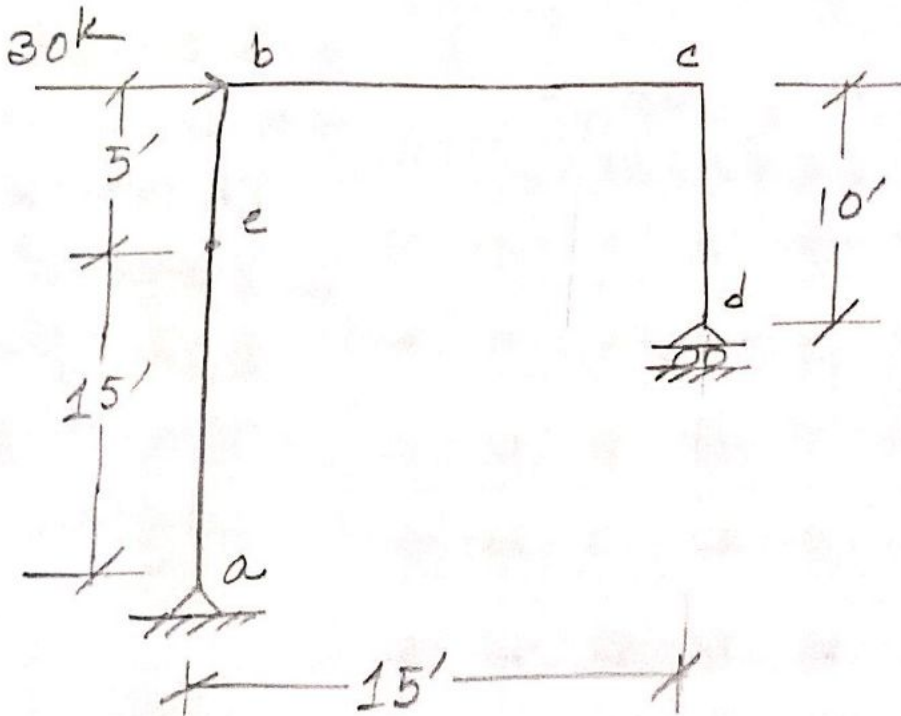
$$\sum Q \cdot \delta_{kb} = \sum \frac{F_g F_p L}{AE} + \int_a^b \frac{M_g M_p}{EI} dy + \int_c^b + \int_d^e = 0$$

$$\Rightarrow 1. \delta_{hb} = \frac{(+1.33)(26.67)20}{\left(\frac{10}{144}\right)(30,000 \times 144)} + \text{for bc moment}$$

$$+ \frac{(-1.33)(-26.67) \times 10}{\left(\frac{10}{144}\right)(30,000 \times 144)}$$

$$+ \int_0^{20} \frac{(y)(20y)}{EI} dy + \int_0^{15} \frac{(1.33x)(26.67x)}{EI} dx + 0$$

Assignment #8



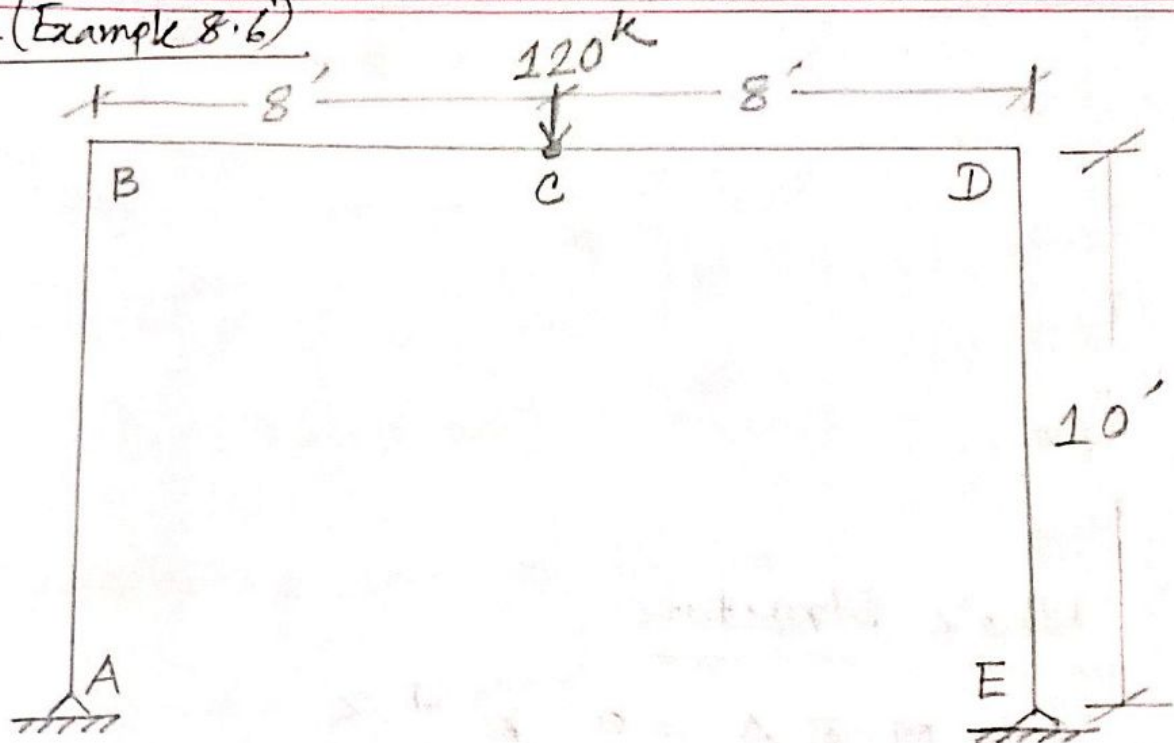
$$E = 30000 \text{ ksi}$$

$$A = 10 \text{ in}^2$$

$$I = 300 \text{ in}^4$$

Compute horizontal deflection at point 'c'

Book (Example 8.6)



Given, $E = 30,000 \text{ ksi}$

$$A = 20 \text{ in}^2$$

$$I = 2500 \text{ in}^4$$

Compute the change in slope of the cross-section on the left side of the hinge 'c'.

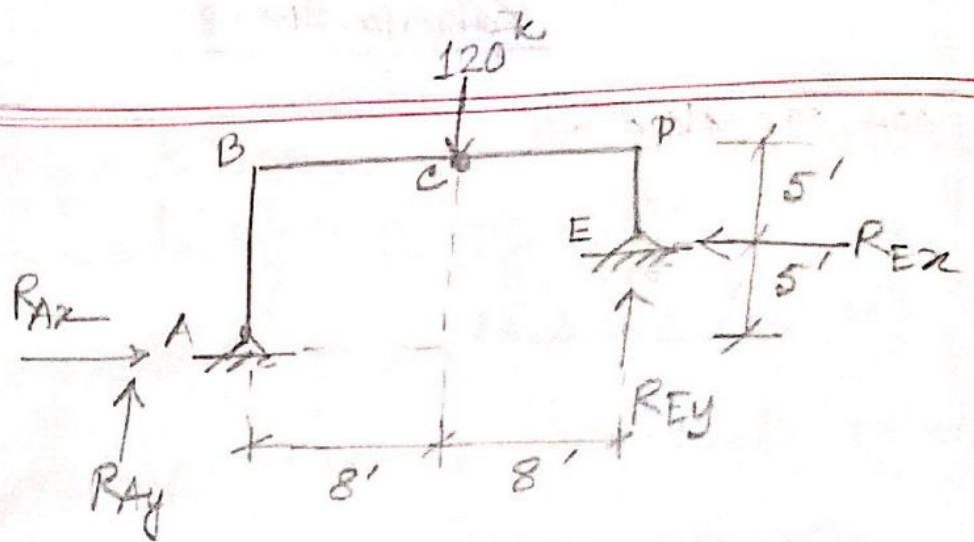
Unknown

R_{Ax}

R_{Ay}

R_{Ex}

R_{Ey}



Whole Structure

$$\sum M @ A = 0 \quad \curvearrowright +ve$$

$$\Rightarrow 120 \times 8 - R_{Ex} \times 5 - R_{Ey} \times 16 = 0$$

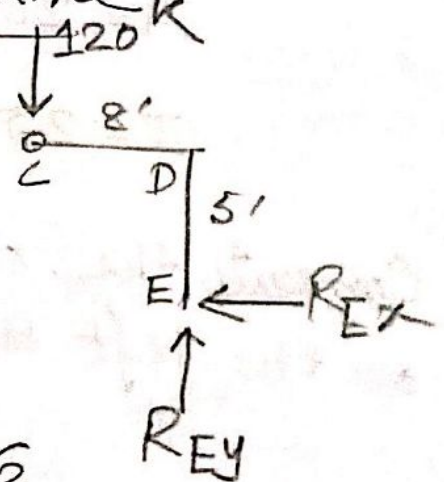
$$\Rightarrow 5R_{Ex} + 16R_{Ey} = 960 \quad \text{--- (1)}$$

Right Part of Structure / Frame

$$\sum M @ C = 0 \quad \curvearrowright +ve$$

$$R_{Ex} \times 5 - R_{Ey} \times 8 = 0$$

$$\Rightarrow 5R_{Ex} - 8R_{Ey} = 0 \quad \text{--- (2)}$$



Solving equations 1) & 2),

$$\textcircled{1} - \textcircled{2} \Rightarrow 24R_{Ey} = 960$$

$$\therefore R_{Ey} = 40 \text{ k}$$

$$R_{Ax} = 80 \text{ k}$$

Substituting value of R_{Ey} in 2),

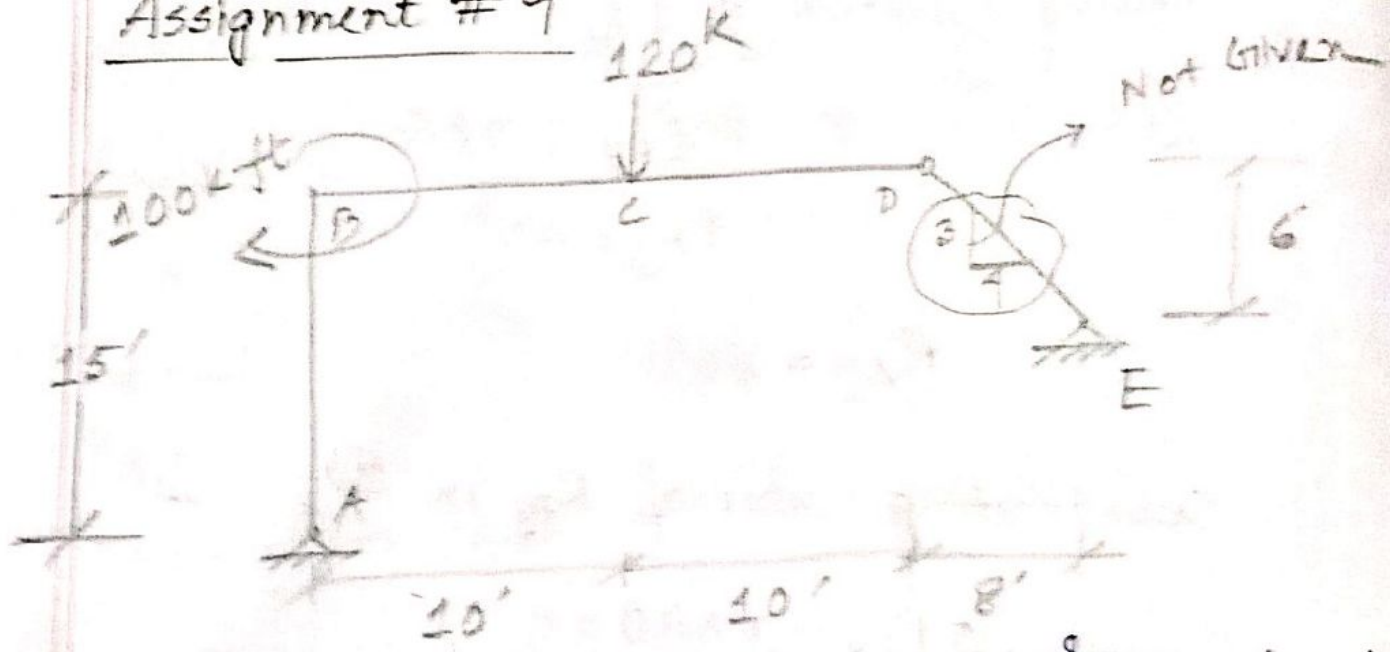
$$5R_{Ex} - 8 \times 40 = 0$$

$$\therefore R_{Ex} = 64 \text{ k} \left(\leftarrow \right)$$

$$\therefore R_{Ax} = 64 \text{ k} \left(\rightarrow \right)$$

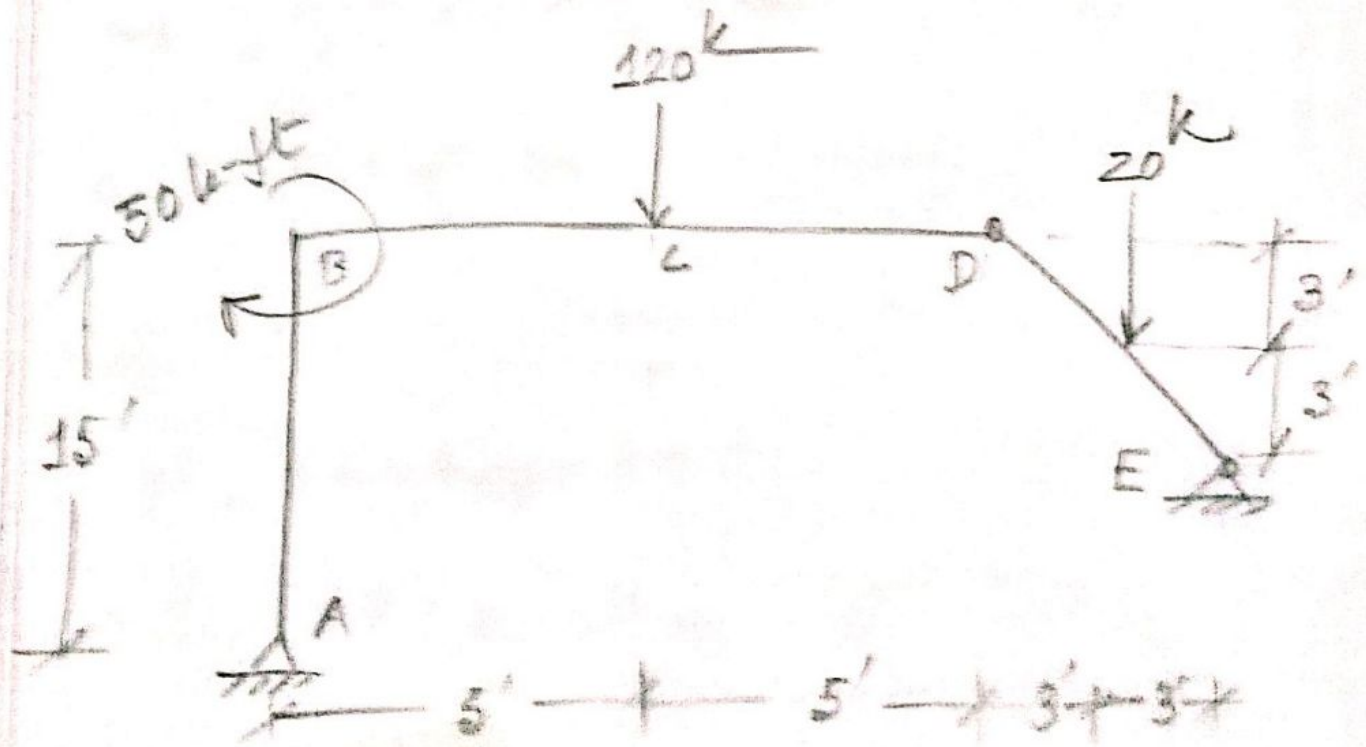
* Frame reaction, SFD, BMD
 - सबी problem 2020/21

Assignment # 9

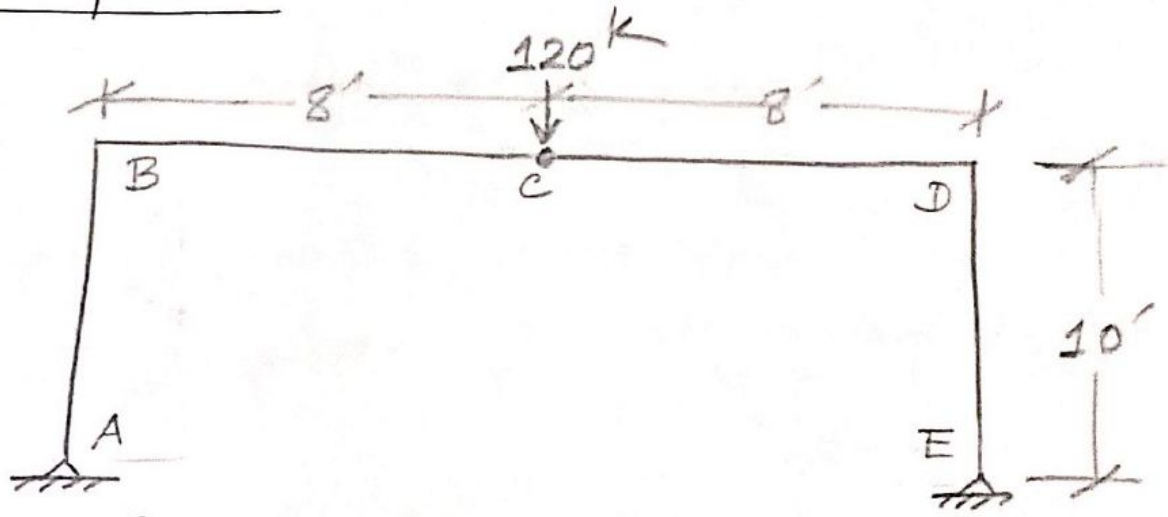


(सबसे पहिले pin सहायता member A force or समान सब त्रुस member or two force member)

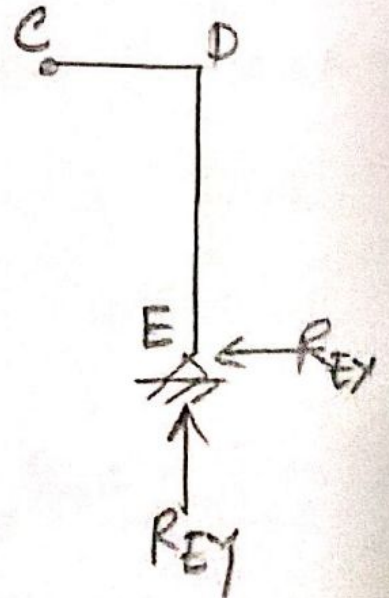
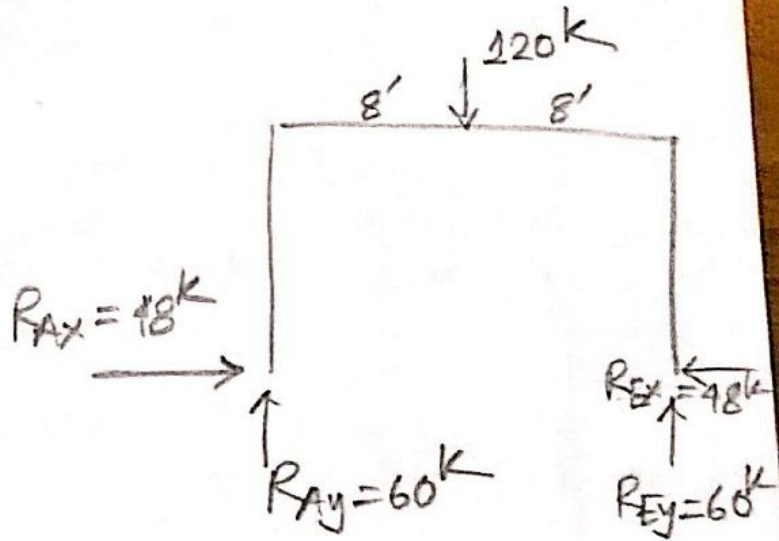
Assignment # 10



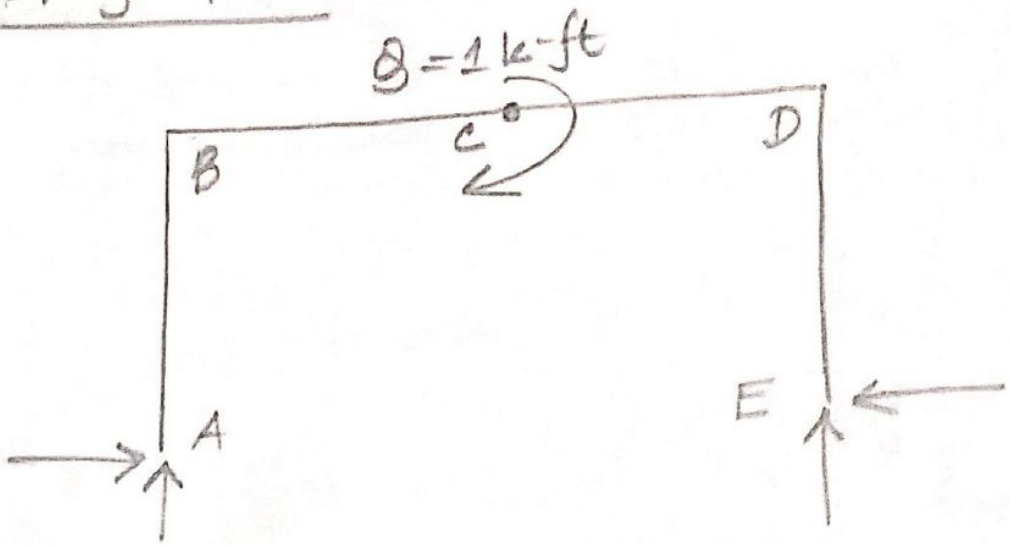
Example 8.6

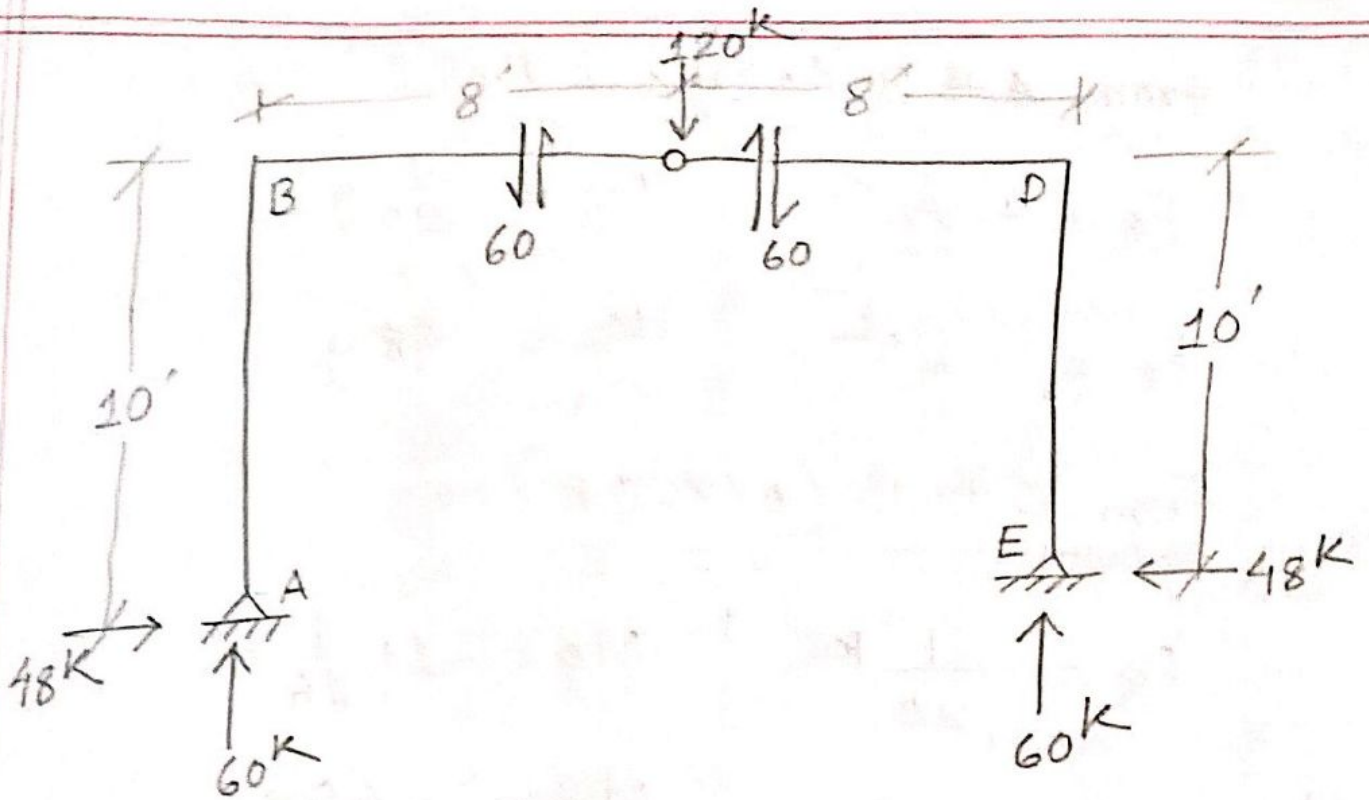


Solve for P-Force

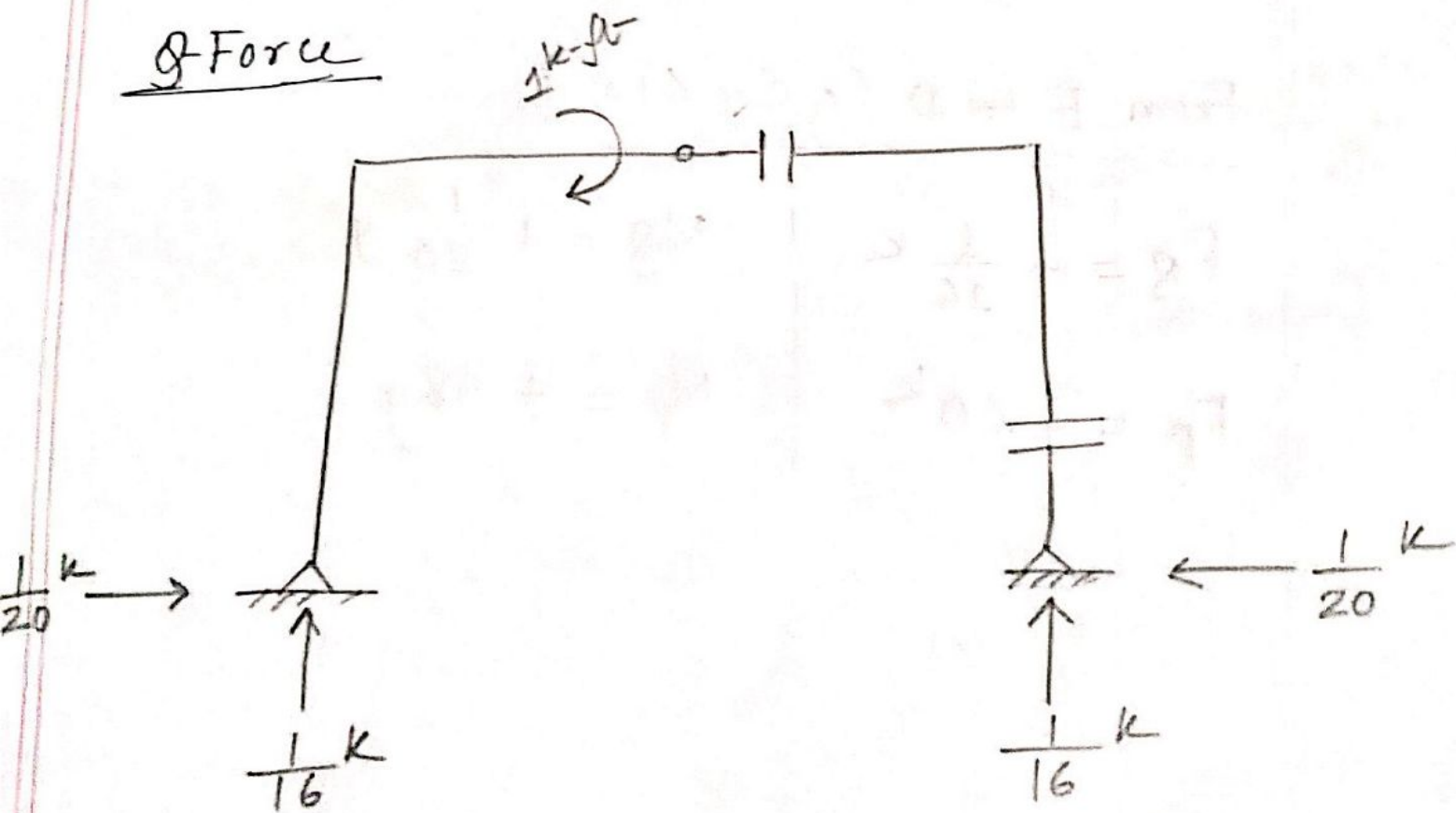


Solve for Q - Force





Q Force



From A to B ($0 < y \leq 10'$)

$$F_g = + \frac{1}{16} k \quad \left| \quad M_g = - \frac{1}{20} y$$

$$F_p = - 60 k \quad \left| \quad M_p = - 48 y$$

From C to B ($0 < x \leq 8'$)

$$F_g = - \frac{1}{20} k \quad \left| \quad M_g = - 1 + \frac{1}{16} x$$

$$F_p = - 48 k \quad \left| \quad M_p = - 60 x$$

From E to D ($0 < y \leq 10'$)

$$F_g = - \frac{1}{16} k \quad \left| \quad M_g = + \frac{1}{20} y$$

$$F_p = - 60 k \quad \left| \quad M_p = + 48 y$$

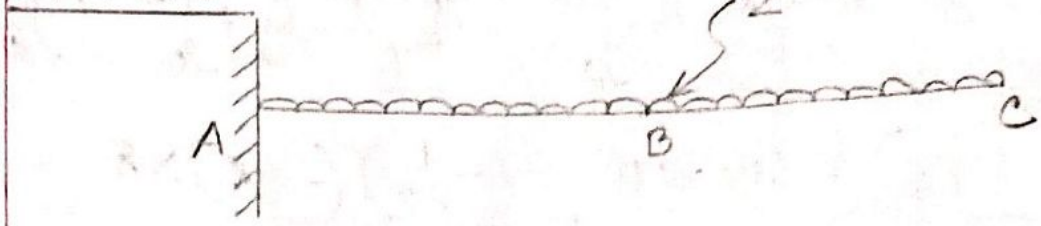
$$\sum Q \cdot \theta_{CL} = \sum \frac{F_B F_P L}{AE} + \int_A^B \frac{M_B M_P}{EI} dy + \int_C^B + \int_C^D + \int_E^D$$

$$\begin{aligned} \therefore \theta_{CL} &= \frac{\left(\frac{1}{16}\right)(-60) \times 10}{20 \times 30000} + \frac{\left(-\frac{1}{20}\right)(-48) \times 8}{20 \times 30000} \\ &+ \frac{\left(-\frac{1}{20}\right)(-48) \times 8}{20 \times 30000} + \frac{\left(-\frac{1}{16}\right)(-60)}{20 \times 30000} \\ &+ \int_0^{10} \frac{\frac{48}{20} y^2}{EI} dy + \int_0^8 \frac{\left(-1 + \frac{1}{12} y\right)(-60x)}{EI} dx \\ &+ \int_0^8 \frac{\frac{60}{16} x^2}{EI} dx + \int_0^{10} \frac{\frac{48}{20} y^2}{EI} dy \end{aligned}$$

$$\therefore \theta_{CL} = 7.75 \times 10^{-6} + \frac{1}{30000 \times \frac{2500}{144}} \times \frac{9920}{3}$$

$$\theta_{CL} = 6.3566 \times 10^{-3} \text{ radian (clockwise)}$$

Problem



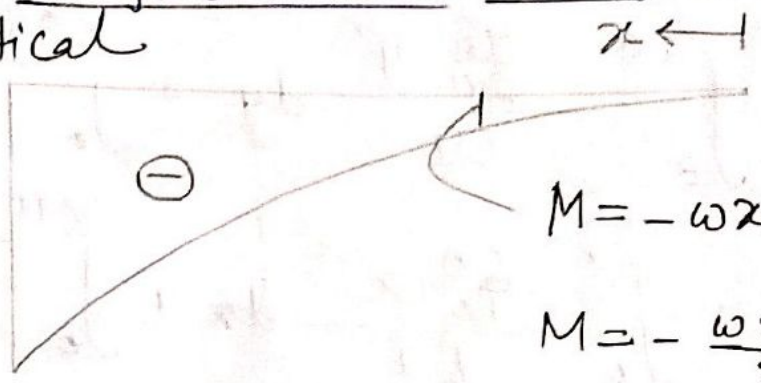
$E = 30000 \text{ ksi}$

$I = 200 \text{ in}^4$



P-force: Moment Diagram

Compute vertical Reaction at point B



$M = -\omega x \cdot \frac{x}{2}$

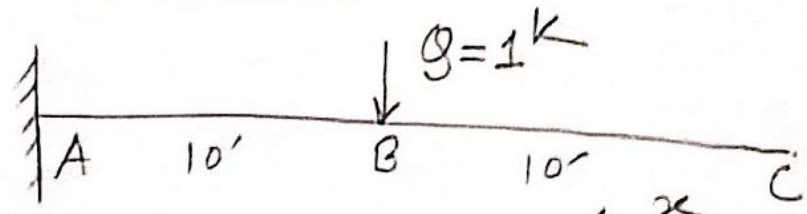
$M = -\frac{\omega x^2}{2}$

$= -\frac{2x^2}{2}$

is valid $\rightarrow M = -x^2$

$\int_0^{10} + \int_{10}^{20}$

Q-force Analysis



Moment Diagram



x axis start from C
 $M = 0$ (from $0 < x < 10$)
 $M = -x + 10$ (from $10 < x < 20$)

✖

From B to A ($10 < x \leq 20$)

$$M_Q = -x + 10$$

$$M_P = -x^2$$

By Virtual Work Method,

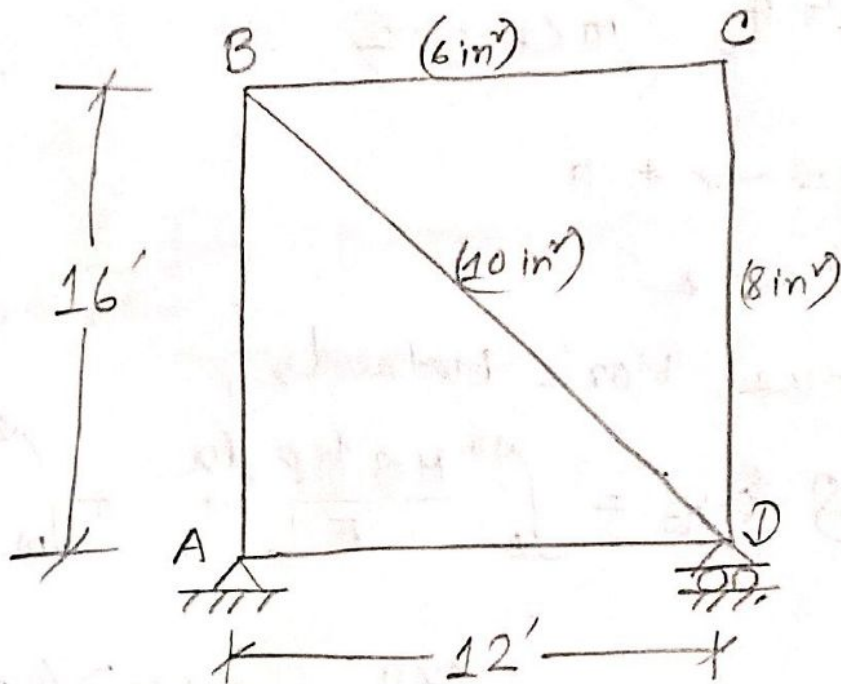
$$\sum Q \delta_{VB} = \int_0^{10} \frac{M_Q M_P dx}{EI} + \int_{10}^{20} \frac{M_Q M_P dx}{EI}$$

$$\therefore 1 \delta_{VB} = \int_{10}^{20} \frac{(-x+10)(-x^2)}{EI} dx$$

$$\delta_{VB} = \int_0^{20} \frac{(x^3 - 10x^2)}{EI} dx$$

Special Truss Problem

Truss with lack of fit or Fabrication Defects



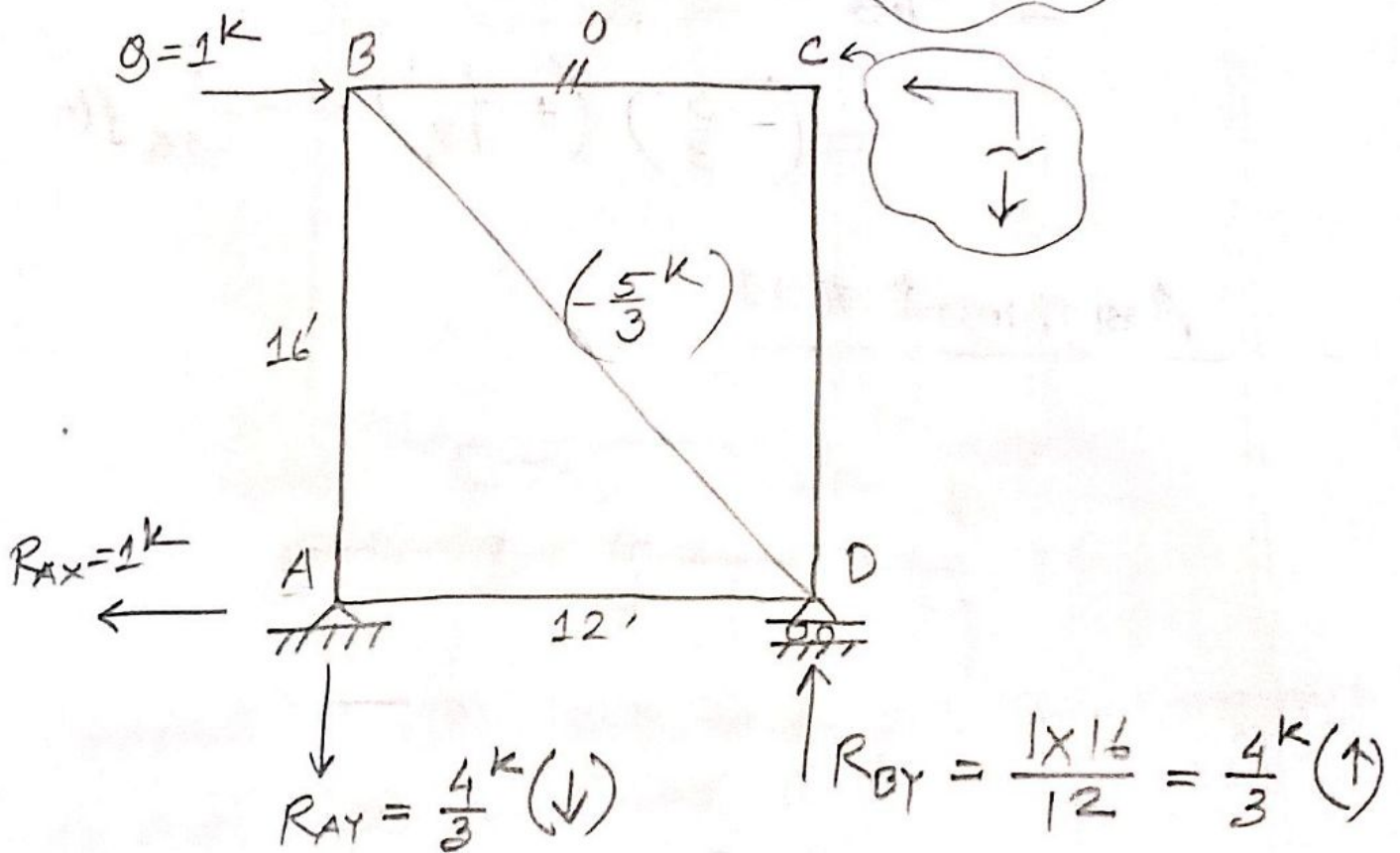
Problem: Calculate δ_{HB} & δ_{vB} if member BD is fabricated 1" larger than specified.

By Virtual Work Method,

$$\sum \delta \delta_{HB} + \cancel{WR} = \sum \frac{F_g F_p L}{AE} + \cancel{F_g \delta t} tL$$
$$= \sum F_g \Delta L$$

$$\Delta L = \frac{PL}{AE}$$

S-Force Analysis



$$(F_{BD})_v = \frac{4}{3}k$$

$$\therefore F_{BD} = \frac{\frac{4}{3}}{16} \times \sqrt{16^2 + 12^2}$$

$$= \frac{\frac{4}{3}}{16} \times 20$$

$$= \frac{5}{3}k \quad (-)$$

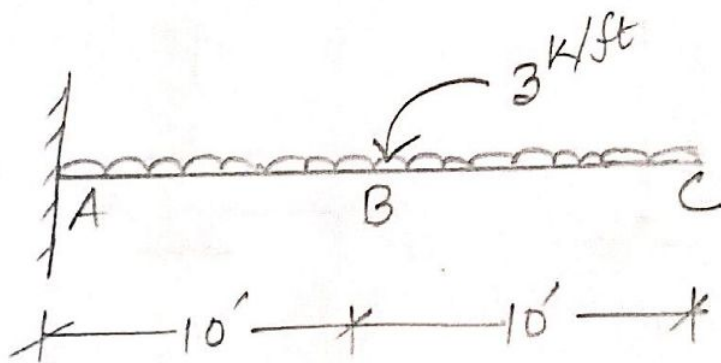
Compression

For member AB, BC, CD and AD; $\Delta L = 0$

$$1. \delta_{HB} = \sum F_g \Delta L$$

$$= \left(-\frac{5}{3}\right) \left(+\frac{1}{12}\right) = -\frac{5}{36} \text{ ft}$$

Assignment # 11

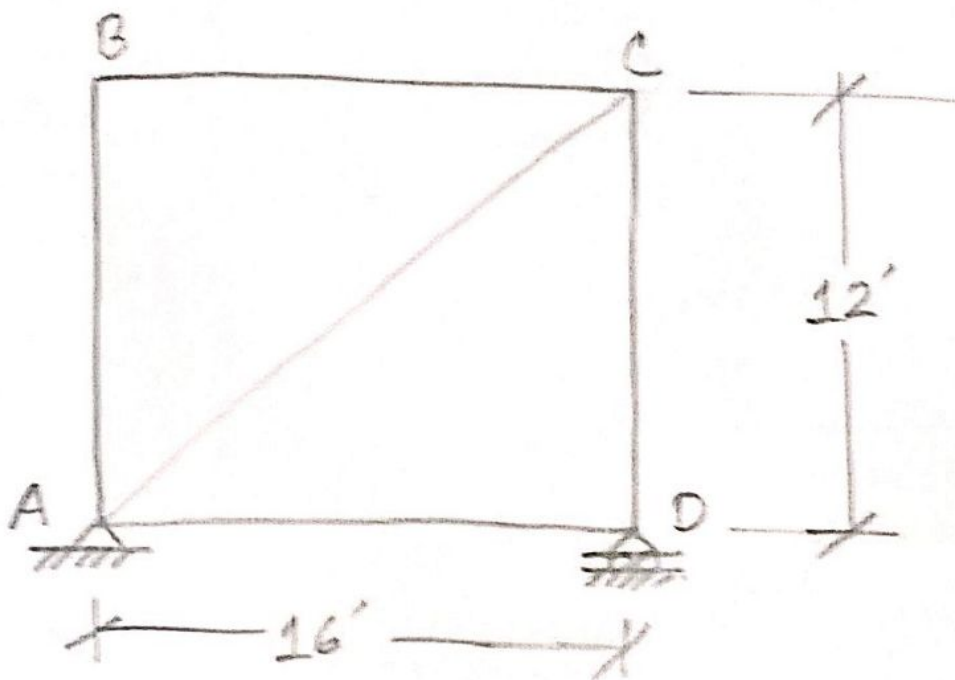


$$E = 30,000 \text{ ksi}$$

$$I = 300 \text{ in}^4$$

Compute change in slope at point 'B'.

Assignment # 12



Compute horizontal and vertical component of deflection at point 'B'.

If member AC is fabricated 2" ~~longer~~ larger and member CD is fabricated 0.5" shorter than specified.

Maximum Reaction for a series of moving Loads

Influence Lines for Reaction, Shear, Moment etc.

→ is drawn for a moving load of 1 unit.

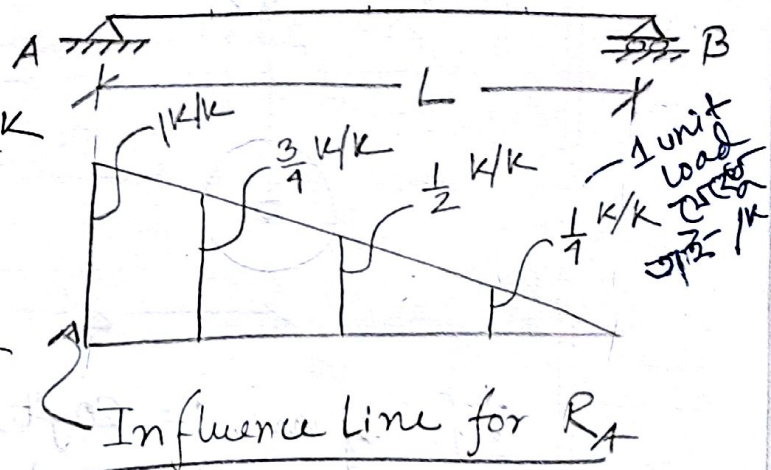
$R_A = ?$ for different position of Unit load (1k)

When unit load 1k at,
 $x = \frac{L}{4}$ ($\tan \theta$), $R_A = \frac{1}{4}k$

When unit load 1k at,
 $x = \frac{L}{2}$, $R_A = \frac{1}{2}k$

When unit load 1k at,
 $x = \frac{3L}{4}$, $R_A = \frac{3}{4}k$

When unit load 1k at,
 $x = L + \tan \theta$, $R_A = 1k$

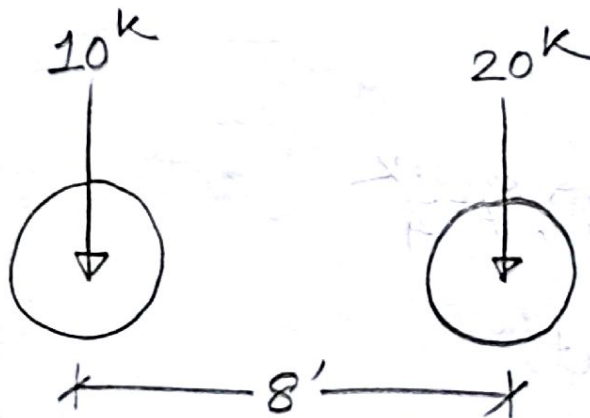


If a wheel load of 10^k is placed at $x = \frac{L}{2}$, $R_A = ?$

From IL diagram, $R_A = \frac{1}{2} \frac{k}{k} \times 10^k = 5^k$.

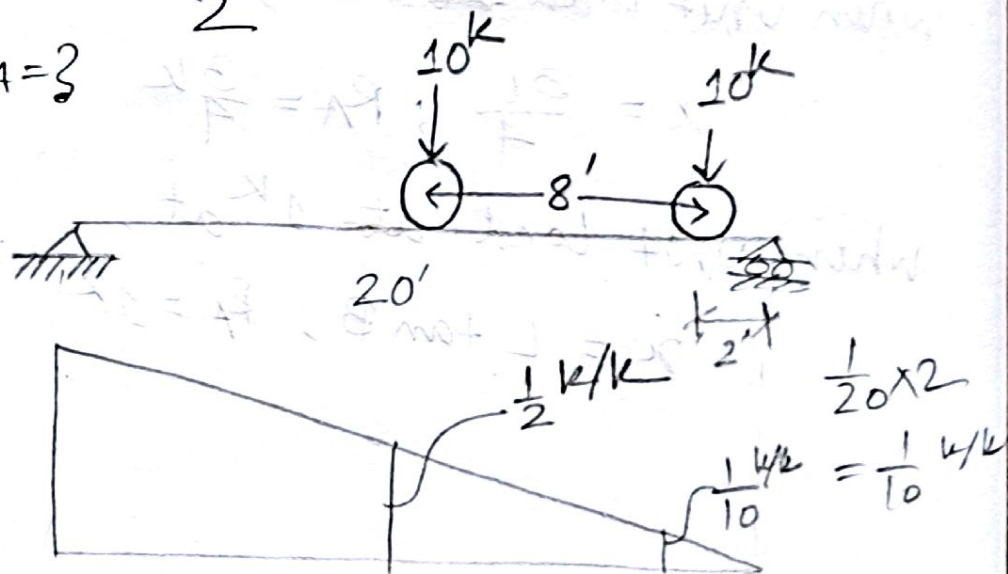
Example

vehicle with two wheels



If, span, $L = 20$ ft, $R_A = ?$, if wheel ① is placed at $x = \frac{L}{2}$.

Maximum, $R_A = ?$



$$R_A = \frac{1}{2} \frac{k}{k} \times 10^k + \frac{1}{10} \frac{k}{k} \times 20^k = 5 + 2 = 7^k$$

~~L for R_A
 $\frac{1}{2} \frac{k}{k}$~~

If, wheel ① is placed at A, $R_A = 1^{k/k} \times 10^k + \left(\frac{1}{20} \times 12\right)^{k/k} \times 20^k = 22^k$

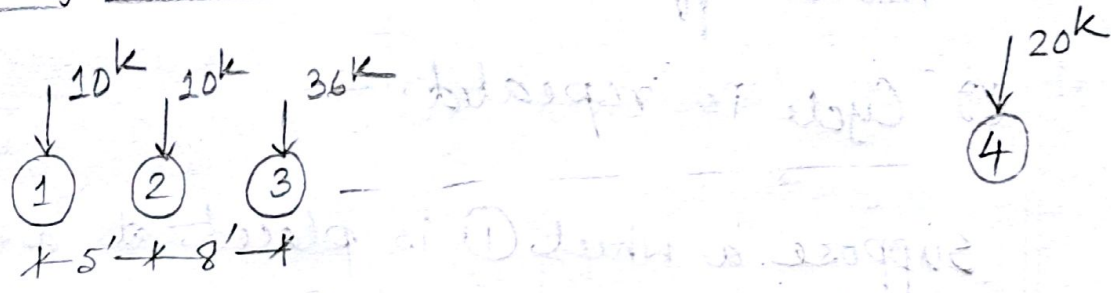
" " ② " " " " A, $R_A = 1^{k/k} \times 20^k = 20^k$

→ Maximum $R_A = 22^k$.

Shedd & Vawter

Article 160: Position of wheel to Produce Maximum Reaction

Wheel loads for a steam locomotive

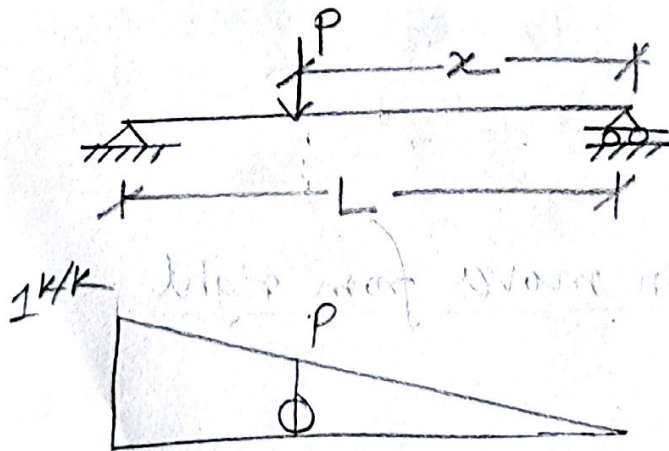


Suppose train moves from right

Observations

- (i) R_A increases till wheel ① comes in A.
- (ii) R_A suddenly decreases as wheel ① moves off the span.
- (iii) R_A increases as wheel ② approaches A.
- (iv) R_A suddenly decreases as wheel ② moves off the span.
- (v) Cycle is repeated.

Suppose a wheel ① is placed at a distance 'x' from Right support B.



$$R_A = \frac{1}{L} x \cdot P = \frac{Px}{L}$$

if P moves a distance ' d ' towards

$$R_A = \frac{P(x+d)}{L}$$

$$\begin{aligned} \text{Change in reaction, } \Delta R &= \frac{P(x+d)}{L} - \frac{Px}{L} \\ &= \frac{Pd}{L} \end{aligned}$$

If ' N ' no.s wheel have distance ' d ' on the span,

$$\Delta R = \sum \frac{Pd}{L}$$

In general, if wheel P_1 moves off the span and wheel P' travels a distance " e " in the span during movement of ' d_1 ' distance,

$$\text{Change in Reaction, } \Delta R = \frac{\sum P_1 y}{L} + \frac{P' e}{L} - P_1$$

where,

$\sum P$ = sum of wheels that stay on the span during "dy" movement.

P' = wheel that comes in the span

e = distance travelled by P'

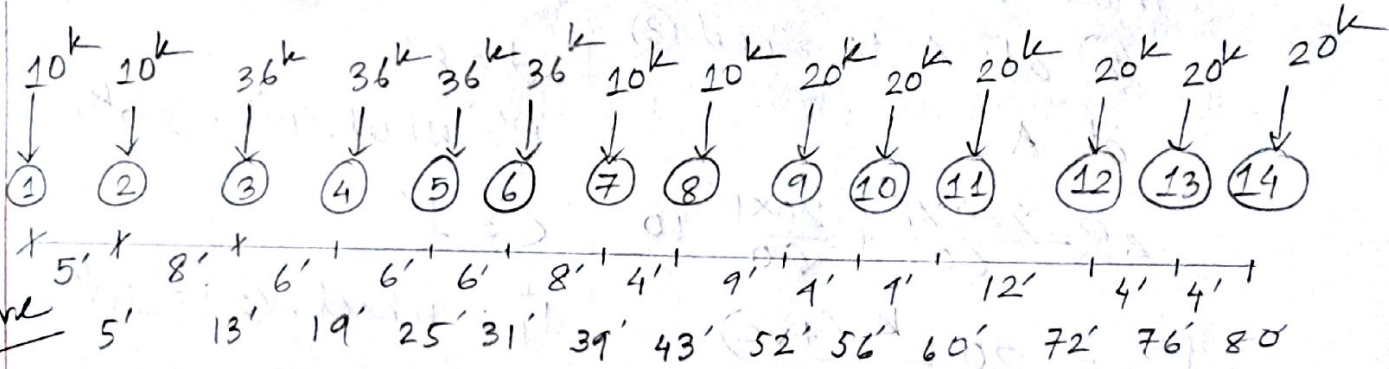
P_1 = wheel that moves off the span

Zakaria Sir-11

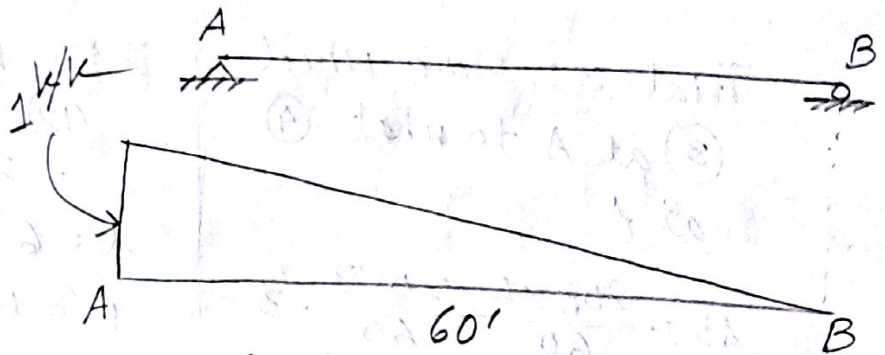
12 April 2015

Problem:

Steam Locomotive



IL for R_A



Given: Simple span of 60ft.
Find maximum reaction

Trial-1: Move wheel ① at A to wheel ② at A

$$\Delta R = \frac{\sum Pd}{L} + \frac{P'e}{L} - P_1$$

$$\text{or, } \Delta R = \frac{234 \times 5'}{60} + 0 - 10^k$$

$$= 9.5 \text{ (+ve)}$$

ΣP = Wheels they stay on span before and after movement

ΣP = wheel ② to wheel ⑪

$\Sigma P = 234^k$

$d = 5'$

$P' = 0$, $e = \text{No need}$

$P_1 = \text{wheel ①} = 10^k$

Trial-2: Move wheel
 ② at A to wheel ③
 at A

$$\Delta R = \frac{224 \times 8}{60} + \frac{20 \times 1}{60} - 10$$

$$= 20.2^k \text{ (+ve)}$$

Trial-3: Move wheel
 ③ at A to wheel ④
 at A

$$\Delta R = \frac{208 \times 6}{60} + \frac{20 \times 3}{60} - 36$$

$$= -14.2^k \text{ (+ve)}$$

$$\Sigma P = \text{Wheel } ③ \text{ to } ⑪ = 224^k$$

$$d = 8'$$

$$P' = \text{Wheel } ⑫ = 20^k$$

$$e = 1'$$

$$P_1 = \text{Wheel } ② = 10^k$$

$$\Sigma P = \text{Wheel } ④ \text{ to Wheel } ⑫$$

$$= 208^k$$

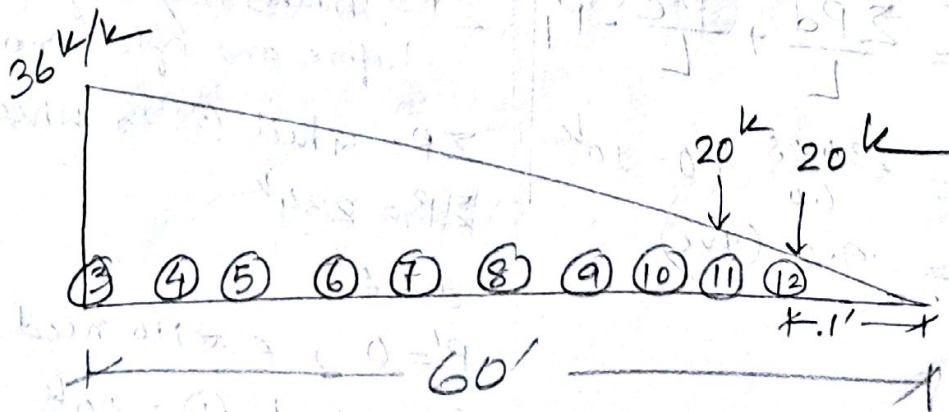
$$d = 6'$$

$$P' = \text{Wheel } ⑬ = 20^k$$

$$e = 3'$$

$$P_1 = \text{Wheel } ② = 36^k$$

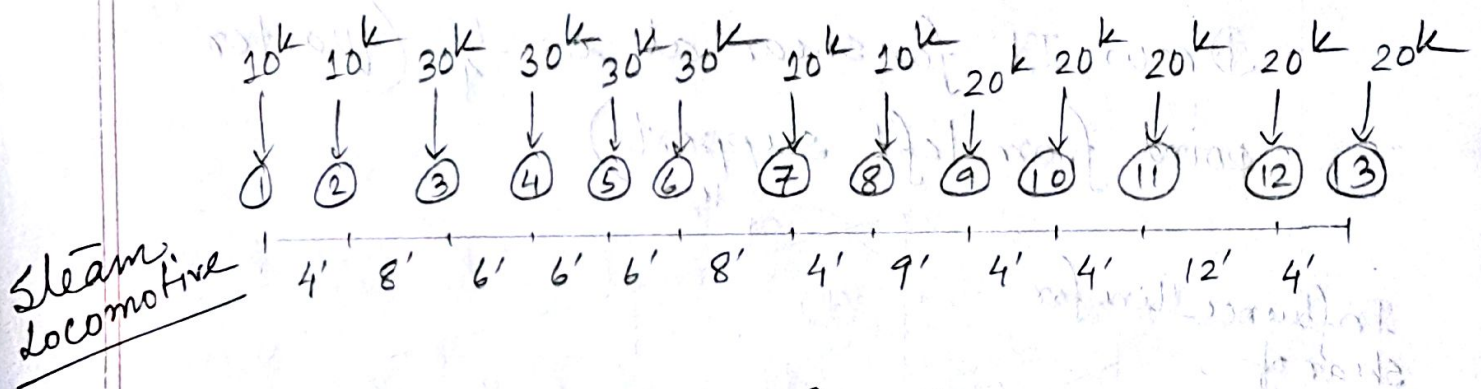
Wheel ③ at A gives maximum Reaction



$$\text{Maximum } R_A = \frac{1}{60} [60' \times 36^k + 54' \times 36^k + 48' \times 36^k + 42' \times 36^k + 34' \times 10^k + 30' \times 10^k + 21' \times 20^k + 17' \times 20^k + 13' \times 20^k + 1' \times 20^k]$$

$$R_A = 150.4^k$$

Assignment # 13



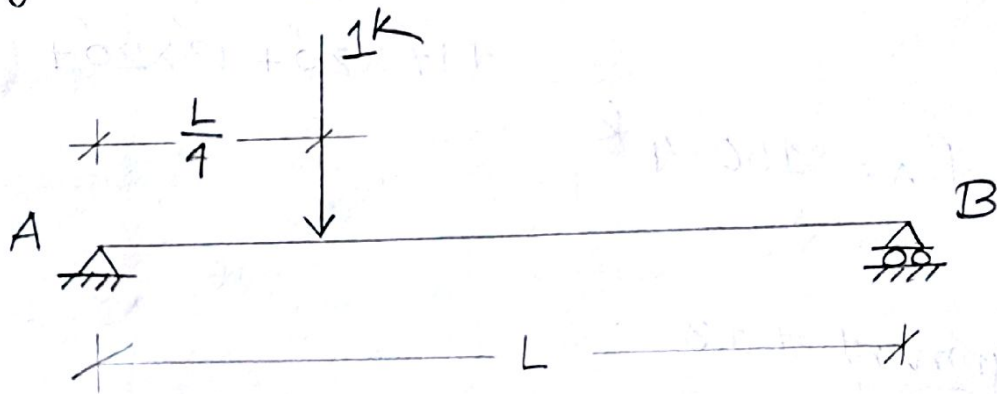
Given: Simple span of 70 ft.
Find maximum reaction

(i) $\frac{1}{2}$ span = 35 ft. \therefore about 1000 lbs reaction

(ii) $\frac{1}{3}$ span = 23.33 ft. \therefore reaction

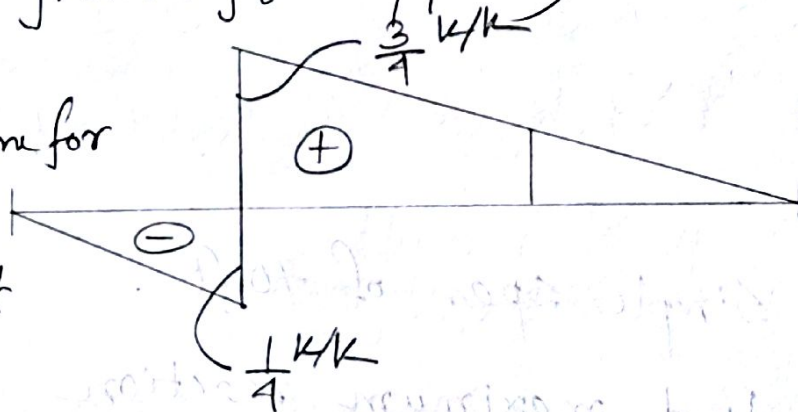
iteration 1: ...

Influence Line Diagram for Shear



Draw IL for shear at $x = \frac{L}{4}$ (quarter point from left support)

Influence line for shear of quarter point from left support.



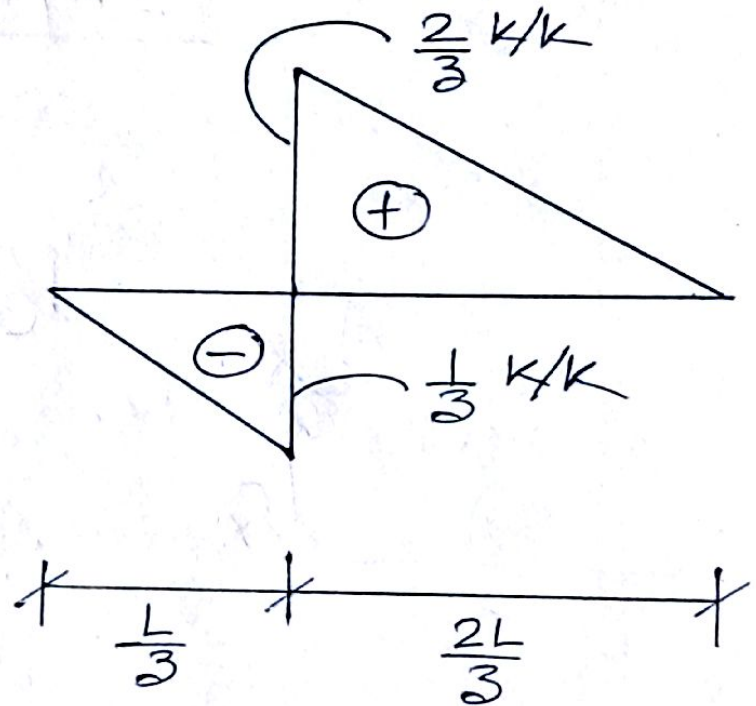
When unit load $1k$, just before $\frac{1}{4}$ (C^-)

$$\text{shear} = ? = -\frac{1}{4}k \text{ (-ve)}$$

When $1k$ just after $\frac{1}{4}$ (C^+)

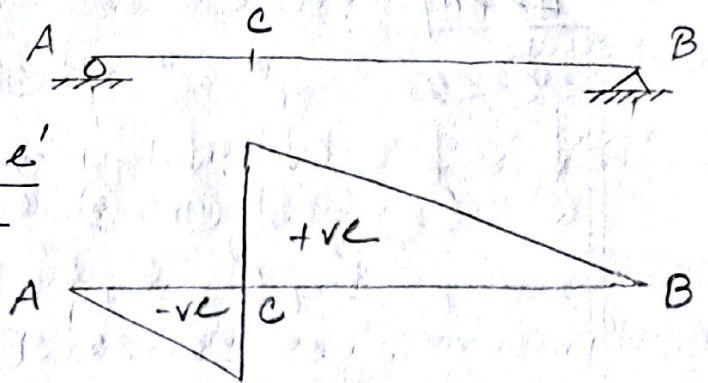
shear =

** If the load is at $\frac{L}{3}$ from left support
(IL for shear at third point from left support)



Change in shear,

$$\Delta V = \frac{\sum P d_i}{L} + \frac{P' e}{L} - P_1 + \frac{P_2 e'}{L}$$



$\sum P$ = Wheel loads, they stay on span before and after movement.

d_i = distance travelled by $\sum P$

P' = Load that comes on the span

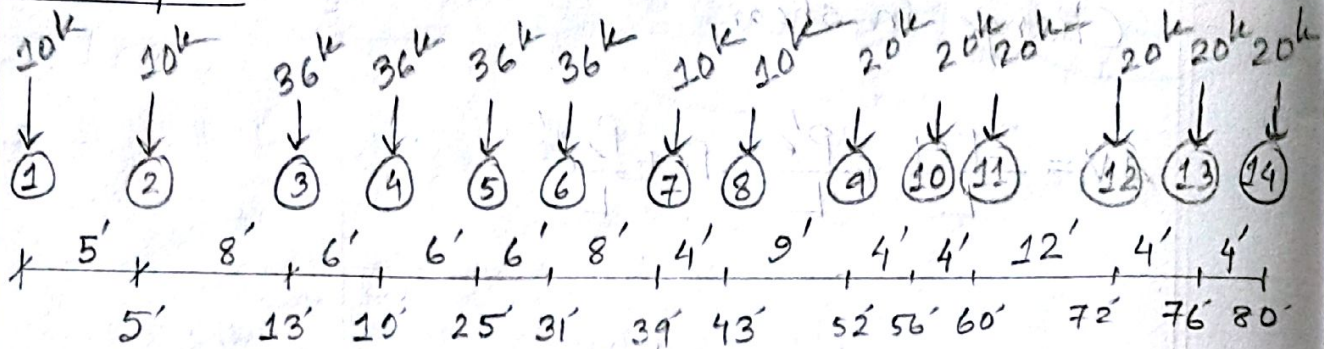
e = distance travelled by P'

P_1 = Load that crossed the control point "c"

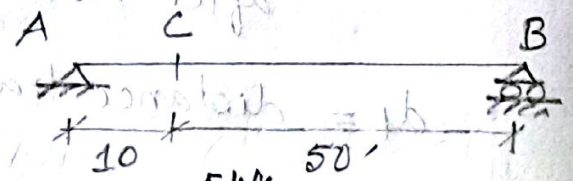
P_2 = load that moves off the span

e' = distance travelled by P_2 before moving off the span.

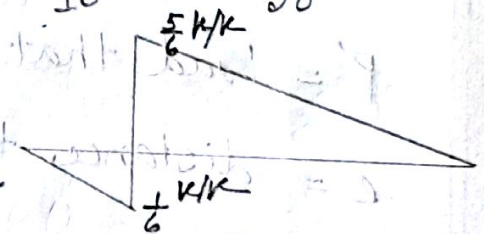
Example:



Find Maximum shear at 10ft from left support of 60' span.



IL for shear at C



Trial 1: Wheel ① at C to wheel ② at C,

$$\Delta V = \frac{\sum P d_1}{L} + \frac{P' e}{L} - P_1 - \frac{P_2 e'}{L}$$

$$\Rightarrow \Delta V = \frac{184 \times 5}{60} + \frac{20 \times 3}{60} - 10 + 0 = +6.33^k \text{ (ve)}$$

$$\sum P = \text{Wheel ① to wheel ⑧} = 184^k$$

$$d_1 = 5'$$

$$P' = \text{Wheel ④} = 20^k$$

$$e = 3'$$

$$P_1 = \text{Wheel ①} = 10^k$$

$$P_2 = 0$$

Trial 2: Wheel (2) at C to wheel (3) at C

$$\Delta V = \frac{194 \times 8}{60} + \frac{20 \times 7 + 20 \times 3}{60}$$

$$-10 + \frac{10 \times 5}{60}$$

$$\Delta V = +20.03 \text{ k (+ve)}$$

$$\Sigma P = \text{wheel (2) to wheel (1)}$$

$$= 194 \text{ k}$$

$$d_1 = 8'$$

$$P' = \begin{cases} \text{wheel (10)} = 20 \text{ k} \\ e_1 = 7' \\ \text{wheel (11)} = 20 \text{ k} \\ e_2 = 3' \end{cases}$$

$$P_1 = \text{wheel (2)} = 10 \text{ k}$$

$$P_2 = \text{wheel (1)} = 10 \text{ k}$$

$$e' = 5'$$

Trial 3: wheel (3) at C to wheel (4) at C

$$\Delta V = \frac{2 \times 24 \times 6}{60} + 0 - 36 + \frac{10 \times 2}{60}$$

$$= -13.26 \text{ k (-ve)}$$

$$\Sigma P = \text{wheel (3) to wheel (11)}$$

$$= 224 \text{ k}$$

$$d_1 = 6'$$

$$P' = 0$$

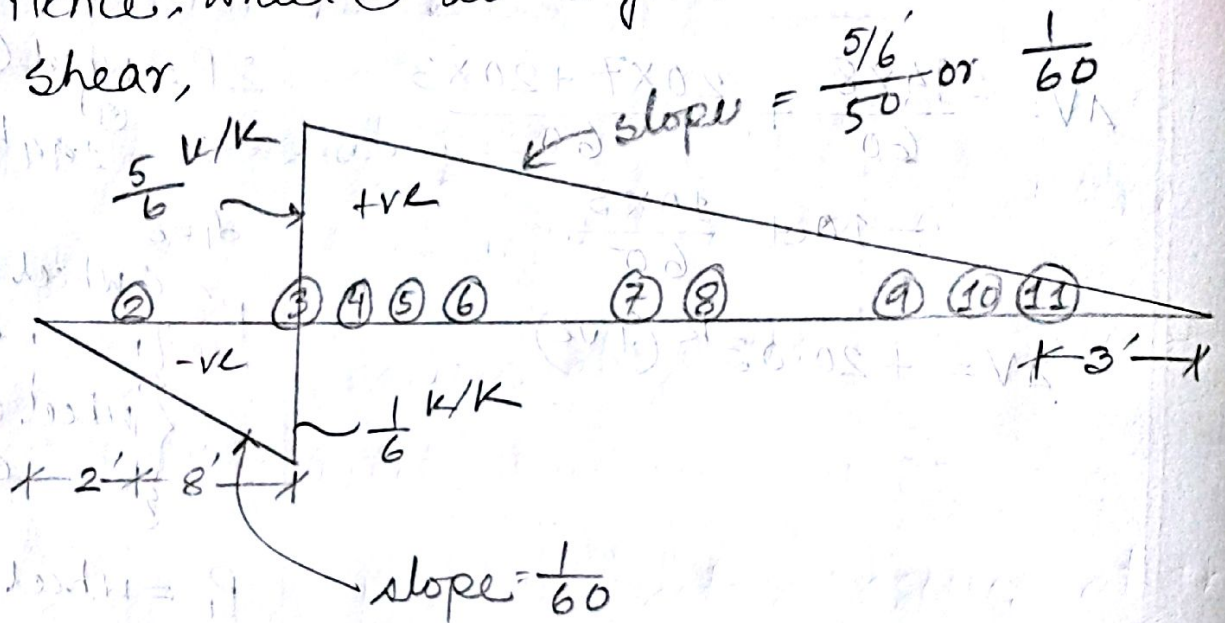
$$e = 0$$

$$P_1 = \text{wheel (3)} = 36 \text{ k}$$

$$P_2 = \text{wheel (2)} = 10 \text{ k}$$

$$e' = 2'$$

Hence, wheel ③ at C gives maximum shear,



$$\begin{aligned}
 V_{\max} &= \frac{1}{60} [50' \times 36^{\text{k}} + 44' \times 36^{\text{k}} + 38 \times 36^{\text{k}} + \\
 & 32 \times 36^{\text{k}} + 24 \times 10^{\text{k}} + 20 \times 10^{\text{k}} \\
 & + 11 \times 20^{\text{k}} + 7 \times 20^{\text{k}} + 3 \times 20^{\text{k}}] \\
 & - \frac{1}{60} [2' \times 10^{\text{k}}] \\
 & = 112.4^{\text{k}}
 \end{aligned}$$

Assignment 14

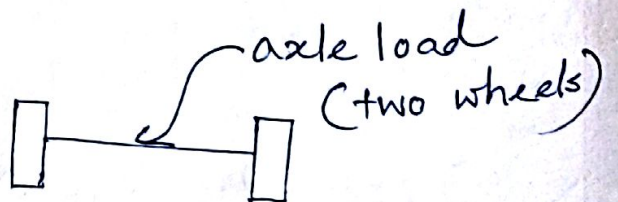
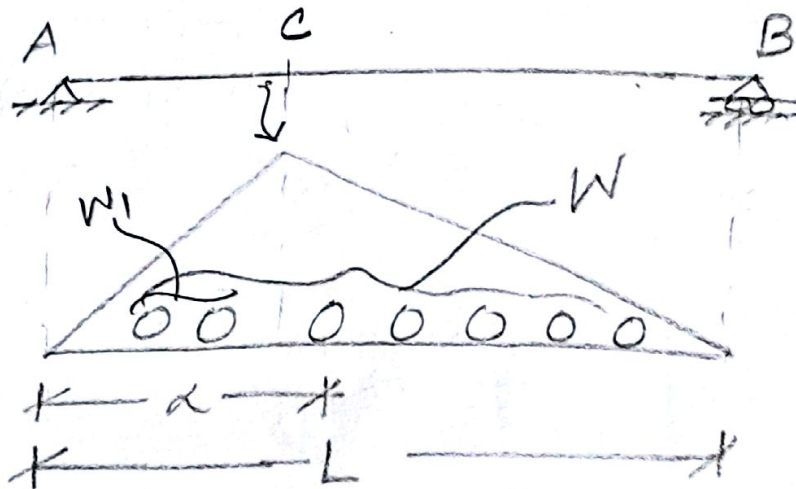
* $\frac{1}{4}$ load $\frac{1}{4}$ span but \max^{m} shear $\frac{1}{4}$ $\frac{1}{4}$ quarter point - 4.

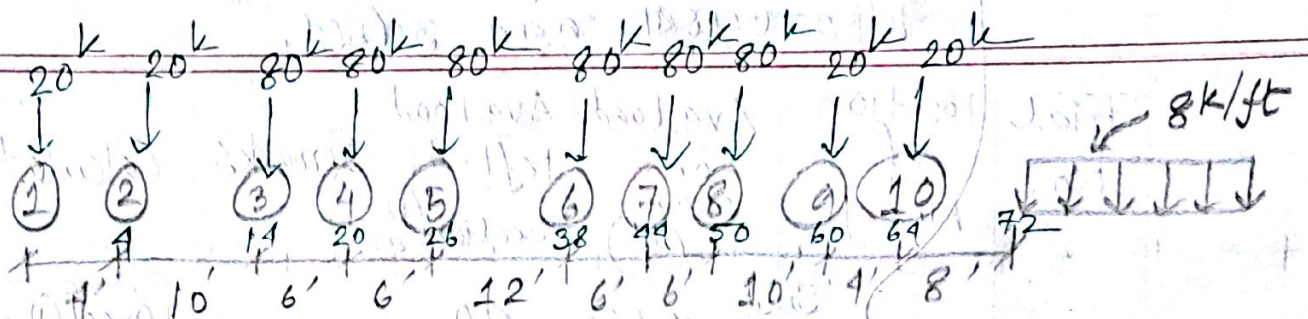
Article 65
(Shedd and Vawter)

Position of wheel to Produce Maximum Moment at a section ^(beam) Maximum Chord Stress of Truss

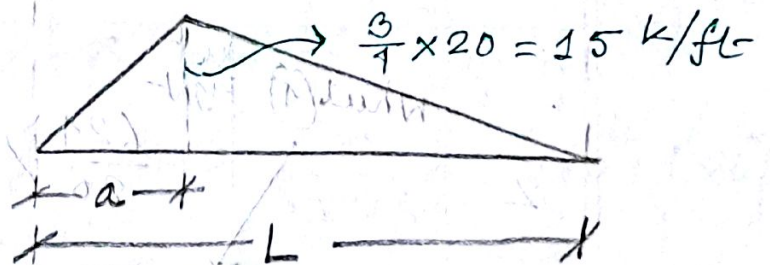
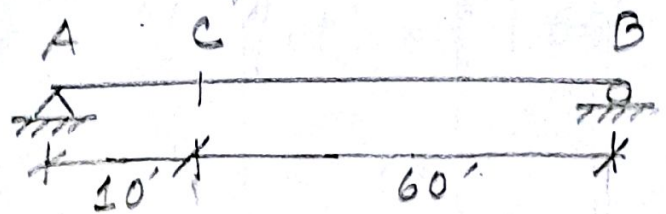
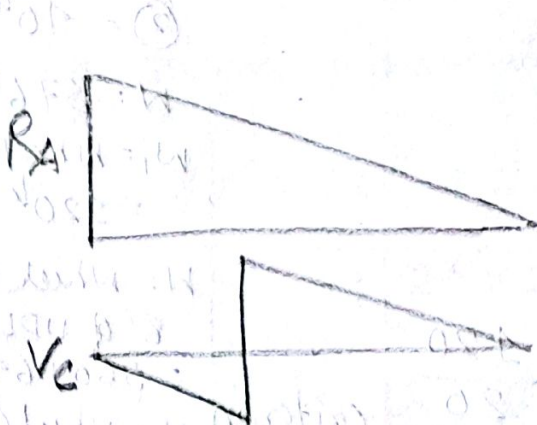
Criterion:

Criteria: $\frac{W}{L} = \frac{W_1}{a} =$ Average load on span = Average load left of section





Find Maximum moment at 20' from left support of 80' span for given axle load.



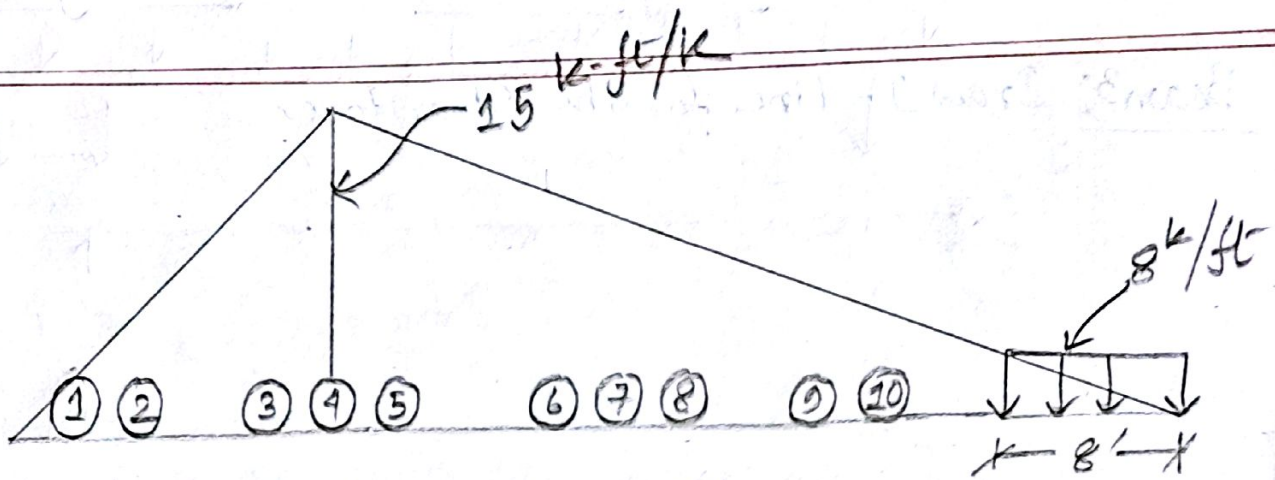
$$a = 20'$$

$$h = 80'$$

প্রথমে- start করা
 ২টি- maximum হয়, ২টি- মাত্র
 ৩টি- মাত্র করা হয় ৩টি- মাত্র

Trial No.	Position of wheel	Avg. Load on span $(\frac{W}{L})$	Avg. Load left to section $(\frac{W}{a})$	Remarks	Calculation
1	wheel (3)	Just Right $\frac{576}{80}$	$\frac{40}{20}$	criterion not satisfied	wheel ① + ⑩ + 2' of UDL $= 560 + 2 \times 8'$ $= 576k$ $W_1 = \text{wheel ① \& ②} = 40k$
		Just Left $\frac{576}{80}$	$\frac{120}{20}$		$W = 576k$ $W_1 = \text{wheel ① to ③} = 120k$
2	wheel (4)	Just Right $\frac{624}{80}$	$\frac{120}{20}$	criterion satisfied	$W = \text{wheel ① to ⑩} + 8' \text{ of UDL}$ $= 560 + 64 = 624k$ $W_1 = \text{wheel ① to ③} = 120k$
		Just Left $\frac{604}{80}$	$\frac{180}{20}$		$W = \text{wheel ② to ⑩} + 8' \text{ of UDL}$ $= 604k$ $W_1 = \text{wheel ② to ④}$ $= 20 + 160 = 180k$

Hence, wheel (4) at C gives maximum moment.

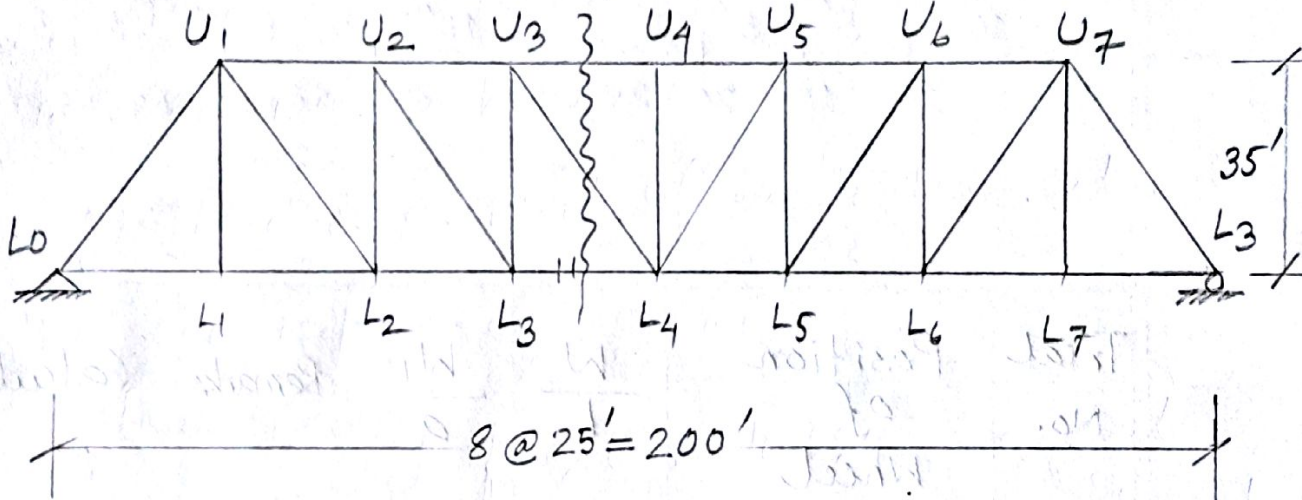


$$\begin{aligned}
 \text{Maximum Moment} &= \frac{15}{60} [60' \times 80^k + 54' \times 80^k \\
 &+ 42' \times 80^k + 36' \times 80^k + 30' \times 80^k \\
 &+ 20' \times 20^k \times 16' \times 20^k] \\
 &+ \frac{1}{2} \times \frac{15}{60} \times 8 \times 8 \times 8 + \frac{15}{20} [14' \times 80^k + 4' \times 20^k] \\
 &\text{area of diagram with UDL} \\
 &= 5584 \text{ k-ft for Axle Load}
 \end{aligned}$$

$$\text{or } \frac{5584}{2} = 2792^k \text{ for wheel load}$$

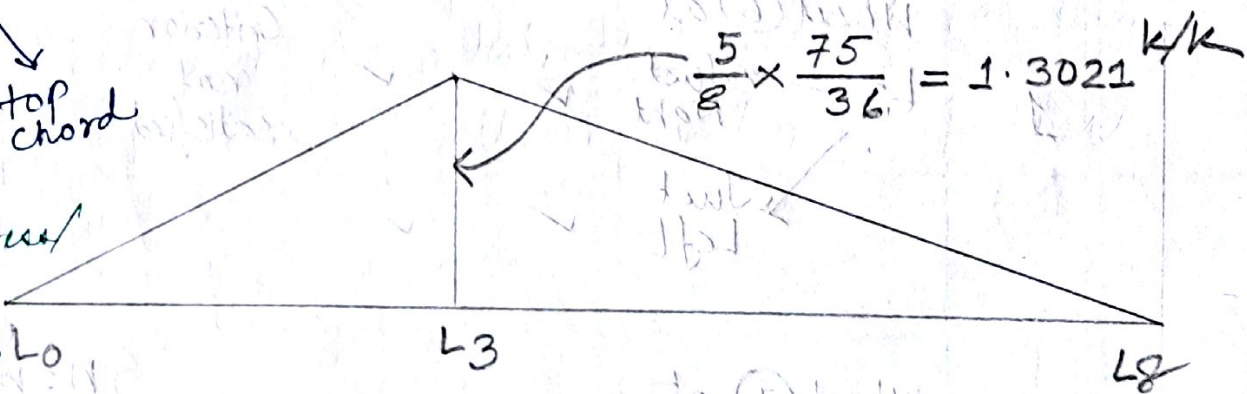
Assignment 15: (Load same)
 Calculate maximum moment at $\frac{1}{3}$ rd point of sample span of 90 ft

Example: Calculate Maximum stress in L_3L_4 of the truss shown.



chord member
 ↙ bottom chord
 ↘ top chord

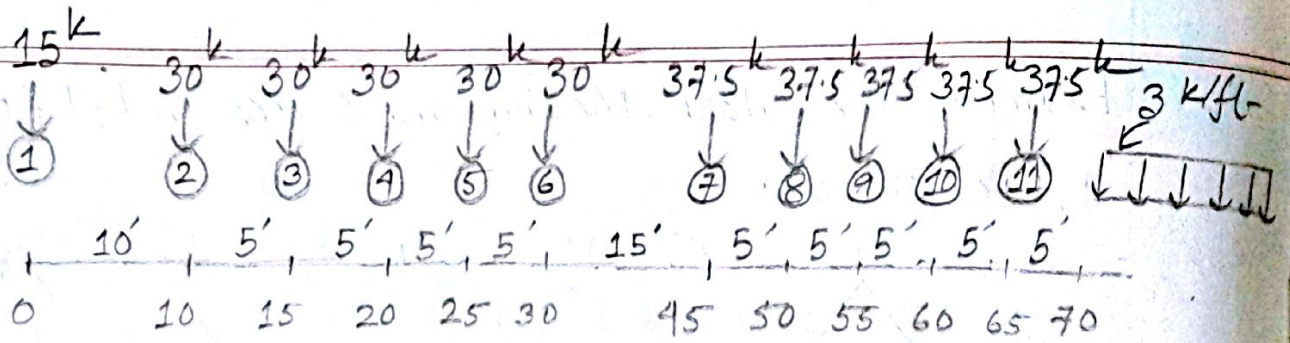
IL for stress/ bar forces of members $L_0L_3L_4$



$$F_{L_3L_4} = \frac{R_{L_3} \times 75'}{36}$$

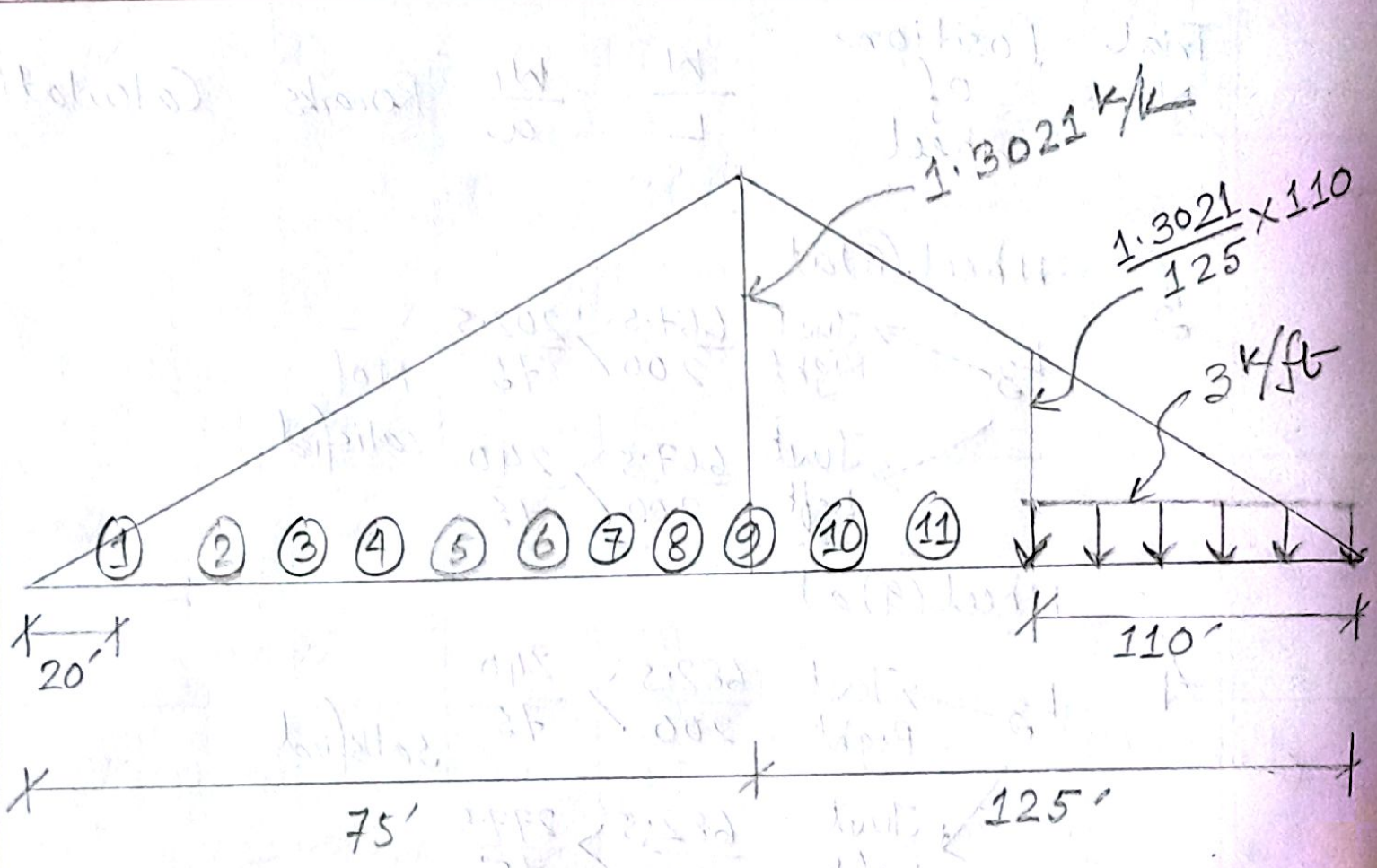
$$= \frac{\frac{5}{8} \times 75}{36}$$

$$= 1.3021 \text{ k/k}$$



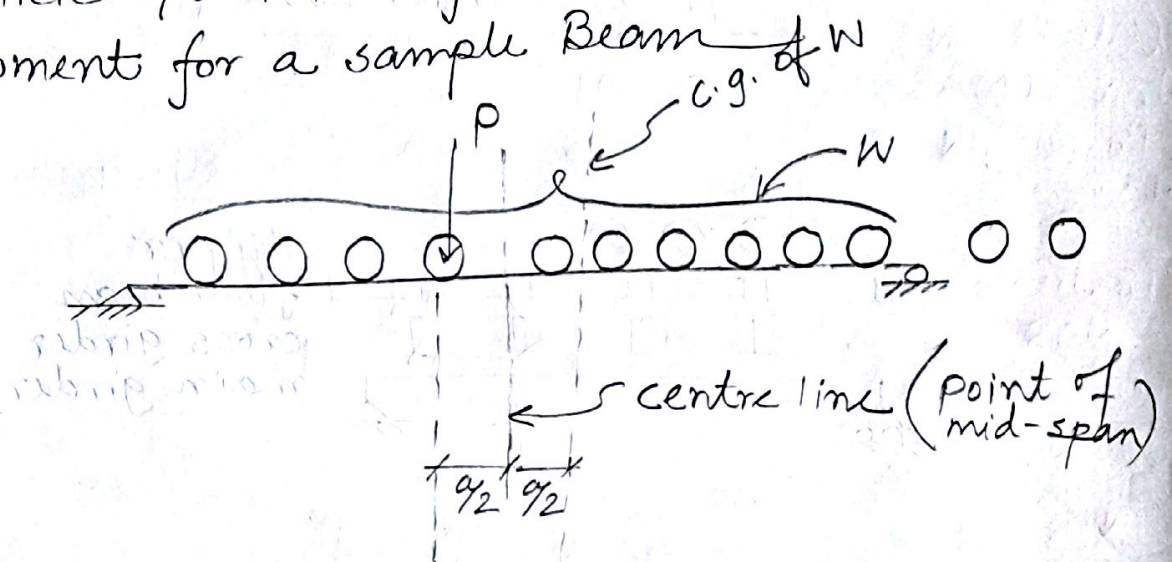
Trial No.	Position of Wheel	$\frac{W}{L}$	$\frac{W_1}{a}$	Remarks	Calculation
1	Wheel ⑥ at L_3 → Just Right ↘ Just Left	✓ ✓	✓ ✓	Criterion not satisfied	<p>Wider head</p> <p>↓</p> <p>not satisfied</p> <p>↓</p> <p>narrow head</p>
2	Wheel ⑦ at L_3 → Just Right ↘ Just Left	$\frac{652.5}{200}$	$\frac{165}{75}$ $\frac{202.5}{75}$		<p>$W = \text{Wheel ① to ⑪} + 100' \text{ of UDL}$ $= 652.5 \text{ k}$</p> <p>$W_1 = \text{Wheel ① to ⑥} = 165 \text{ k}$</p> <p>$W = 652.5 \text{ k}$</p> <p>$W_1 = \text{Wheel ① to ⑦} = 202.5 \text{ k}$</p>

Trial No.	Position of wheel	$\frac{W}{L}$	$\frac{W_1}{a}$	Remarks	Calculation
3	Wheel ⑧ at $L_3 \rightarrow$ Just Right \searrow Just Left	$\frac{667.5}{200}$	$\frac{202.5}{75}$	Not satisfied	
		$\frac{667.5}{200}$	$\frac{240}{75}$		
4	Wheel ⑨ at $L_3 \rightarrow$ Just Right \searrow Just Left	$\frac{682.5}{200}$	$\frac{240}{75}$	satisfied	
		$\frac{682.5}{200}$	$\frac{277.5}{75}$		



$$\begin{aligned}
 \text{Maximum stress/bar force} &= \frac{1.3021}{125} \left[125 * 37.5^k \right. \\
 &+ 120' * 37.5^k + 115' * 37.5^k \left. \right] + \frac{1}{2} \left(\frac{1.3021}{125} \times 110 \right) \times 110 \times 3^k \\
 &+ \frac{1.3021}{75} \left[70 \times 37.5 + 65 \times 37.5 + 50 \times 30 + 45 + 30 \right. \\
 &\quad \left. + 40 \times 30 + 35 \times 30 + 30 \times 30 + 20 \times 15 \right] \\
 &= 526.96^k \quad (\text{Tension})
 \end{aligned}$$

Article 67: Point of Greatest or Absolute Moment for a sample Beam



W = weight of wheel on the span

P = wheel that gives maximum moment at midspan

a = distance between P & C.G. of W

Procedure:

(1) Find the wheel (P) that gives maximum moment at midspan-section

(2) Place this wheel " P " & C.G. of W (loads on the span) equidistant ($\frac{a}{2}$) from mid-section.

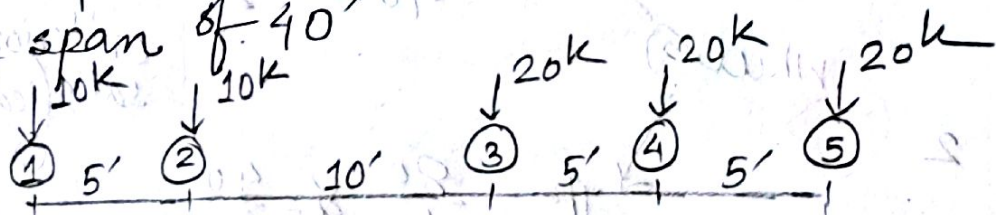
(3) Check criterion for maximum moment $(\frac{W}{L} - \frac{W}{a'})$ at the section under load "P".

If OK, calculate greatest or Absolute moment.

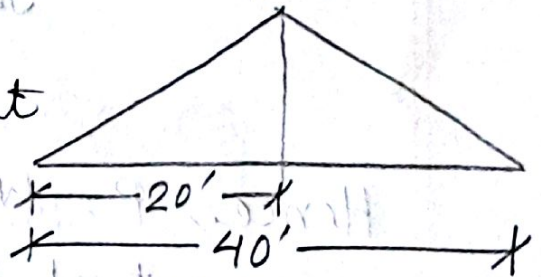
If Not, try with next wheel.

Example

Find Greatest or Absolute moment for a sample span of 40'

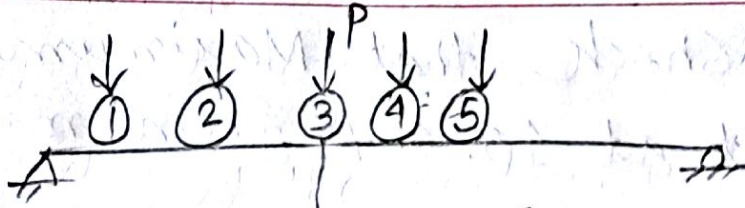


Step-1: Find the wheel that gives maximum moment at Mid-section

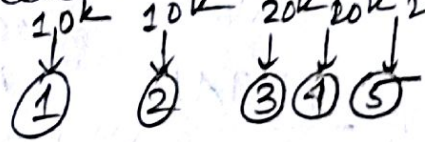


<u>TRIAL</u>	<u>Position of wheel</u>	$\frac{W}{L}$	$\frac{W_1}{a'}$	<u>Remarks</u>	<u>CALCULATIONS</u>
1	wheel ② at midspan	JR	$\frac{60}{40} > \frac{10}{20}$	Criterion not ok	$\left\{ \begin{array}{l} W = \text{wheel ① to ④} = 60k \\ W_1 = \text{wheel ①} = 10k \end{array} \right.$
		JL	$\frac{80}{40} > \frac{20}{20}$		
2	wheel ③	JR	$\frac{80}{40} > \frac{20}{20}$	Criterion satisfied	$\left\{ \begin{array}{l} W = 80k \\ W_1 = 20k \end{array} \right.$
		JL	$\frac{80}{40} \neq \frac{40}{20}$		

Hence, $P = \text{wheel } \textcircled{2} \textcircled{3}$ that gives maximum moment at mid-section



Step-2: Calculate CG of wheel on the span



Distance of CG from wheel ⑤

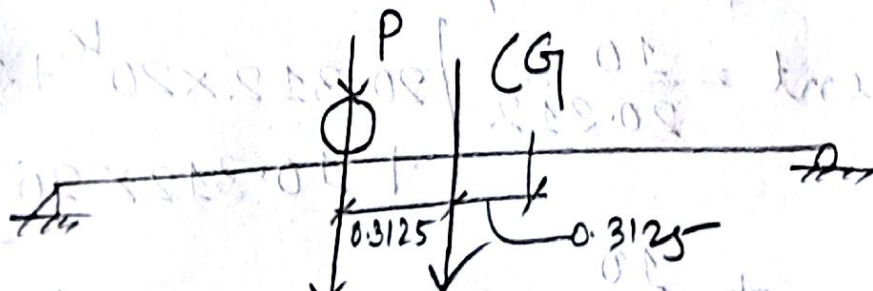
$$X = \frac{20 \times 0 + 20 \times 5 + 20 \times 10 + 10 \times 20 + 10 \times 25}{20 + 20 + 20 + 10 + 10}$$

$$= 9.375' \text{ (from wheel ⑤)}$$

a = distance between P & CG

$$= 10 - 9.375$$

$$= 0.625'$$

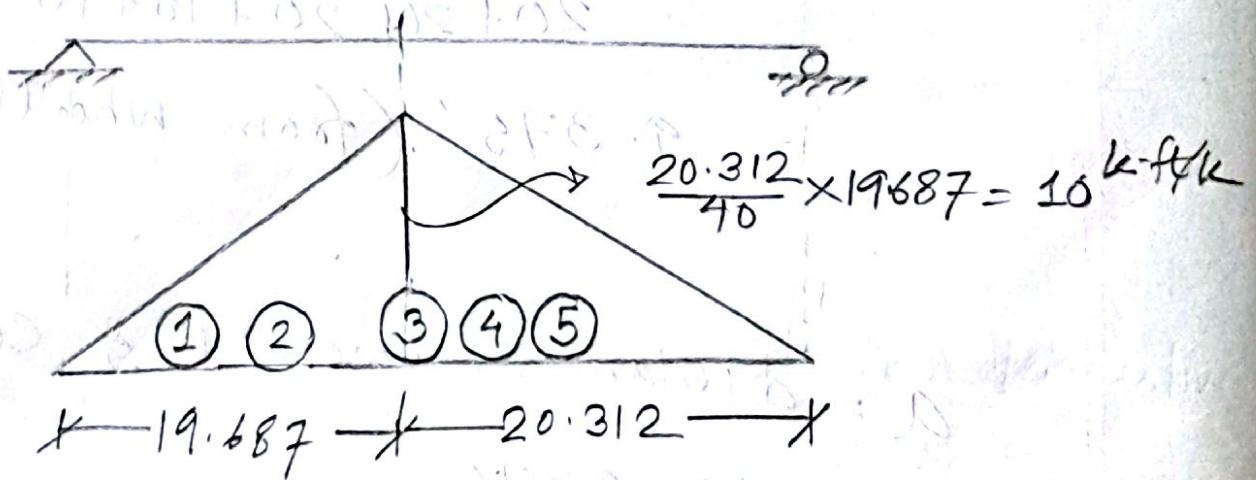


$$20 - 0.3125$$

$$= 19.6875$$

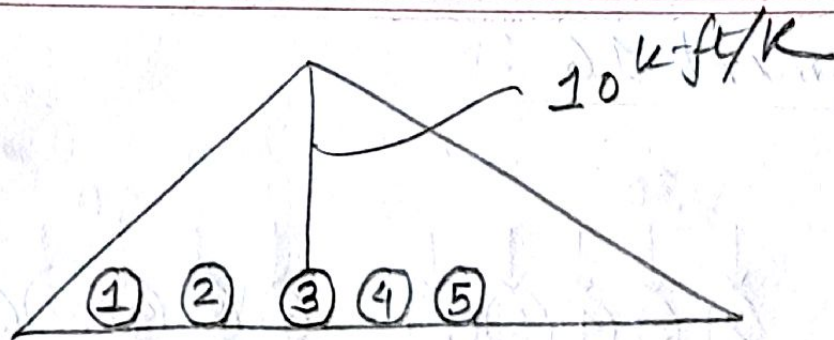
Step-3: Check that Maximum Moment occur at at 19.687' from mid-span.

<u>TRIAL</u>	<u>Position of wheel</u>	$\frac{W}{L}$	$\frac{W_1}{a'}$	<u>Remarks</u>
1	Wheel ③ → JR ↘ JL	$\frac{80}{40}$	$> \frac{20}{20}$	Criterion satisfied
		$\frac{80}{40} = \frac{40}{20}$		



Hence, Greatest/Absolute

$$\begin{aligned}
 \text{moment} &= \frac{10}{20.312} \left[20.312 \times 20^k + 15.312 \times 20 \right. \\
 &\quad \left. + 10.312 \times 20 \right] \\
 &\quad + \frac{10}{19.687} \left[9.687 \times 10 + 4.687 \times 10 \right] \\
 &= 525.317 \text{ k-ft} > 525
 \end{aligned}$$



Maximum Moment at Mid-Section

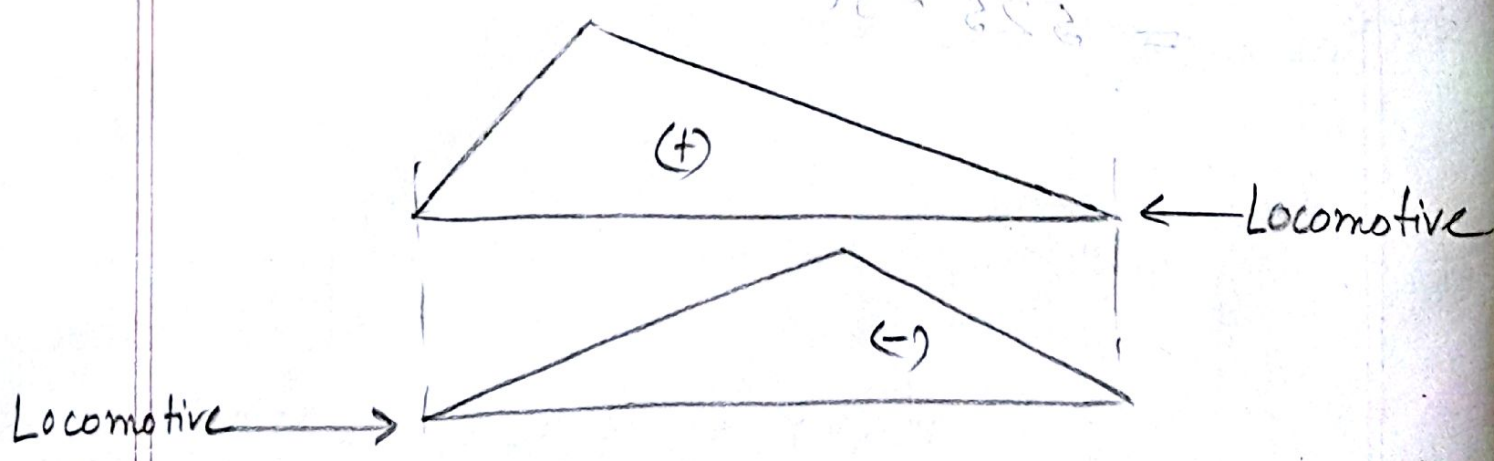
$$\begin{aligned}
 &= \frac{10}{20} [20 \times 20 + 15 \times 20 + 10 \times 20] \\
 &\quad + \frac{10}{20} [10 \times 10 \times 5 \times 10] \\
 &= 525 \text{ k-ft}
 \end{aligned}$$

Assignment 16 :



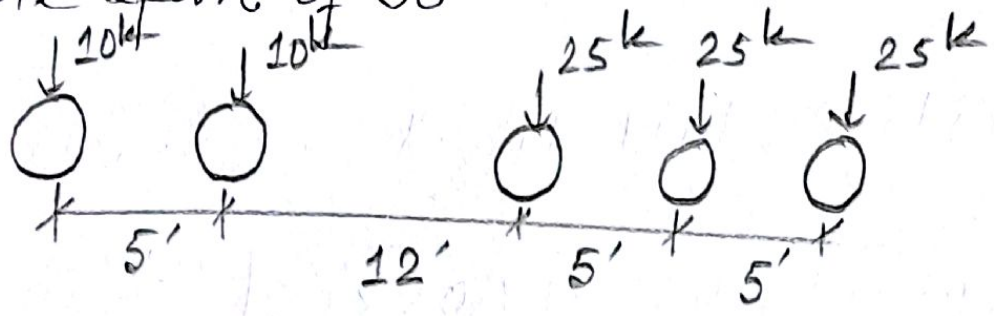
Find U4U5, distance same

Result: compression 20, locomotive may need to be moved from left to right.



Assignment 17:

Find absolute moment for a ~~span~~
simple span of 50'



Article 69

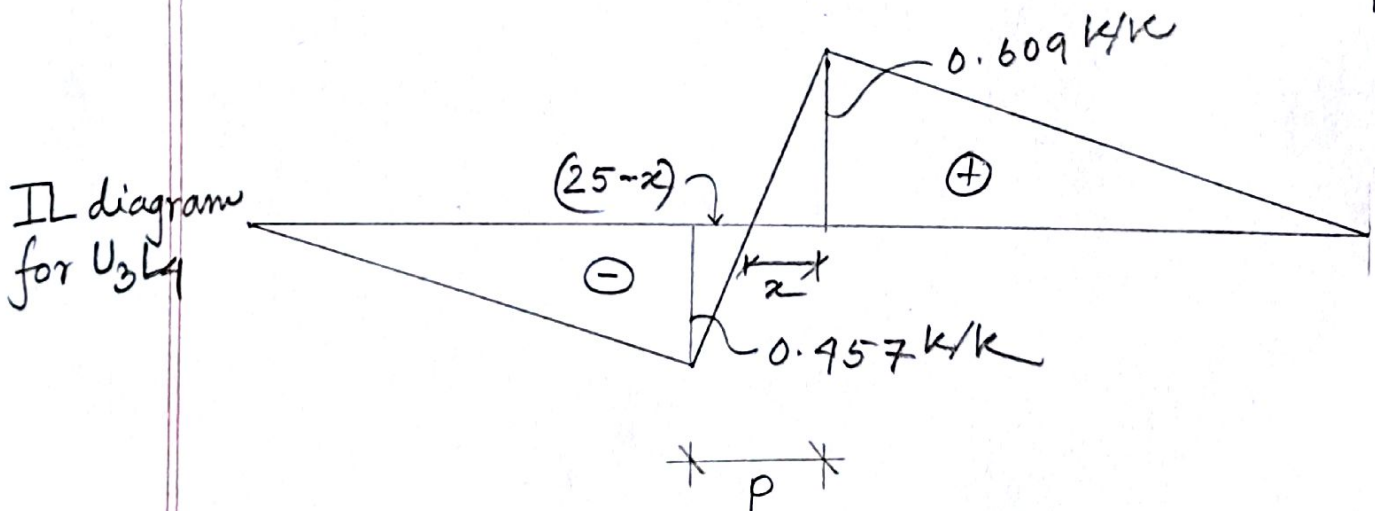
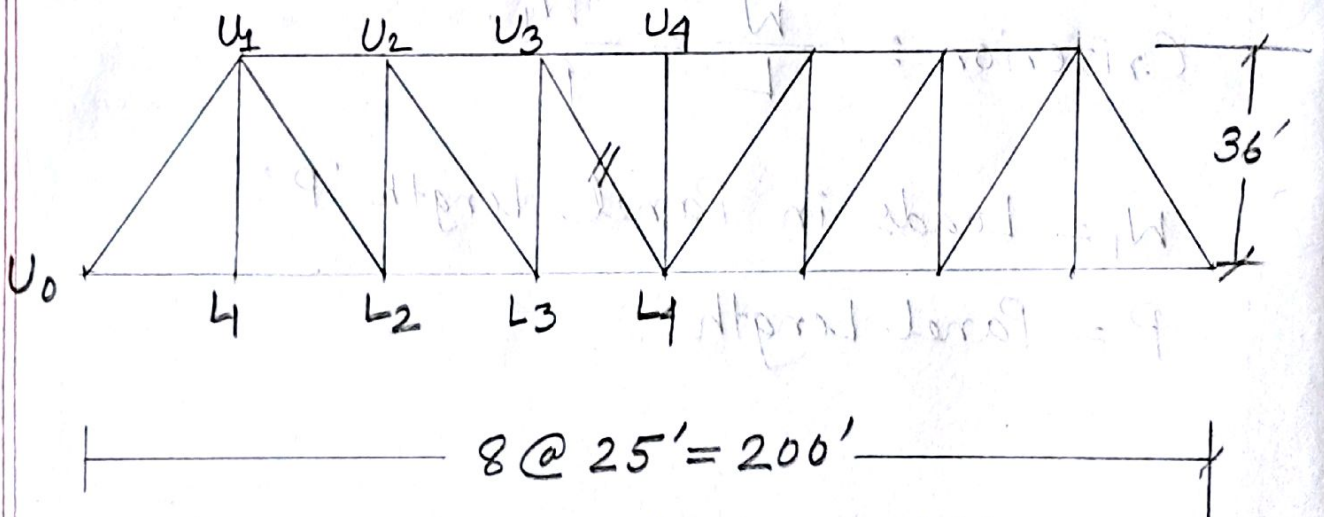
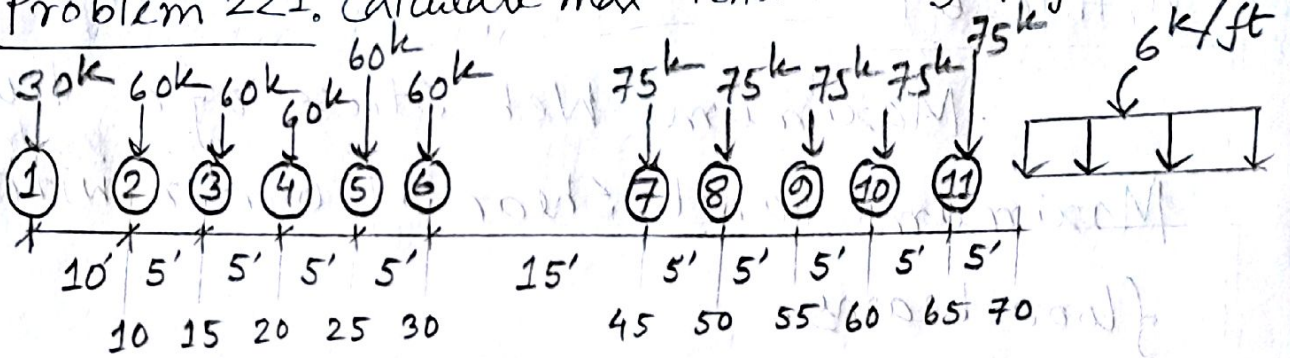
Maximum Web Stress of a Truss/
Maximum Panel Shear of Girder with
floor beams.

Criterion: $\frac{W}{L} = \frac{W_1}{P}$

W_1 = Loads in Panel length 'P'

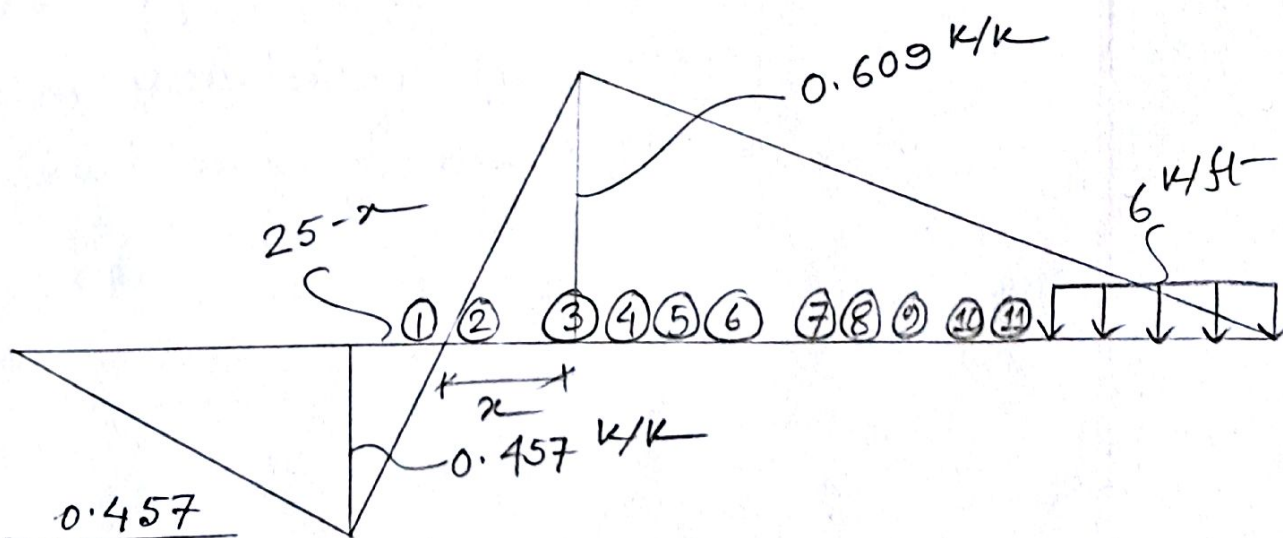
P = Panel Length

Problem 221: Calculate max^m tension in U_3L_4 of the beam:



- (1) For maximum Tension Load Moves from Right to Left
- (2) For maximum Compression Load Moves from L → R

Trial No.	Position of Wheel	$\frac{W}{L}$	$\frac{W_1}{P}$	Remarks	Calculation
1	Wheel (2) at L_1	$\begin{matrix} \nearrow R & \frac{945}{200} > \frac{30}{25} \\ \searrow L & \frac{945}{200} > \frac{90}{25} \end{matrix}$		criteria not ok.	$\begin{cases} W = \text{Wheel } \textcircled{1} \text{ to } \textcircled{11} \\ + 40' \text{ UDL} \\ = 945 \text{ k} \\ W_1 = \text{Wheel } \textcircled{1} = 30 \text{ k} \end{cases}$ $\begin{cases} W = 945 \text{ k} \\ W_1 = \text{Wheel } \textcircled{1} \text{ to } \textcircled{2} \\ = 90 \text{ k} \end{cases}$
2	Wheel (3) at L_1	$\begin{matrix} \nearrow R & \frac{975}{200} > \frac{90}{25} \\ \searrow L & \frac{975}{200} < \frac{150}{25} \end{matrix}$		criteria satisfied	$\begin{cases} W = \\ W_1 = \end{cases}$ $\begin{cases} W = \\ W_1 = \end{cases}$



$$\frac{0.609}{x} = \frac{0.457}{25-x}$$

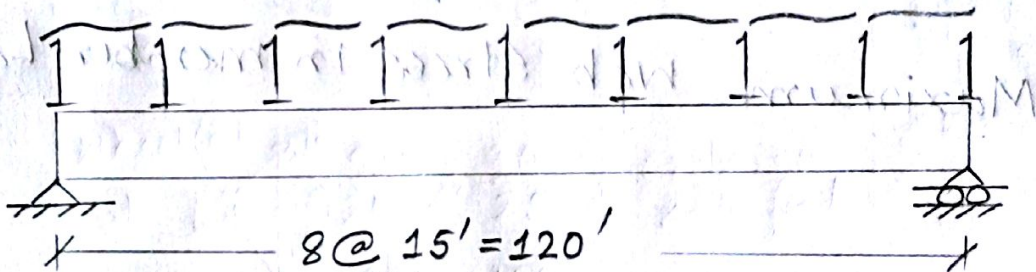
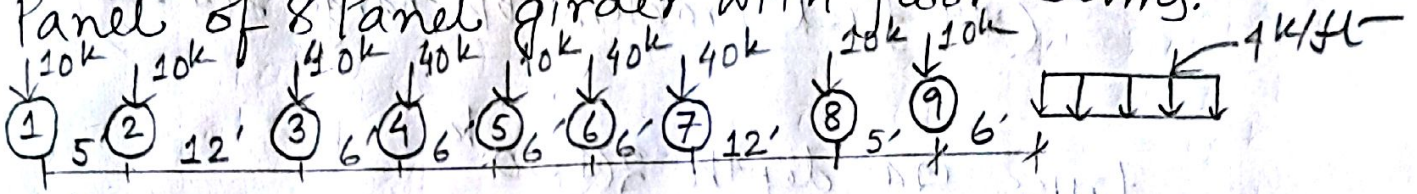
$$\therefore x = 9.28 \text{ ft}$$

Maximum Tension =

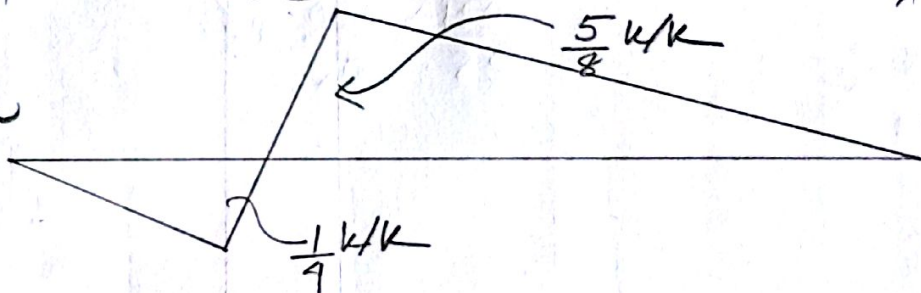
$$= 166.04 \text{ k}$$

Problem 134: Calculate Maximum shear in 3rd

Panel of 8 Panel girder with floor beams.



IL-diagram
for 3rd
panel



Maximum shear = +ve shear

Load move from Right to Left

→ Ans. 119.26k

Assignment 18:

(** first problem B.P. change 2015)

truss depth 30^k

60^k load 65^k

Maximum web stress in member L_4U_5

Assignment 19:

Girder with floor beam

(** second problem = $\frac{b}{d}$ change $\frac{2}{3}$)

panel length $\frac{2}{3}$ 18' (8 @ 18')

40k load $\frac{2}{3}$ $\frac{2}{3}$ $\frac{2}{3}$ - 45k load $\frac{2}{3}$ $\frac{2}{3}$ $\frac{2}{3}$