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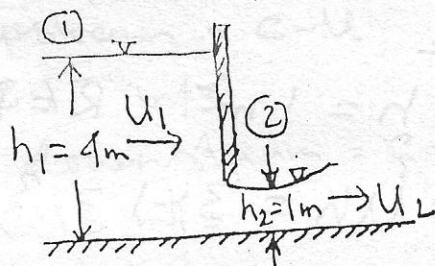
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OPEN CHANNEL FLOW

Solutions to Problems of Chap. 1

- 1.1(a) Rectangle $b = 6\text{ m}$
 $h_1 = 4\text{ m}$, $h_2 = 1\text{ m}$,
 $U_1 = 2\text{ m/s}$, $U_2 = 8\text{ m/s}$



Upstream

$$A_1 = bh_1 = 24\text{ m}^2, P_1 = b + 2h_1 = 14\text{ m}, R_1 = A_1/P_1 = 1.714\text{ m}$$

$$Re_1 = \frac{U_1 R_1}{\nu} = \frac{2 \times 1.714}{10^{-6}} = 3.43 \times 10^6 > 12,500$$

$$Fr_1 = \frac{U_1}{\sqrt{gD_1}} = \frac{U_1}{\sqrt{gh_1}} = \frac{2}{\sqrt{9.81 \times 4}} = 0.32 < 1$$

Hence, the flow upstream of the sluice gate is subcritical turbulent.

Downstream (at the vena contracta)

$$A_2 = bh_2 = 6\text{ m}^2, P_2 = b + 2h_2 = 8\text{ m}, R_2 = \frac{A_2}{P_2} = 0.75\text{ m}$$

$$Re_2 = \frac{U_2 R_2}{\nu} = \frac{8 \times 0.75}{10^{-6}} = 6 \times 10^6 > 12,500$$

$$Fr_2 = \frac{U_2}{\sqrt{gD_2}} = \frac{U_2}{\sqrt{gh_2}} = \frac{8}{\sqrt{9.81 \times 1}} = 2.55 > 1$$

Hence, the flow downstream of the sluice gate is supercritical turbulent.

(b) $A = 33,500\text{ m}^2$; $Q = 56,200\text{ m}^3/\text{s}$; $B = 3820\text{ m}$

The river is wide.

$$U = Q/A = 56200/33500 = 1.68\text{ m/s}$$

$$h = R = D = A/B = 33500/3820 = 8.77\text{ m}$$

$$Re = \frac{UR}{\nu} = \frac{Uh}{\nu} = \frac{1.68 \times 8.77}{10^{-6}} = 14.73 \times 10^6 > 12,500$$

$$Fr = \frac{U}{\sqrt{gD}} = \frac{U}{\sqrt{gh}} = \frac{1.68}{\sqrt{9.81 \times 8.77}} = 0.18 < 1$$

Hence, the state of flow is subcritical turbulent. ①

1.2 $h = 1\text{ m}$ $U = 3\text{ m/s}$

i) Wide

$$A = h = 1\text{ m}^2 \quad R = D = h = 1\text{ m}$$

$$Q = Uh = 3 \times 1 = 3\text{ m}^3/\text{s/m or m}^2/\text{s}$$

$$Re = \frac{UR}{\nu} = \frac{Uh}{\nu} = \frac{3 \times 1}{10^{-6}} = 3 \times 10^6 > 12,500$$

$$Fr = \frac{U}{\sqrt{gD}} = \frac{U}{\sqrt{gh}} = \frac{3}{\sqrt{9.81 \times 1}} = 0.96 < 1$$

Hence, the flow is subcritical turbulent.

$$c = \sqrt{gD} = \sqrt{gh} = \sqrt{9.81 \times 1} = 3.13\text{ m/s}$$

$$\therefore \text{Speed of wave front upstream} = c - U \\ = 3.13 - 3 = 0.13\text{ m/s}$$

$$\text{Speed of wave front downstream} = c + U \\ = 3.13 + 3 = 6.13\text{ m/s}$$

ii) Rectangle $b = 6\text{ m}$

$$A = bh = 6 \times 1 = 6\text{ m}^2 \quad P = b + 2h = 6 + 2 \times 1 = 8\text{ m}$$

$$R = A/P = 6/8 = 0.75\text{ m} \quad D = A/B = h = 1\text{ m}$$

$$Q = AU = 6 \times 3 = 18\text{ m}^3/\text{s}$$

$$Re = \frac{UR}{\nu} = \frac{3 \times 0.75}{10^{-6}} = 2.25 \times 10^6 > 12,500$$

$$Fr = \frac{U}{\sqrt{gD}} = \frac{U}{\sqrt{gh}} = \frac{3}{\sqrt{9.81 \times 1}} = 0.96 < 1$$

Hence, the flow is subcritical turbulent.

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$$c = \sqrt{gD} = \sqrt{gh} = \sqrt{9.81 \times 1} = 3.13 \text{ m/s}$$

$$\therefore \text{Speed of wave front upstream} = c - U$$

$$= 3.13 - 3 = 0.13 \text{ m/s}$$

$$\text{Speed of wave front downstream} = c + U$$

$$= 3.13 + 3 = 6.13 \text{ m/s}$$

iii) Trapezoid $b = 6 \text{ m}$ $s = 2$

$$A = (b + sh)h = (6 + 2 \times 1) \times 1 = 8 \text{ m}^2$$

$$P = b + 2\sqrt{1+s^2}h = 6 + 2\sqrt{1+2^2} \times 1 = 10.47 \text{ m}$$

$$B = b + 2sh = 6 + 2 \times 2 \times 1 = 10 \text{ m}$$

$$R = A/P = 8/10.47 = 0.76 \text{ m}$$

$$D = A/B = 8/10 = 0.8 \text{ m}$$

$$Q = AU = 8 \times 3 = 24 \text{ m}^3/\text{s}$$

$$Re = \frac{UR}{\nu} = \frac{3 \times 0.76}{10^{-6}} = 2.28 \times 10^6 > 12,500$$

$$Fr = \frac{U}{\sqrt{gD}} = \frac{3}{\sqrt{9.81 \times 0.8}} = 1.07 > 1$$

Hence, the flow is supercritical turbulent.

$$c = \sqrt{gD} = \sqrt{9.81 \times 0.8} = 2.80 \text{ m/s} < U$$

Hence, both the wave fronts move downstream.

$$\text{Speed of one wave front} = U - c = 3 - 2.80$$
$$= 0.20 \text{ m/s}$$

$$\text{Speed of another wave front} = U + c$$

$$= 3 + 2.80 = 5.80 \text{ m/s}$$

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1) Triangle $s = 2$

$$A = sh^2 = 2 \times 1^2 = 2 \text{ m}^2 \quad P = 2\sqrt{1+s^2}h = 2\sqrt{1+2^2} \times 1$$

$$B = 2sh = 2 \times 2 \times 1 = 4 \text{ m} \quad = 4.47 \text{ m}$$

$$R = A/P = 2/4.47 = 0.45 \text{ m} \quad D = A/B = 2/4 = 0.50 \text{ m}$$

$$Q = AU = 2 \times 3 = 6 \text{ m}^3/\text{s}$$

$$Re = \frac{UR}{\nu} = \frac{3 \times 0.45}{10^{-6}} = 1.35 \times 10^6 > 12,500$$

$$Fr = \frac{U}{\sqrt{gD}} = \frac{3}{\sqrt{9.81 \times 0.50}} = 1.35 > 1$$

Hence, the flow is supercritical turbulent.

$$c = \sqrt{gD} = \sqrt{9.81 \times 0.50} = 2.21 \text{ m/s} < U$$

Hence, both the wave fronts move downstream.

$$\text{Speed of one wave front} = U - c = 3 - 2.21 = 0.79 \text{ m/s}$$

$$\text{Speed of another wave front} = U + c = 3 + 2.21 = 5.21 \text{ m/s}$$

1) Parabola $B = 4 \text{ m}, h = 1 \text{ m}, \frac{4h}{B} = \frac{4 \times 1}{4} = 1$

$$A = \frac{2}{3} Bh = \frac{2}{3} \times 4 \times 1 = 2.667 \text{ m}^2$$

$$P = \frac{B}{2} \left[\sqrt{1 + (4h/B)^2} + \frac{B}{4h} \ln \left(\frac{4h}{B} + \sqrt{1 + (4h/B)^2} \right) \right]$$
$$= \frac{4}{2} \left[\sqrt{1 + 1} + 1 \times \ln(1 + \sqrt{1 + 1}) \right] = 4.591 \text{ m}$$

$$R = A/P = 2.667/4.591 = 0.58 \text{ m}$$

$$D = A/B = 2.667/4 = 0.667 \text{ m}$$

$$Q = AU = 2.667 \times 3 = 8 \text{ m}^3/\text{s}$$

$$Re = \frac{UR}{\nu} = \frac{3 \times 0.58}{10^{-6}} = 1.74 \times 10^6 > 12,500$$

$$Fr = \frac{U}{\sqrt{gD}} = \frac{3}{\sqrt{9.81 \times 0.667}} = 1.17 > 1$$

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Hence, the flow is supercritical turbulent.

$$c = \sqrt{gD} = \sqrt{9.81 \times 0.667} = 2.56 \text{ m/s} < U$$

Hence, both the wave fronts move downstream.

$$\text{Speed of one wave front} = U - c = 3 - 2.56 = 0.44 \text{ m/s}$$

$$\text{Speed of another wave front} = U + c = 3 + 2.56 = 5.56 \text{ m/s}$$

vi) Circle $d_0 = 2.5 \text{ m}$

$$\omega = 2 \cos^{-1} \left(1 - \frac{2h}{d_0} \right) = 2 \cos^{-1} \left(1 - \frac{2+1}{2.5} \right) = 2.739 \text{ rad}$$

$$A = \frac{1}{8} (\omega - 3 \sin \omega) d_0^2 = \frac{1}{8} (2.739 - 3 \sin 2.739) 2.5^2 = 1.833 \text{ m}^2$$

$$P = \frac{1}{2} \omega d_0 = \frac{1}{2} \times 2.739 \times 2.5 = 3.424 \text{ m}$$

$$B = \left(\sin \frac{\omega}{2} \right) d_0 = \left(\sin \frac{2.739}{2} \right) \times 2.5 = 2.449 \text{ m}$$

$$R = A/P = 1.833/3.424 = 0.535 \text{ m}$$

$$D = A/B = 1.833/2.449 = 0.748 \text{ m}$$

$$Q = AU = 1.833 \times 3 = 5.50 \text{ m}^3/\text{s}$$

$$Re = \frac{UR}{\nu} = \frac{3 \times 0.535}{10^{-6}} = 1.61 \times 10^6 > 12,500$$

$$Fr = \frac{U}{\sqrt{gD}} = \frac{3}{\sqrt{9.81 \times 0.748}} = 1.11 > 1$$

Hence, the flow is supercritical turbulent.

$$c = \sqrt{gD} = \sqrt{9.81 \times 0.748} = 2.71 \text{ m/s} < U$$

Hence, both the wave fronts move downstream.

$$\text{Speed of one wave front} = U - c = 3 - 2.71 = 0.29 \text{ m/s}$$

$$\text{Speed of another wave front} = U + c = 3 + 2.71 = 5.71 \text{ m/s}$$

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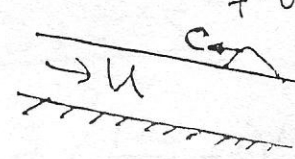
a) Wide $h = 5\text{m}$ Distance = 30km

Speed of tidal wave, $c = \sqrt{gD} = \sqrt{gh}$

(\because for a wide channel, $D=h$)

$$\therefore c = \sqrt{9.81 \times 5} = 7.00 \text{ m/s}$$

$$\text{i) } t = \frac{\text{Distance}}{c} = \frac{30 \times 1000}{7.00} = 4286 \text{ s} = 1.19 \text{ hrs}$$

$$\text{ii) } U = 1 \text{ m/s}$$


$$t = \frac{\text{Distance}}{c - U} = \frac{30 \times 1000}{7 - 1} = 5000 \text{ s} = 1.39 \text{ hrs}$$

b) Let c be the celerity of the waves which travel a distance equal to 50m in 10 s .

$$\therefore c \times 10 = 50 \quad \therefore c = 50/10 = 5 \text{ m/s}$$

But $c = \sqrt{gD} = \sqrt{gh}$ (assuming the pond is wide)

$$\therefore h = \frac{c^2}{g} = \frac{5^2}{9.81} = 2.548 \text{ m}$$

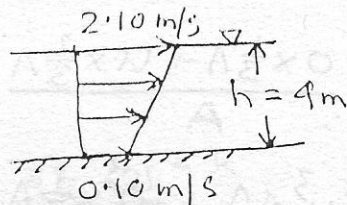
$$\begin{aligned} \therefore \text{Volume of water in the pond} &= \text{surface area of the pond} \times \text{depth} \\ &= \pi r^2 \times h = 3.14 \times 50^2 \times 2.548 = 20,015 \text{ m}^3 \end{aligned}$$

The volume computed is approximate since it cannot be expected that the wave travels at a uniform speed from the centre of the lake to its edge.

(6)

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1.4 (a) Since the river is wide,
 $R = D = h = 4 \text{ m}$



$$i) \bar{U} = \frac{0.1 + 2.10}{2} = 1.10 \text{ m/s}$$

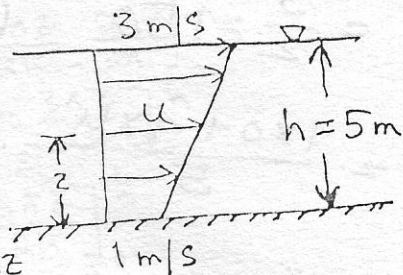
$$\therefore q = \bar{U}h = 1.10 \times 4 = 4.40 \text{ m}^3/\text{s}$$

$$ii) Re = \frac{\bar{U}R}{\nu} = \frac{\bar{U}h}{\nu} = \frac{1.10 \times 4}{10^{-6}} = 4.40 \times 10^6 > 12,500$$

$$Fr = \frac{\bar{U}}{\sqrt{gD}} = \frac{\bar{U}}{\sqrt{gh}} = \frac{1.10}{\sqrt{9.81 \times 4}} = 0.18 < 1$$

Hence, the flow is subcritical turbulent.

(b) Since the river is wide,
 $R = D = h = 5 \text{ m}$



$$i) \bar{U} = \frac{1}{h} \int_0^h u dz = \frac{1}{h} \int_0^h (1 + 2z/h) dz$$

$$= \frac{1}{h} \left[z + \frac{2z^2}{2h} \right]_0^h = \frac{1}{h} [h + h] = 2 \text{ m/s}$$

or, using the trapezoidal rule

$$\bar{U} = \frac{1 + 3}{2} = 2 \text{ m/s}$$

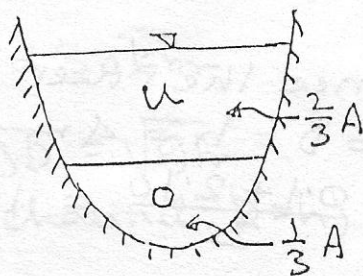
$$\therefore q = \bar{U}h = 2 \times 5 = 10 \text{ m}^3/\text{s}$$

$$ii) Re = \frac{\bar{U}R}{\nu} = \frac{\bar{U}h}{\nu} = \frac{2 \times 5}{10^{-6}} = 10 \times 10^6 > 12,500$$

$$Fr = \frac{\bar{U}}{\sqrt{gD}} = \frac{\bar{U}}{\sqrt{gh}} = \frac{2}{\sqrt{9.81 \times 5}} = 0.29 < 1$$

Hence, the flow is subcritical turbulent.

$$5(a) \quad U = \frac{0 \times \frac{1}{3}A + u \times \frac{2}{3}A}{A} = \frac{2}{3}u$$



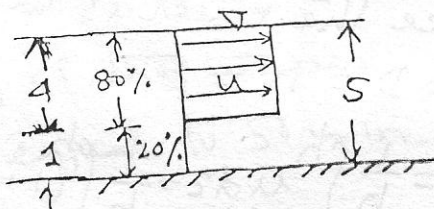
$$\alpha = \frac{\sum u^3 \Delta A}{U^3 A} = \frac{0^3 \times \frac{1}{3}A + u^3 \times \frac{2}{3}A}{\left(\frac{2}{3}u\right)^3 A}$$

$$= \frac{u^3 \times \frac{2}{3} + A}{\frac{2}{3} \times \frac{2}{3} + \frac{2}{3} + u^3 \times A} = \frac{9}{4} = 2.25$$

$$\beta = \frac{\sum u^2 \Delta A}{U^2 A} = \frac{0 \times \frac{1}{3}A + u^2 \times \frac{2}{3}A}{\left(\frac{2}{3}u\right)^2 A} = \frac{u^2 \times \frac{2}{3} \times A}{\frac{2}{3} \times \frac{2}{3} \times u^2 \times A}$$

$$= \frac{3}{2} = 1.50 \quad \left(\frac{\alpha-1}{\beta-1} = \frac{2.25-1}{1.5-1} = \frac{1.25}{.5} = 2.5 \right)$$

$$(b) \quad \bar{U} = \frac{1 \times 0 + 4 \times u}{5} = \frac{4}{5}u$$



$$\alpha = \frac{\sum u^3 \Delta A}{\bar{U}^3 A} = \frac{0^3 \times 1 + u^3 \times 4}{\left(\frac{4}{5}u\right)^3 \times 5}$$

$$= \frac{4u^3 \times 5^3}{4^3 \times u^3 \times 5} = \frac{5^2}{4^2} = \frac{25}{16} = 1.5625$$

$$\beta = \frac{\sum u^2 \Delta A}{\bar{U}^2 A} = \frac{0^2 \times 1 + u^2 \times 4}{\left(\frac{4}{5}u\right)^2 \times 5} = \frac{u^2 \times 4 \times 5^2}{4^2 \times u^2 \times 5}$$

$$= \frac{5}{4} = 1.25 \quad \left(\frac{\alpha-1}{\beta-1} = \frac{1.5625-1}{1.25-1} = \frac{0.5625}{0.25} = 2.25 \right)$$

1.6(a)

\bar{U}

α

$\beta =$

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$\frac{\alpha-1}{\beta-1}$

(8)

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$$6(a) \quad u = u_0 \sin \frac{\pi z}{2h}$$

$$\bar{u} = \frac{1}{h} \int_0^h u_0 \sin \frac{\pi z}{2h} dz = \frac{u_0}{h} \left[-\cos \frac{\pi z}{2h} \cdot \frac{2h}{\pi} \right]_0^h$$

$$= -\frac{2u_0}{\pi} [0 - 1] = \frac{2u_0}{\pi}$$

$$\alpha = \frac{\int_A u^3 dA}{\bar{u}^3 A} = \frac{\pi^3}{8u_0^3 h} \int_0^h u_0^3 \sin^3 \frac{\pi z}{2h} dz$$

$$= \frac{\pi^3}{8h} \int_0^h \frac{1}{4} \left(3 \sin \frac{\pi z}{2h} - \sin \frac{3\pi z}{2h} \right) dz$$

$$= \frac{\pi^3}{32h} \int_0^h \left[+3 \sin \frac{\pi z}{2h} - \sin \frac{3\pi z}{2h} \right] dz$$

$$= \frac{\pi^3}{32h} \left[-3 \cos \frac{\pi z}{2h} \cdot \frac{2h}{\pi} + \cos \frac{3\pi z}{2h} \cdot \frac{2h}{3\pi} \right]_0^h$$

$$= \frac{\pi^3}{32h} \left[-\frac{6h}{\pi} \cos \frac{\pi z}{2h} + \frac{2h}{3\pi} \cos \frac{3\pi z}{2h} \right]_0^h$$

$$= \frac{\pi^3 \times 2h}{3\pi \times 32h} \left[-9 \cos \frac{\pi z}{2h} + \cos \frac{3\pi z}{2h} \right]_0^h$$

$$= \frac{\pi^3}{48} [-0 + 9 + 0 - 1] = \frac{\pi^2}{6} = 1.645$$

$$B = \frac{\int_A \bar{u} dA}{\bar{u} A} = \frac{\pi^2}{4u_0^2 h} \int_0^h u_0^2 \sin^2 \frac{\pi z}{2h} dz$$

$$= \frac{\pi^2}{4h} \int_0^h \frac{1}{2} \left(1 - \cos \frac{2\pi z}{2h} \right) dz = \frac{\pi^2}{8h} \left[z - \sin \frac{\pi z}{h} \cdot \frac{h}{\pi} \right]_0^h$$

$$= \frac{\pi^2}{8h} [h - 0 - 0 + 0] = \frac{\pi^2 h}{8h} = \frac{\pi^2}{8} = 1.234$$

$$\frac{\alpha-1}{\beta-1} = \frac{1.645-1}{1.234-1} = \frac{0.645}{0.234} = 2.76$$

(9)

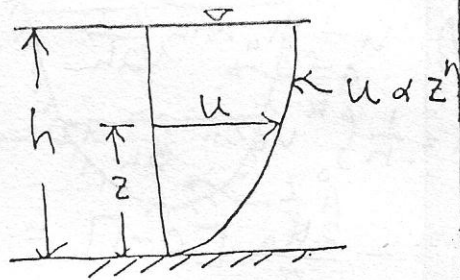
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6(b) $u = kz^n$, $k = \text{constant}$

$$\bar{u} = \frac{1}{h} \int_0^h u dz = \frac{1}{h} \int_0^h kz^n dz$$

$$= \frac{k}{h} \left[\frac{z^{n+1}}{n+1} \right]_0^h = \frac{kh^n}{n+1}$$



$$\alpha = \frac{1}{\bar{u}^3 h} \int_0^h u^3 dz = \frac{1}{\bar{u}^3 h} \int_0^h k^3 z^{3n} dz$$

$$= \frac{k^3}{\bar{u}^3 h} \left[\frac{z^{3n+1}}{3n+1} \right]_0^h = \frac{k^3}{\bar{u}^3 h} \frac{h^{3n+1}}{3n+1}$$

$$= \frac{(n+1)^3 k^3}{k^3 h^{3n} h} \cdot \frac{h^{3n+1}}{3n+1} = \frac{(n+1)^3}{3n+1}$$

$$\beta = \frac{1}{\bar{u}^2 h} \int_0^h u^2 dz = \frac{1}{\bar{u}^2 h} \int_0^h k^2 z^{2n} dz$$

$$= \frac{k^2}{\bar{u}^2 h} \left[\frac{z^{2n+1}}{2n+1} \right]_0^h = \frac{k^2}{\bar{u}^2 h} \frac{h^{2n+1}}{2n+1}$$

$$= \frac{(n+1)^2 k^2}{k^2 h^{2n} h} \cdot \frac{h^{2n+1}}{2n+1} = \frac{(n+1)^2}{2n+1}$$

$$\therefore \frac{\alpha-1}{\beta-1} = \frac{\frac{(n+1)^3}{3n+1} - 1}{\frac{(n+1)^2}{2n+1} - 1} = \frac{\frac{(n+1)^3 - (3n+1)}{3n+1}}{\frac{(n+1)^2 - (2n+1)}{2n+1}}$$

$$= \frac{(n^3 + 3n^2 + 3n + 1 - 3n - 1)(2n+1)}{(n^2 + 2n + 1 - 2n - 1)(3n+1)} = \frac{(2n+1)(n+3)}{3n+1}$$

When $n = \frac{1}{7}$, $\alpha = 1.045$, $\beta = 1.016$ and $\frac{\alpha-1}{\beta-1} = 2.83$

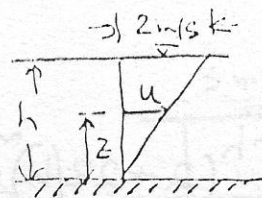
	α	β	$\frac{\alpha-1}{\beta-1}$
Laminar	1.64	1.234	2.76
Turbulent	1.045	1.016	2.83

$$1.7(i) \quad u = 2z/h$$

$$\bar{U} = \frac{1}{h} \int_0^h u dz = \frac{1}{h} \int_0^h \frac{2z}{h} dz$$

$$= \frac{2}{h^2} \int_0^h z dz = \frac{2}{h^2} \left[\frac{z^2}{2} \right]_0^h = \frac{2}{h^2} \times \frac{h^2}{2} = 1 \text{ m/s}$$

or $\bar{U} = \frac{0+2}{2} = 1 \text{ m/s}$



$$\alpha = \frac{1}{\bar{U}^3 h} \int_0^h u^3 dz = \frac{1}{\bar{U}^3 h} \int_0^h \frac{8z^3}{h^3} dz = \frac{8}{\bar{U}^3 h^4} \left[\frac{z^4}{4} \right]_0^h$$

$$= \frac{8}{\bar{U}^3 h^4} \times \frac{h^4}{4} = \frac{2}{1^3} = 2$$

$$\beta = \frac{1}{\bar{U}^2 h} \int_0^h u^2 dz = \frac{1}{\bar{U}^2 h} \int_0^h \frac{4z^2}{h^2} dz = \frac{4}{\bar{U}^2 h^3} \left[\frac{z^3}{3} \right]_0^h$$

$$= \frac{4}{1^2 \times h^3} \times \frac{h^3}{3} = 1.333$$

$$\therefore (\alpha-1)/(\beta-1) = 1/0.333 = 3.00$$

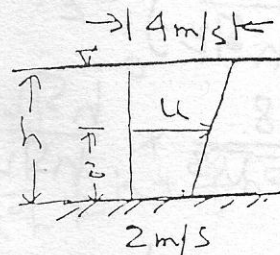
$$1.7(ii) \quad u = 2 + 2z/h, \quad \bar{U} = \frac{2+4}{2} = 3 \text{ m/s}$$

$$\alpha = \frac{1}{\bar{U}^3 h} \int_0^h (2 + 2z/h)^3 dz$$

$$= \frac{1}{\bar{U}^3 h} \int_0^h (8 + 24z/h + 24z^2/h^2 + 8z^3/h^3) dz$$

$$= \frac{1}{\bar{U}^3 h} \left[8h + \frac{24}{h} \cdot \frac{h^2}{2} + \frac{24}{h^2} \cdot \frac{h^3}{3} + \frac{8}{h^3} \cdot \frac{h^4}{4} \right]$$

$$= \frac{1}{\bar{U}^3 h} [8h + 12h + 8h + 2h] = \frac{30h}{\bar{U}^3 h} = \frac{30}{27} = 1.111$$



(11)

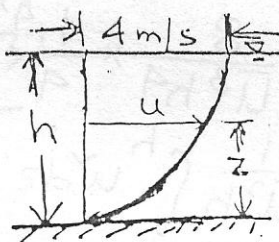
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$$\begin{aligned}
 B &= \frac{1}{\bar{u}^2 h} \int_0^h (2 + 2z/h) \bar{u} dz = \frac{1}{\bar{u}^2 h} \int_0^h (4 + 8z/h + 4z^2/h^2) dz \quad (iv) \\
 &= \frac{1}{\bar{u}^2 h} \left(4h + \frac{8}{h} \cdot \frac{h^2}{2} + \frac{4}{h^2} \cdot \frac{h^3}{3} \right) = \frac{1}{\bar{u}^2 h} \left(4h + 4h + \frac{4}{3}h \right) \\
 &= \frac{1}{\bar{u}^2 h} \times \frac{28h}{3} = \frac{28}{3\bar{u}^2} = \frac{28}{3 \times 9} = \frac{28}{27} = 1.037
 \end{aligned}$$

$$\therefore (\alpha - 1) / (B - 1) = (3/27) \div (1/27) = 3.000$$

$$(ii) \quad u = 4(z/h)^{1/2}$$

$$\begin{aligned}
 \bar{u} &= \frac{1}{h} \int_0^h u dz = \frac{1}{h} \int_0^h 4(z/h)^{1/2} dz \\
 &= \frac{4}{h^{3/2}} \int_0^h z^{1/2} dz = \frac{4}{h^{3/2}} \left[\frac{z^{3/2}}{3/2} \right]_0^h = \frac{8}{3} \frac{h^{3/2}}{h^{3/2}} = \frac{8}{3} \text{ m/s}
 \end{aligned}$$



$$\begin{aligned}
 \alpha &= \frac{1}{\bar{u}^3 h} \int_0^h u^3 dz = \frac{1}{\bar{u}^3 h} \int_0^h 64 z^{3/2} / h^{3/2} dz \\
 &= \frac{64}{\bar{u}^3 h^{5/2}} \int_0^h z^{3/2} dz = \frac{64}{\bar{u}^3 h^{5/2}} \left[\frac{z^{5/2}}{5/2} \right]_0^h \\
 &= \frac{128}{5\bar{u}^3} \times \frac{h^{5/2}}{h^{5/2}} = \frac{128 \times 3 \times 3 \times 3}{5 \times 8 \times 8 \times 8} = \frac{27}{20} = 1.350
 \end{aligned}$$

$$\begin{aligned}
 B &= \frac{1}{\bar{u}^2 h} \int_0^h u^2 dz = \frac{1}{\bar{u}^2 h} \int_0^h \frac{16z}{h} dz = \frac{16}{\bar{u}^2 h^2} \int_0^h z dz \\
 &= \frac{16}{\bar{u}^2 h^2} \left[\frac{z^2}{2} \right]_0^h = \frac{8h^2}{\bar{u}^2 h^2} = \frac{8 \times 3 \times 3}{8 \times 8} = \frac{9}{8} = 1.125
 \end{aligned}$$

$$\therefore \frac{\alpha - 1}{B - 1} = \frac{0.350}{0.125} = 2.80$$

(12)

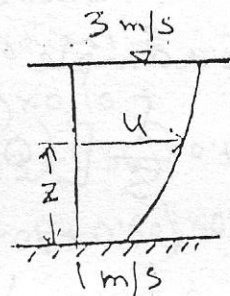
(12)

dz

(h)

$$(iv) u = 1 + 2(z/h)^{1/2}$$

$$\begin{aligned} \bar{u} &= \frac{1}{h} \int_0^h u dz = \frac{1}{h} \int_0^h [1 + 2(z/h)^{1/2}] dz \\ &= \frac{1}{h} \left[z + \frac{2z^{3/2}}{(3/2)h} \right]_0^h = \frac{1}{h} \left[h + \frac{4}{3}h \right] \\ &= 1 + \frac{4}{3} = \frac{7}{3} \text{ m/s} \end{aligned}$$



$$\alpha = \frac{1}{\bar{u}^3 h} \int_0^h u^3 dz = \frac{1}{\bar{u}^3 h} \int_0^h [1 + 2(z/h)^{1/2}]^3 dz$$

$$= \frac{1}{\bar{u}^3 h} \int_0^h \left[1 + \frac{6z^{1/2}}{h^{1/2}} + \frac{12z}{h} + \frac{8z^{3/2}}{h^{3/2}} \right] dz$$

$$= \frac{1}{\bar{u}^3 h} \left[z + \frac{6z^{3/2}}{(3/2)h^{1/2}} + \frac{12z^2}{2h} + \frac{8z^{5/2}}{(5/2)h^{3/2}} \right]_0^h$$

$$= \frac{1}{\bar{u}^3 h} \left[h + 4h + 6h + \frac{16}{5}h \right] = \frac{1}{\bar{u}^3} \left[1 + 4 + 6 + \frac{16}{5} \right]$$

$$= \frac{71}{5 \times 7^3} = \frac{71 \times 3 \times 3 \times 3}{5 \times 7 \times 7 \times 7} = 1.118$$

$$\beta = \frac{1}{\bar{u}^2 h} \int_0^h u^2 dz = \frac{1}{\bar{u}^2 h} \int_0^h [1 + 2(z/h)^{1/2}]^2 dz$$

$$= \frac{1}{\bar{u}^2 h} \int_0^h \left[1 + \frac{4z^{1/2}}{h^{1/2}} + \frac{4z}{h} \right] dz$$

$$= \frac{1}{\bar{u}^2 h} \left[z + \frac{4z^{3/2}}{(3/2)h^{1/2}} + \frac{4z^2}{2h} \right]_0^h = \frac{1}{\bar{u}^2 h} \left[h + \frac{8}{3}h + 2h \right]$$

$$= \frac{1}{\bar{u}^2 h} \times \frac{17h}{3} = \frac{17}{3 \times 7^2} = \frac{17 \times 3 \times 3}{3 \times 7 \times 7} = \frac{51}{49} = 1.041$$

$$\therefore (\alpha - 1) / (\beta - 1) = 0.118 / 0.041 = 2.89$$

The numerical values of α and β are same for $h = 5 \text{ m}$ and $h = 10 \text{ m}$.

(13)

(13)

(12)

dz

125

8(a) Example 1.5 by Simpson's rule

$$A = \frac{d}{3} [(O_0 + O_n) + 2(O_2 + O_4 + \dots) + 4(O_1 + O_3 + \dots)]$$

To apply the Simpson's rule, no. of ordinates must be odd and no. of strips must be even.

In this problem, no. of strips, $n = 6$ and no. of ordinates, $n+1 = 7$. Hence, the Simpson's rule can be used directly.

$$Q = \frac{1}{3} [(0 + 4.51) + 2(3.31 + 3.95) + 4(2.95 + 3.62 + 4.12)]$$

$$= \frac{1}{3} \times 61.79 = 20.597 \text{ m}^3/\text{s}$$

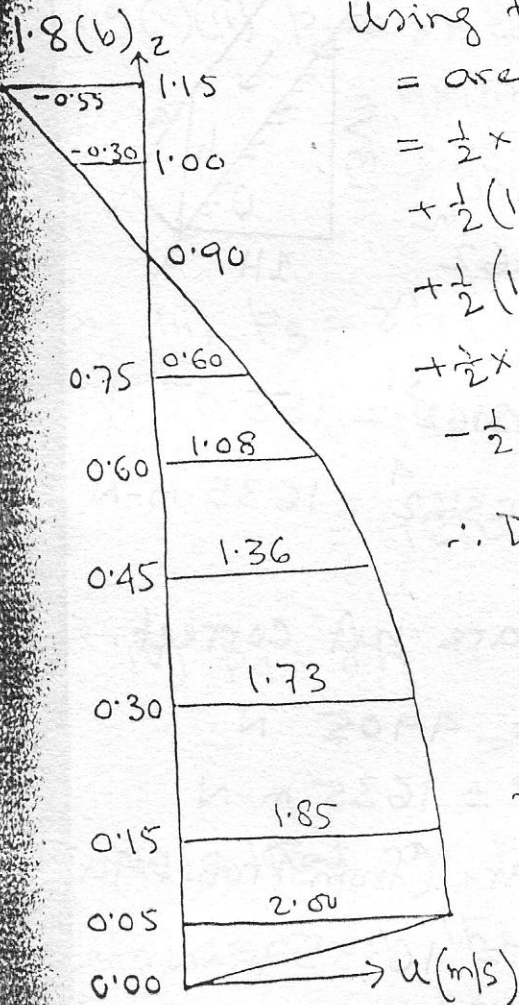
$$\bar{U} = Q/h = 20.597/6 = 3.433 \text{ m/s}$$

$$\alpha = \frac{1}{3.433^3 \times 6} \times \frac{1}{3} [(0^3 + 4.51^3) + 2(3.31^3 + 3.95^3) + 4(2.95^3 + 3.62^3 + 4.12^3)]$$

$$= \frac{859.702}{3.433^3 \times 6 \times 3} = 1.181$$

$$\beta = \frac{1}{3.433^2 \times 6} \times \frac{1}{3} [(0^2 + 4.51^2) + 2(3.31^2 + 3.95^2) + 4(2.95^2 + 3.62^2 + 4.12^2)]$$

$$= \frac{228.5825}{3.433^2 \times 6 \times 3} = 1.078$$



Using trapezoidal rule, $\sum u \Delta z$
 = area of the plot of \$u\$ against \$z\$
 = $\frac{1}{2} \times 2 \times 0.05 + \frac{1}{2} (2.00 + 1.85) \times 0.10 +$
 $+\frac{1}{2} (1.85 + 1.73) \times 0.15 + \frac{1}{2} (1.73 + 1.36) \times 0.15$
 $+\frac{1}{2} (1.36 + 1.08) \times 0.15 + \frac{1}{2} (1.08 + 0.60) \times 0.15$
 $+\frac{1}{2} \times 0.60 \times 0.15 - \frac{1}{2} \times 0.30 \times 0.10$
 $- \frac{1}{2} (0.30 + 0.55) \times 0.15 = 1.018$

\therefore Discharge per unit width
 $q = \sum u \Delta z = 1.018 \text{ m}^3/\text{s}$

Total depth, \$h = 1.15 \text{ m}\$

\therefore Mean velocity, $\bar{u} = \frac{q}{h} = \frac{1.018}{1.15}$
 $= 0.885 \text{ m/s}$

5) $\sum u^3 \Delta z = \frac{1}{2} \times 2^3 \times 0.05 + \frac{1}{2} (2.00^3 + 1.85^3) \times 0.10 + \frac{1}{2} (1.85^3 + 1.73^3) \times 0.15$
 $+\frac{1}{2} (1.73^3 + 1.36^3) \times 0.15 + \frac{1}{2} (1.36^3 + 1.08^3) \times 0.15 + \frac{1}{2} (1.08^3 + 0.60^3) \times 0.15$
 $+\frac{1}{2} \times 0.60^3 \times 0.15 - \frac{1}{2} \times 0.30^3 \times 0.10 - \frac{1}{2} (0.30^3 + 0.55^3) \times 0.15 = 2.7509$

$\therefore \alpha = \frac{\sum u^3 \Delta z}{\bar{u}^3 h} = \frac{2.7509}{0.885^3 \times 1.15} = 3.450$

15) $\sum u^2 \Delta z = \frac{1}{2} \times 2^2 \times 0.05 + \frac{1}{2} (2.00^2 + 1.85^2) \times 0.10 + \frac{1}{2} (1.85^2 + 1.73^2) \times 0.15$
 $+\frac{1}{2} (1.73^2 + 1.36^2) \times 0.15 + \frac{1}{2} (1.36^2 + 1.08^2) \times 0.15 + \frac{1}{2} (1.08^2 + 0.60^2) \times 0.15$
 $+\frac{1}{2} \times 0.60^2 \times 0.15 + \frac{1}{2} \times 0.30^2 \times 0.10 + \frac{1}{2} (0.30^2 + 0.55^2) \times 0.15 = 1.7107$

$\therefore \beta = \frac{\sum u^2 \Delta z}{\bar{u}^2 h} = \frac{1.7107}{0.885^2 \times 1.15} = 1.906$

(15)

(15)

(14)

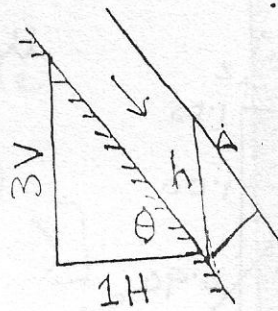
1.9(a) Slope = 1H:3V, $h = 1\text{ m}$

$\tan\theta = 3, \theta = 71.56^\circ, \cos\theta = 0.3162$

$\therefore p_{bed} = 1000 \times 9.81 \times 1 \times 0.3162^2$
 $= 980.83 \text{ N/m}^2$

$F = \frac{1}{2} \times 1000 \times 9.81 \times 1^2 \times 0.3162^3 = 155.07 \text{ N}$

O.M. = $\frac{1}{6} \times 1000 \times 9.81 \times 1^3 \times 0.3162^4 = 16.35 \text{ m-N}$



(b) The computed results are not correct.

$F_{\text{wrong}} = \frac{1}{2} \times 1000 \times 9.81 \times 1^2 = 4905 \text{ N}$

O.M. wrong = $\frac{1}{6} \times 1000 \times 9.81 \times 1^3 = 1635 \text{ m-N}$

The corrected results are (from Prob. 1.9a)

$F = 155.07 \text{ N}$ and $\text{O.M.} = 16.35 \text{ m-N}$

% error for $F = \frac{4905 - 155.07}{155.07} \times 100 = 3063.09$

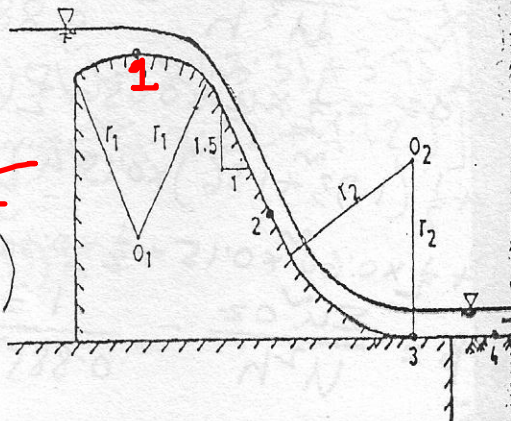
% error for O.M. = $\frac{1635 - 16.35}{16.35} \times 100 = 9900.00$

(c) $r_1 = r_2 = 20 \text{ m}, q_v = 6.5 \text{ m}^3/\text{s}$

$h_1 = h_2 = h_3 = h_4 = 1 \text{ m}$

$U = \frac{q_v}{h} = \frac{6.5}{1} = 6.5 \text{ m/s}$

i) $p_1 = \gamma h \left(1 - \frac{U^2}{g r}\right)$ $\gamma h = \frac{\gamma h^2}{r}$
 $= 1000 \times 9.81 \times 1 \times \left(1 - \frac{6.5^2}{9.81 \times 20}\right)$
 $= 9810 (1 - 0.215) \text{ N/m}^2$
 $= 7,697.50 \text{ N/m}^2$



$$1.9(c)(ii) \quad p_2 = \gamma h \cos^2 \theta$$

$$= 9810 \times 1 \times \cos^2 56.31^\circ$$

$$= 3,018.46 \text{ N/m}^2$$

$$\tan \theta = \frac{1.5}{1} = 1.5$$

$$\therefore \theta = \tan^{-1} 1.5 = 56.31^\circ$$

$$(iii) \quad p_3 = \gamma h \left(1 + \frac{u^2}{gr} \right)$$

$$\gamma h + \gamma h \frac{u^2}{r} = \gamma h + \frac{\gamma h u^2}{r}$$

$$= 9810 \times 1 \times \left(1 + \frac{6.5^2}{9.81 \times 20} \right)$$

$$= 9810 (1 + 0.215) = 11,922.50 \text{ N/m}^2$$

$$(iv) \quad p_4 = \gamma h = 9810 \times 1 \text{ N/m}^2$$

$$= 9,810 \text{ N/m}^2$$

1.9a) Note that at point 1, the centrifugal force acts against the hydrostatic force. Point 2 lies on the slope of the spillway and hence the effect of the slope is to be included. At point 3, the centrifugal force adds to the hydrostatic force. Point 4 lies on the horizontal and the pressure due to hydrostatic head h only.

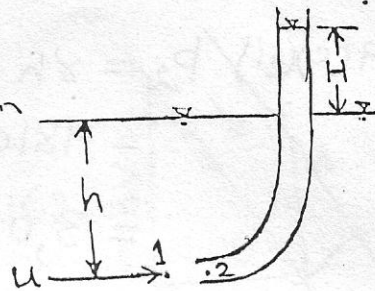
(17)

(16)

(17)

Chapter 2

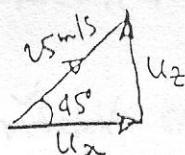
2.1 Applying the energy equation between points 1 and 2 and neglecting the energy loss, we get



$$0 + h + \frac{u^2}{2g} = 0 + (h+H) + 0$$

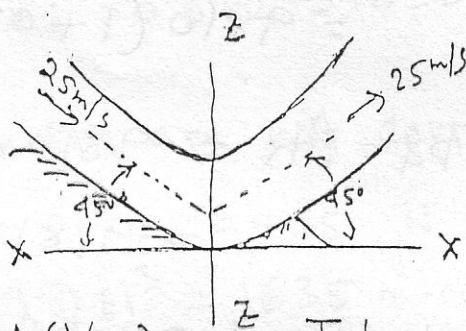
or, $\frac{u^2}{2g} = H \quad \therefore u = \sqrt{2gH}$ (Proved)

2.2



$$u_z = 25 \sin 45^\circ = 25/\sqrt{2} \text{ m/s}$$

$$u_x = 25 \cos 45^\circ = 25/\sqrt{2} \text{ m/s}$$



From $V^2 = u^2 - 2gh$, we get ($V=0$) Take unit width

$$h = \frac{u_z^2}{2g} = \frac{25^2}{2 \times 2 \times 9.81} = 15.93 \text{ m}$$

Force ⁱⁿ the z-z direction (i.e. vertical force)

= mass \times change in velocity

= $8Q$ (change in velocity)

$$= 1000 \times 0.5 \times 1 \times 2.5 \times [25/\sqrt{2} - (-25/\sqrt{2})]$$

$$= 49194 \text{ N (acting downwards) per unit width}$$

$$[8Q \cdot 2u_z = \frac{\text{kgm}}{\text{m}^3} \times \frac{\text{m}^3}{\text{s}} \times \frac{\text{m}}{\text{s}} = \text{kgm-in/s}^2 = \text{force}]$$

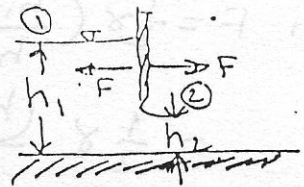
Force in the x-x direction (i.e. horizontal force)

= mass \times change in velocity

$$= 8Q(u_x - u_x) = 0$$

⊗

2.3 (a) Consider unit width of the channel. The force exerted on the fluid by the gate is F which



is equal and opposite to that exerted by the fluid on the gate. Assume that (i) section 1 is sufficiently up from the gate, and (ii) section 2 is at the vena contracta, (iii) $\alpha_1 = \alpha_2 = 1$, (iv) $\beta_1 = \beta_2 = 1$, (v) energy losses between the sections is negligible, and (vi) the bottom friction force F_f between the sections is negligible. Then writing the

$$momentum equation between sections 1 and 2$$

$$8q(U_2 - U_1) = F_{p1} - F_{p2} - F$$

$$\text{or, } 8q\left(\frac{q}{h_2} - \frac{q}{h_1}\right) = \frac{1}{2} \rho h_1^2 - \frac{1}{2} \rho h_2^2 - F$$

it width

$$\text{or, } F = \frac{1}{2} \rho h_1^2 - \frac{1}{2} \rho h_2^2 - 8q^2 \left(\frac{1}{h_2} - \frac{1}{h_1}\right)$$

$$= \frac{1}{2} \rho h_1^2 - \frac{1}{2} \rho h_2^2 - 8q^2 \left(\frac{h_1 - h_2}{h_1 h_2}\right)$$

$$= \frac{1}{2} \rho \left[(h_1^2 - h_2^2) - \frac{2q^2}{g} \left(\frac{h_1 - h_2}{h_1 h_2}\right) \right]$$

$$= \frac{1}{2} \rho \frac{(h_1 - h_2)}{h_1 h_2} \left[h_1 h_2 (h_1 + h_2) - \frac{2q^2}{g} \right] \quad (1)$$

The energy equation between sections 1 + 2 gives

$$h_1 + \frac{U_1^2}{2g} = h_2 + \frac{U_2^2}{2g} \quad \text{or} \quad h_1 + \frac{q^2}{2gh_1^3} = h_2 + \frac{q^2}{2gh_2^3}$$

$$\text{or, } \frac{q^2}{2g} \left[\frac{1}{h_2^3} - \frac{1}{h_1^3} \right] = h_1 - h_2$$

$$\text{or, } \frac{q^2}{2g} \left[\frac{h_1 + h_2}{h_1 h_2} \right] = 1 \quad \text{or} \quad \frac{q^2}{g} = \frac{2h_1 h_2}{h_1 + h_2} \quad (2)$$

Combining Eqs (1) and (2)

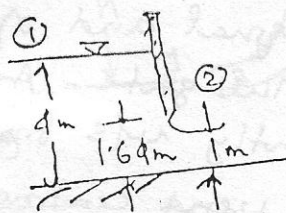
$$F = \frac{1}{2} \rho \frac{(h_1 - h_2)}{h_1 h_2} \left[h_1 h_2 (h_1 + h_2) - \frac{4h_1 h_2}{h_1 + h_2} \right]$$

$$\text{or, } F = \frac{1}{2} \rho \left(\frac{h_1 - h_2}{h_1 + h_2} \right) \left[\frac{h_1 h_2 (h_1 + h_2) - 4 h_1 h_2}{h_1 + h_2} \right]$$

$$= \frac{1}{2} \rho \frac{(h_1 - h_2)^3}{h_1 + h_2} \quad (3)$$

∴ The force on the gate is equal and opposite to F.

1.3 (b)(i) $h_1 = 4\text{m}$, $h_2 = 1\text{m}$, $b = 6\text{m}$



Applying the continuity equation between sections 1 and 2.

$$A_1 U_1 = A_2 U_2 \quad \text{or} \quad b h_1 U_1 = b h_2 U_2 \quad \text{or} \quad 4 U_1 = U_2 \quad (1)$$

Applying the energy equation between sections 1 & 2

$$h_1 + \frac{U_1^2}{2g} = h_2 + \frac{U_2^2}{2g} \quad \text{or, } 4 + \frac{U_1^2}{2g} = 1 + \frac{U_2^2}{2g}$$

$$\text{or, } 4 + \frac{U_1^2}{2g} = 1 + \frac{16 U_1^2}{2g} \quad (\text{using (1)})$$

$$\text{or, } 15 \frac{U_1^2}{2g} = 4 - 1 = 3 \quad \therefore U_1 = \sqrt{\frac{2 \times 9.81 \times 3}{15}} = 1.98 \text{ m/s}$$

$$\therefore Q = A_1 U_1 = b h_1 U_1 = 6 \times 4 \times 1.98 = 47.52 \text{ m}^3/\text{s}$$

$$\text{ii) } Q = 47.52 \text{ m}^3/\text{s} \quad U_1 = 1.98 \text{ m/s} \quad U_2 = 7.92 \text{ m/s}$$

$$gQ(U_2 - U_1) = \frac{1}{2} \rho b h_1^2 - \frac{1}{2} \rho b h_2^2 - F$$

$$\text{or, } F = \frac{1}{2} \rho b h_1^2 - \frac{1}{2} \rho b h_2^2 - gQ(U_2 - U_1)$$

$$= \frac{1}{2} \times 1000 \times 9.81 \times 6 (4^2 - 1^2) - 1000 \times 47.52 (7.92 - 1.98)$$

$$= 44,1450 - 28,2387.6 = 159,062.4 \text{ N} = 159.06 \text{ kN}$$

When the formula is used

$$F = \frac{1}{2} \rho b \frac{(h_1 - h_2)^3}{h_1 + h_2} = \frac{1}{2} \times 1000 \times 9.81 \times 6 \frac{(4-1)^3}{4+1} = 158922 \text{ N}$$

(3)

(20)

3 (contd.) Assuming that the pressure distribution is hydrostatic

$$F = \frac{1}{2} \gamma b (h_1 - h_2)$$

$$= \frac{1}{2} \times 1000 \times 9.81 \times 6 (4 - 1.64)$$

$$= 16,391.3 \text{ N} = 163.91 \text{ kN}$$

$$h_2 = \frac{h_1}{c_c} = \frac{1}{0.61} \\ = 1.64 \text{ m}$$

Also

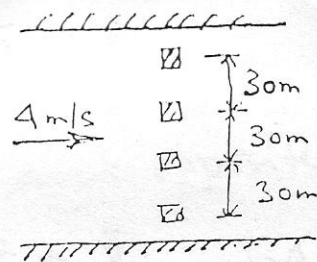
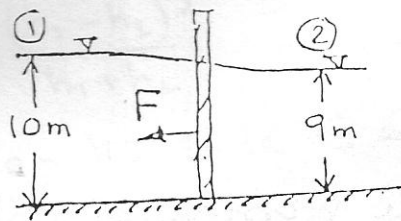
Intensity of pressure on the C.G. of the gate

$$= \gamma \bar{h}$$

$$\therefore F = \gamma \bar{h} A = \gamma \bar{h} A = 1000 \times 9.81 \times (4 - 1.64) / 2 \times (4 - 1.64) \times 6 \\ = 11575.8 \times 14.16 = 163913.3 \text{ N} = 163.91 \text{ kN}$$

The force on the gate is equal and opposite to F.

2.4



$$b = 30\text{m}, h_1 = 10\text{m}, \\ h_2 = 9\text{m}, U_1 = 4\text{m/s}$$

$$Q = h_1 U_1 = 10 \times 4 = 40 \text{ m}^3/\text{s}$$

$$U_2 = Q/h_2 = 40/9 = 4.44 \text{ m/s}$$

Since the piers are placed @ 30m c/c, we consider a width of 30m and apply the steady 1-D momentum eqn.

assuming that $\beta_1 = \beta_2 = 1$.

$$\rho Q(U_2 - U_1) = F_{p1} - F_{p2} + W \sin \theta - F_f - F$$

(F is the force on a pier)

$$\therefore F = F_{p1} - F_{p2} - \rho Q(U_2 - U_1)$$

$$= \frac{1}{2} \rho b (h_1^2 - h_2^2) - \rho b Q (U_2 - U_1)$$

$$= \frac{1}{2} \times 1000 \times 9.81 \times 30 (10^2 - 9^2) - 1000 \times 40 \times 30 \times (4.44 - 4)$$

$$= 1000 \times 30 \left(\frac{1}{2} \times 9.81 \times 19 - 40 \times 0.44 \right)$$

$$= 1000 \times 30 \times 75.595 = 2268 \times 10^3 \text{ N}$$

$$= 2268 \text{ kN}$$

(5)

(22)