

## Fluid Mechanics

### # Circular Disk under water:

Q1

Find the resultant force on one side of a 25 mm diameter vertical circular plate standing at the bottom of 3 m pool water? Density of water =  $1000 \text{ kg/m}^3$ ,  $g = 9.8 \text{ m/s}^2$ .

[SGDL-2017]

Q2

25 mm diameter circular disk is on the base of a swimming pool of depth 2.5 m, width 3 m. Find the force on the circular disk,  $g = 9.8 \text{ m/s}^2$ .

[BHP-2017]

Tazhar Kabir

CUE T (Civil)

Passing year : 2018

Assistant Engineer (Civil)

Payra Port Authority

Cell : 01406 316084

email : tazhar.ppa@gmail.com

### # Flow through Pipe :

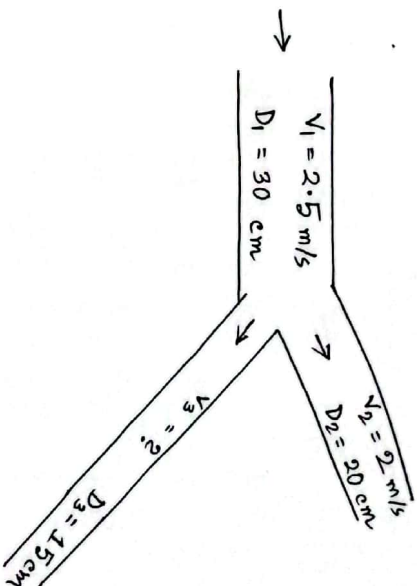
Q1 Water is flowing in a pipe of a diameter 12 cm at a velocity of 2 m/s. Find out the discharge. [DPPDC-2014]

Q2 Calculate the diameter of a circular pipe that has discharges 2.5 cusec water with a velocity of 6 fps ?

[PGCL-2014, PCEB-2015, BIFPEL-2015]

Q3 Water velocity 8 ft/sec, Quantity of water is 11.0304 ft<sup>3</sup>/min. What is the pipe diameter ?

Q1



What is the value of  $V_3 = ?$

Q2 A circular pipe with 10 cm diameter having a flow with an average velocity of 10 m/s, calculate the discharge in liter/sec.

Also calculate the velocity of another end if the diameter of the pipe is gradually increased to 20 cm. [BPDB-2021]

## Fluid Mechanics

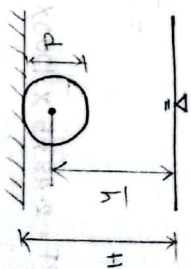
# Circular Disk under water?

Q1 Solution: Force on circular plate,

$$F = \rho \times A$$

$$F = \bar{h} \rho g \times A$$

$$= \bar{h} \rho g \times \left( \frac{\pi}{4} d^2 \right)$$



$$\Rightarrow F = 2.9875 \times 1000 \times 9.8 \times \left( \frac{\pi}{4} \times 0.025^2 \right)$$

$$= 14.37 \text{ N}$$

(Ans)

(Hints)

$$F = \bar{h} \rho g A$$

$$= \left( m \times \frac{kg}{m^3} \times \frac{m}{s^2} \times m^2 \right)$$

$$= kg \times \frac{m}{s^2}$$

$$= N \quad [ \because F = m a ]$$

$$H = 3 \text{ m}$$

$$d = 25 \text{ mm} = 0.025 \text{ m}$$

$$\therefore \bar{h} = \frac{25 \text{ mm}}{2} = 12.5 \text{ mm}$$

$$= 0.0125$$

$$\therefore \bar{h} = 3 - 0.0125$$

$$= 2.9875 \text{ m}$$

$$\rho = 1000 \text{ kg/m}^3$$

$$g = 9.8 \text{ m/s}^2$$

Q1 Solution: Force on circular disk,

$$F = P \times A$$

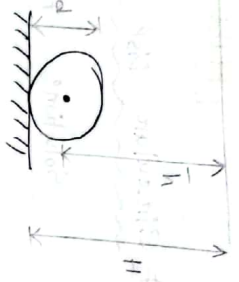
$$\Rightarrow F = \bar{h} \rho g \times A$$

$$= \bar{h} \rho g \times \left(\frac{\pi}{4} d^2\right)$$

$$\Rightarrow F = 2.4875 \times 1000 \times 9.8 \times \left(\frac{\pi}{4} \times 0.025^2\right)$$

$$\therefore F = 11.966 \text{ N}$$

(Am.)



$$d = 25 \text{ mm} = 0.025 \text{ m}$$

$$H = 2.5 \text{ m}$$

$$W = 3 \text{ m}$$

$$F = ?$$

$$\rho = 9.8 \text{ m/s}^2$$

$$\text{Radius, } r = \frac{0.025}{2} \text{ m}$$

$$= 0.0125 \text{ m}$$

$$h = H - r$$

$$= 2.4875 \text{ m}$$

$$\rho = 1000 \text{ kg/m}^3$$

# Flow Through Pipe:

Q1 Solution:

$$\text{Discharge, } Q = A \times V$$

$$= \left(\frac{\pi}{4} d^2\right) \times V$$

$$= \left(\frac{\pi}{4} \times 0.12^2\right) \times 2$$

$$= 0.023 \text{ m}^3/\text{s}$$

(Am.)

$$d = 12 \text{ cm} = 0.12 \text{ m}$$

$$V = 2 \text{ m/s}$$

$$Q = ?$$

# Q2 Solution:

$$Q = AV$$

$$\Rightarrow Q = \left(\frac{\pi}{4} \times d^2\right) \times V$$

$$\Rightarrow 2.5 = \left(\frac{\pi}{4} \times d^2\right) \times 6$$

$$\therefore d = 0.728 \text{ ft}$$

$$= 8.74 \text{ inch}$$

(Am.)

$$d = ?$$

$$Q = 2.5 \text{ cusec}$$

$$= 2.5 \text{ ft}^3/\text{s}$$

$$V = 6 \text{ fps}$$

Q1 Water velocity 6 ft/sec, Quantity of water is 11.0304 ft<sup>3</sup>/min. What is the pipe diameter?

Solution:  $Q = A V$

$$\Rightarrow Q = \left(\frac{\pi}{4} d^2\right) \times V$$

$$\Rightarrow 0.1838 = \left(\frac{\pi}{4} \times d^2\right) \times 6$$

$$\therefore d = 0.1975 \text{ ft} \\ = 2.37 \text{ inch}$$

(Ans.)

$$V = 6 \text{ ft/s}$$

$$Q = 11.0304 \text{ ft}^3/\text{min}$$

$$= \frac{11.0304}{60} \text{ ft}^3/\text{s}$$

$$= 0.1838 \text{ ft}^3/\text{s}$$

Q2 Determine discharge if velocity 2.5 m/s and dia of pipe 15mm, water flow through pipe. (L/sec unit)

Solution:  $Q = A V$

$$\Rightarrow Q = \left(\frac{\pi}{4} d^2\right) \times V$$

$$= \left(\frac{3.1416}{4} \times 0.015^2\right) \times 2.5$$

$$= 4.418 \times 10^{-4} \text{ m}^3/\text{s}$$

$$= 0.442 \text{ Litre/sec}$$

(Ans.)

$$Q = ?$$

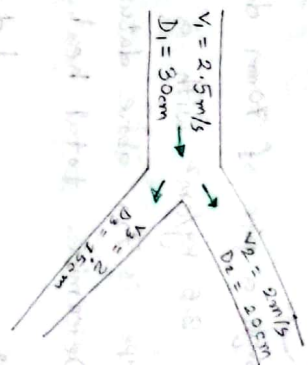
$$V = 2.5 \text{ m/s}$$

$$d = 15 \text{ mm}$$

$$= 0.015 \text{ m}$$

$$1 \text{ m}^3 = 1000 \text{ L}$$

# Q1



$$Q_1 = Q_2 + Q_3$$

$$\Rightarrow A_1 V_1 = A_2 V_2 + A_3 V_3$$

$$\Rightarrow \left(\frac{\pi}{4} D_1^2\right) \times V_1 = \left(\frac{\pi}{4} D_2^2\right) \times V_2 + \left(\frac{\pi}{4} D_3^2\right) \times V_3$$

$$\Rightarrow \left(\frac{\pi}{4} \times 0.3^2\right) \times 2.5 = \left(\frac{\pi}{4} \times 0.2^2\right) \times V_2 + \left(\frac{\pi}{4} \times 0.15^2\right) \times V_3$$

$$\therefore V_3 = 6.44 \text{ m/s}$$

$$Q_1 = A_1 \times V_1$$

$$Q_1 = \left(\frac{\pi}{4} d_1^2\right) \times V_1$$

$$= 0.0785 \text{ m}^3/\text{s}$$

$$= 78.54 \text{ L/s}$$

$$A_1 V_1 = A_2 V_2$$

$$\Rightarrow 0.0785 = \left(\frac{\pi}{4} \times 0.2^2\right) \times V_2$$

$$\therefore V_2 = 2.499 \text{ m/s}$$

(Ans.)

$$D_1 = 30 \text{ cm} = 0.3 \text{ m}$$

$$D_2 = 20 \text{ cm} = 0.2 \text{ m}$$

$$D_3 = 15 \text{ cm} = 0.15 \text{ m}$$

$$d_1 = 10 \text{ cm} = 0.1 \text{ m}$$

$$V_1 = 10 \text{ m/s}$$

$$Q = ?$$

$$d_2 = 20 \text{ cm} = 0.2 \text{ m}$$

$$V_2 = ?$$

$$Q_1 = Q_2 = Q$$

Continuity Equation

### # Water Head:

Water flowing through a pipe of 70mm dia under a gauge pressure of 3.5 kg/cm<sup>2</sup> with a mean velocity of 1.5 m/s. The pipe is 7m above datum line. Neglecting ~~negative~~ friction. Determine total head of water.

Solution: Total Head,

$$H = \frac{P}{\rho} + \frac{V^2}{2g} + Z$$

$$= \frac{3.5 \times 100^2}{1000} + \frac{(1.5)^2}{2 \times 9.81} + 7$$

$$= 42.115 \text{ m}$$

(Ans.)

Neglecting Friction. So, No Head Loss.

d = 70mm  
P = 3.5 kg/cm<sup>2</sup>  
= 3.5 × 100<sup>2</sup> kg/m<sup>2</sup>

V = 1.5 m/s  
Z = 7m  
ρ = 1000 kg/m<sup>3</sup>

Q2) Calculate the total head for Z<sub>1</sub> = 7m, V = 25 m/s, P = 25 kN/m<sup>2</sup>, Neglect friction.

Solution: Total Head,

$$H = \frac{P}{\rho} + \frac{V^2}{2g} + Z_1$$

$$= \frac{25}{9.81} + \frac{25^2}{2 \times 9.81} + 7$$

$$= 41.40 \text{ m}$$

(Ans.)

ρ = 9.81 kN/m<sup>3</sup>

g = 9.81 m/s<sup>2</sup>

Z<sub>1</sub> = Elevation

### # Bernoulli's Equation:

Sol Solution:

No energy loss, h<sub>f</sub> = 0

V<sub>A</sub> =  $\frac{Q}{A_A}$  = 16.81 ft<sup>3</sup>/s

V<sub>B</sub> =  $\frac{Q}{A_B}$  = 4.202 ft/s

From Bernoulli's Equation,

$$\frac{P_A}{\rho} + \frac{V_A^2}{2g} + Z_A = \frac{P_B}{\rho} + \frac{V_B^2}{2g} + Z_B$$

$$\Rightarrow 22.1 + \frac{16.81^2}{2 \times 32.2} + 10 = \frac{P_B}{\rho} + \frac{4.202^2}{2 \times 32.2} + 25$$

∴ Pressure head  $\frac{P_B}{\rho} = 11.21 \text{ ft}$  (Ans.)

d<sub>A</sub> = 12 in =  $\frac{10}{12}$  ft

∴ A<sub>A</sub> =  $\frac{\pi}{4} d_A^2 = 0.7854 \text{ ft}^2$

d<sub>B</sub> = 24 in = 2 ft

A<sub>B</sub> =  $\frac{\pi}{4} d_B^2 = 3.1416 \text{ ft}^2$

Q = Q<sub>A</sub> = Q<sub>B</sub> = 43.2 cfs

P<sub>A</sub> = 22.1 ft

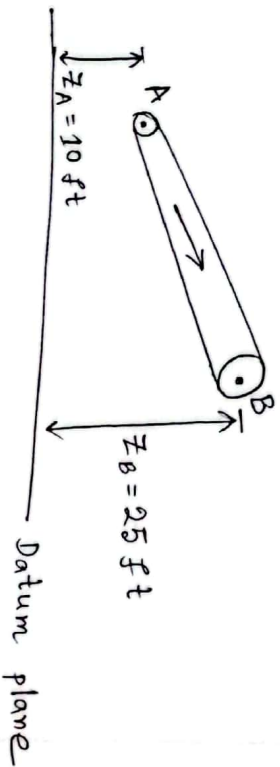
P<sub>B</sub> = ? = Pressure head

Z<sub>A</sub> = 10 ft, Z<sub>B</sub> = 25 ft

$\frac{P_A}{\rho} \rightarrow \frac{P_B}{\rho} = \text{Pressure Head}$   
g = 32.2 ft/s<sup>2</sup>

### # Bernoulli's Energy Equation

201 Water flows from A, where the diameter is 12 in, to B, where the diameter is 24 in, at the rate of 13.2 cfs. The pressure head at A is 22.1 ft. Considering no loss of energy from A to B, find the pressure head at B.



221

In a horizontal pipe of 30 cm diameter, water flows with a velocity 2 m/s and the pressure is 52 kN/m<sup>2</sup>. If the diameter of pipe is 60 cm in the other end, what is the pressure?

[BMDB-2019]

221

Water flows through a pipe from A to B at 80 l/sec, cross sectional area of the pipe at A and B is 20 cm<sup>2</sup> and 80 cm<sup>2</sup> respectively. The height of point B from A is 0.5 m and head loss is 0.05 m. If the pressure at point A is 50 Pa, find out the pressure at point B.

[PQCB-2021]

\* Q21 Horizontal pipe,  $Z_1 = Z_2$

$$A_1 = \frac{\pi}{4} d_1^2 = 0.071 \text{ m}^2$$

$$A_2 = \frac{\pi}{4} d_2^2 = 0.283 \text{ m}^2$$

$$A_1 V_1 = A_2 V_2$$

$$\Rightarrow 0.071 \times V_2 = 0.283 \times V_1$$

$$\therefore V_2 = 0.5 \text{ m/s}$$

$$\frac{P_1}{\rho} + \frac{V_1^2}{2g} + Z_1 = \frac{P_2}{\rho} + \frac{V_2^2}{2g} + Z_2$$

$$\Rightarrow \frac{P_1}{\rho} + \frac{V_1^2}{2g} = \frac{P_2}{\rho} + \frac{V_2^2}{2g}$$

$$\Rightarrow \frac{52}{9.81} + \frac{2^2}{2 \times 9.81} = \frac{P_2}{9.81} + \frac{0.5^2}{2 \times 9.81}$$

$$\therefore P_2 = 53.875 \text{ KN/m}^2$$

(Ans)

$$d_1 = 30 \text{ cm} = 0.3 \text{ m}$$

$$V_1 = 2 \text{ m/s}$$

$$P_1 = 52 \text{ KN/m}^2$$

$$d_2 = 60 \text{ cm} = 0.6 \text{ m}$$

$$P_2 = ?$$

$$\rho = 9.81 \frac{\text{KN}}{\text{m}^3}$$

\* Q21 Solution:

Flow from A to B.

$$\frac{P_A}{\rho} + \frac{V_A^2}{2g} + Z_A = \frac{P_B}{\rho} + \frac{V_B^2}{2g} + Z_B + h_L$$

$$\text{Now, } V_A = \frac{Q}{A_A} = 40 \text{ m/s}$$

$$V_B = \frac{Q}{A_B} = 10 \text{ m/s}$$

$$Q = 80 \text{ L/s} = \frac{80}{1000} \frac{\text{m}^3}{\text{s}}$$

$$= Q_A = Q_B$$

$$A_A = 20 \text{ cm}^2 = 0.002 \text{ m}^2$$

$$A_B = 80 \text{ cm}^2 = 0.008 \text{ m}^2$$

$$Z_B = 0.5 \text{ m}, Z_A = 0 \text{ m}$$

$$h_L = 0.05 \text{ m}$$

$$P_A = 50 \text{ Pa} = 50 \frac{\text{N}}{\text{m}^2}$$

$$P_B = ?$$

$$\rho = 9.81 \times 10^3 \text{ N/m}^3$$

$$\frac{P_A}{\rho} + \frac{V_A^2}{2g} + Z_A = \frac{P_B}{\rho} + \frac{V_B^2}{2g} + Z_B + h_L$$

$$\Rightarrow \frac{50}{9.81 \times 10^3} + \frac{40^2}{2 \times 9.81} + 0 = \frac{P_B}{9.81 \times 10^3} + \frac{10^2}{2 \times 9.81} + 0.5 + 0.05$$

$$\therefore P_B = 744654.5 \text{ Pa}$$

$$= 744654.5 \text{ N/m}^2$$

$$= 744.65 \text{ KN/m}^2$$

(Ans)

$$\text{KN/m}^2 = \text{KPa}$$

26]

A pipe discharge  $40 \text{ m}^3/\text{s}$ , Head loss is 10% of the difference of velocity head. Find out the pressure at point Q.

	Point P	Point Q
Elevation	1011 m	1050 m
Pipe Dia	50 cm	100 cm
Pressure	50 Kpa	???

[BWDB-2018]

28]

The value of pressure, elevation and diameter of a pipe in a point P are  $12 \text{ N/m}^2$ , 5m and 25 cm. Another point Q whose elevation and diameter are 3m and 17 cm. What will be the pressure at Q? Given discharge P to Q is  $2 \text{ m}^3/\text{s}$  and head loss 1m.

[BWDB-2016, WASA-2017]

sol Solution:

$$A_p = \frac{\pi}{4} \times d_p^2 = 0.196 \text{ m}^2$$

$$A_q = \frac{\pi}{4} \times d_q^2 = 0.785 \text{ m}^2$$

$$V_p = \frac{Q}{A_p} = 204.08 \text{ m/s}$$

$$V_q = \frac{Q}{A_q} = 50.96 \text{ m/s}$$

$$h_L = 10\% \text{ of } \left( \frac{V_p^2}{2g} - \frac{V_q^2}{2g} \right)$$

$$= \frac{10}{100} \times \left( \frac{204.08^2}{2 \times 9.81} - \frac{50.96^2}{2 \times 9.81} \right)$$

$$= 199.01 \text{ m}$$

$$\frac{P_p}{\rho} + \frac{V_p^2}{2g} + Z_p = \frac{P_q}{\rho} + \frac{V_q^2}{2g} + Z_q + h_L$$

$$\Rightarrow \frac{50}{9.81} + \frac{204.08^2}{2 \times 9.81} + 1011 = \frac{P_q}{9.81} + \frac{50.96^2}{2 \times 9.81} + 1050 + 199.01$$

$$\Rightarrow P_q = 17240.69 \text{ KPa} = 17240.69 \text{ KN/m}^2 \quad (\text{Ans})$$

$$Q = 40 \text{ m}^3/\text{s}$$

$$h_L = 10\% \text{ of } \left( \frac{V_p^2}{2g} - \frac{V_q^2}{2g} \right)$$

$$P_q = 2$$

$$P_p = 50 \text{ KPa}$$

$$= 50 \text{ KN/m}^2$$

$$d_p = 50 \text{ cm} = 0.5 \text{ m}$$

$$d_q = 100 \text{ cm} = 1 \text{ m}$$

$$\gamma_w = 9.81 \text{ KN/m}^3$$

Hint: For pipe of uniform diameter,

$$\frac{V_1^2}{2g} = \frac{V_2^2}{2g}$$

sol Solution:

$$A_p = \frac{\pi}{4} \times d_p^2 = 0.05 \text{ m}^2$$

$$A_q = \frac{\pi}{4} \times d_q^2 = 0.023 \text{ m}^2$$

$$V_p = \frac{Q}{A_p} = 40 \text{ m/s}$$

$$V_q = \frac{Q}{A_q} = 86.96 \text{ m/s}$$

From Bernoulli's Equation,

$$\frac{P_p}{\rho} + \frac{V_p^2}{2g} + Z_p = \frac{P_q}{\rho} + \frac{V_q^2}{2g} + Z_q + h_L$$

$$\Rightarrow \frac{12}{9.81 \times 10^3} + \frac{40^2}{2 \times 9.81} + 5 = \frac{P_q}{9.81 \times 10^3} + \frac{86.96^2}{2 \times 9.81} + 3 + 1$$

$$\Rightarrow P_q = -29711.98 \text{ N/m}^2 = -2971.2 \text{ KN/m}^2 \quad (\text{Ans})$$

(Pressure Negative  $\rightarrow$  Not Possible)

$$P \Rightarrow P_p = 12 \text{ N/m}^2$$

$$Z_p = 5 \text{ m}$$

$$d_p = 25 \text{ cm} = 0.25 \text{ m}$$

$$Q \Rightarrow$$

$$Z_q = 3 \text{ m}$$

$$d_q = 17 \text{ cm} = 0.17 \text{ m}$$

$$P_q = ?$$

$$Q = Q_p = Q_q = 2 \text{ m}^3/\text{s}$$

$$h_L = 1 \text{ m}$$

$$\gamma_w = 9.81 \times 10^3 \text{ N/m}^3$$

Q5] Water is flowing in a Fine nose with velocity of 1 m/s with pressure 200 kPa. At the nozzle, the pressure decreases to atmosphere pressure 101.3 kPa and there is no change in height. Calculate the velocity of water exiting at the nozzle. [NESCO-2021]



Solution:

No change in height.

$$\frac{P_1}{\rho} + \frac{V_1^2}{2g} + Z_1 = \frac{P_2}{\rho} + \frac{V_2^2}{2g} + Z_2$$

$$\Rightarrow \frac{P_1}{\rho} + \frac{V_1^2}{2g} = \frac{P_2}{\rho} + \frac{V_2^2}{2g}$$

[∵ No change in ht.]

$$\Rightarrow \frac{200}{9.81} + \frac{1^2}{2 \times 9.81} = \frac{101.3}{9.81} + \frac{V_2^2}{2 \times 9.81}$$

$$\therefore V_2 = 14.085 \text{ m/s} \quad (\text{Ans})$$

$V_1 = 1 \text{ m/s}$   
 $P_1 = 200 \text{ kPa}$   
 $= 200 \text{ kN/m}^2$   
 $P_2 = 101.3 \text{ kPa}$   
 $= 101.3 \text{ kN/m}^2$   
 $V_2 = ?$   
 $\rho = 9.81 \text{ kN/m}^3$

# Head Loss; Reynolds's Number:

Q6] Water at 40°C flow in a 75mm pipe @ Reynolds's number 80000. If the pipe contains a uniform sand roughness of grain size 0.12mm dia, how much head loss is to be expected in 300m of the pipe?

Solution:

$$f = 1.76 \sqrt{d_{mm}}$$

$$= 1.76 \times \sqrt{0.12}$$

$$= 0.61 = f'$$

Reynolds's number,

$$Re = \frac{VD}{\nu}$$

$$\Rightarrow 80000 = \frac{V \times 0.075}{0.66 \times 10^{-6}}$$

$$\therefore V = 0.704 \text{ m/s}$$

$$\text{Head loss, } h_L = \frac{f' L V^2}{2g d}$$

$$= \frac{0.61 \times 300 \times 0.704^2}{2 \times 9.81 \times 0.075}$$

$$= 61.64 \text{ m} \quad (\text{Ans})$$

$D = 75 \text{ mm} = 0.075 \text{ m}$   
 $Re = 80000$   
 $d_{mm} = 0.12 \text{ mm}$   
 $h_L = ?$   
 $L = 300 \text{ m}$   
 Water at 40°C,  
 $\nu = 0.66 \times 10^{-6} \text{ m}^2/\text{s}$   
 $D = \text{dia} = d$   
 $d_{mm} = \text{grain size}$



31] Solution:

$$Re = \frac{VD}{\mu}$$

$$\Rightarrow Re = \frac{VD}{\left(\frac{\mu}{\rho}\right)} \quad \left[ \mu = \frac{\mu}{\rho} \right]$$

$$\therefore Re = \frac{VD\rho}{\mu}$$

$$\Rightarrow 2300 = \frac{V \times 0.05 \times 804}{0.00192}$$

$$\therefore V = 0.11 \text{ m/s}$$

$$\therefore Q = AV$$

$$= \left(\frac{\pi}{4} d^2\right) \times V$$

$$= 2.16 \times 10^{-4}$$

$$\text{m}^3/\text{s}$$

$$\text{(Am.)}$$

$$Re = 2300$$

$$D = d = 5 \text{ cm} = 0.05 \text{ m}$$

$$Q = ? \text{ (m}^3/\text{s)}$$

$$\rho = 804 \text{ Kg/m}^3$$

$$\mu = 0.00192 \text{ Kg/m}\cdot\text{s}$$

32] Solution: Head Loss,

$$h_L = \frac{(4f) L V^2}{2gD}$$

$$= \frac{(4 \times 0.005) \times 100 \times 2.037^2}{2 \times 9.81 \times 0.5}$$

$$= 0.846 \text{ m}$$

$$\text{(Am.)}$$

$$\text{dia } d = 0.5 \text{ m}$$

$$L = 100 \text{ m}$$

$$Q = 0.4 \text{ m}^3/\text{s}$$

$$f = 0.005$$

$$h_L = ?$$

$$V = \frac{Q}{A} = \frac{Q}{\frac{\pi}{4} d^2}$$

$$= 2.037 \text{ m/s}$$

33] Solution:

Difference of pressure head,  $h_L = 4 \text{ m}$

$$\text{Now, } h_L = \frac{(4f) L V^2}{2gD}$$

$$\Rightarrow 4 = \frac{(4 \times 0.009) \times 500 \times V^2}{2 \times 9.81 \times 0.2}$$

$$\therefore V = 0.934 \text{ m/s}$$

Discharge,

$$Q = AV$$

$$= 0.031 \times 0.934$$

$$= 0.029 \text{ m}^3/\text{s}$$

$$d = 200 \text{ mm}$$

$$= 0.2 \text{ m}$$

$$A = \frac{\pi}{4} d^2$$

$$= 0.031 \text{ m}^2$$

$$L = 500 \text{ m}$$

$$f = 0.009$$

$$Q = ?$$

# Shear stress ; Critical Shear ;  
Critical Shear Velocity :

22] A plate separated by 0.5 mm from a fixed plate moves at 0.50 m/s under a force per unit area of  $4 \text{ N/m}^2$ .

Determine the viscosity of the fluid between the plates.

23] If co-efficient of dynamic viscosity,  $\mu = 1 \frac{\text{N-sec}}{\text{m}^2}$  and velocity distribution,

$v = 0.9 - y^2$ , Determine the shear stress if  $y = 0.45$ , [BMDB-2016]

26] Find the kinematic viscosity of an oil having density  $981 \text{ kg/m}^3$ . The shear stress at a point in oil is  $0.2452 \text{ N/m}^2$  and velocity gradient at that point is 0.2 per second.

27] The dimensionless effective stress is  $0.045 \frac{\text{N}}{\text{m}^2}$ . Find (i) effective velocity (ii) incipient depth of a wide river channel with the bed sediment size of 0.2 mm with longitudinal river bed slope of 0.0001, specific gravity is 2.65, [BMDB-2016]

29] A rectangular channel of 4 m depth and 6 m width has a side slope of 0.0001, calculate the shear velocity, [BUET M.Sc - 2018]

### # Shear stress ; Critical Shear; Critical Shear Velocity;

20] If the coefficient of dynamic viscosity is

$\mu = 1 \frac{\text{N}\cdot\text{s}}{\text{m}^2}$  and velocity distribution is  $0.9 \text{ m/s}$ ,

Determine shear stress at  $y = 0.45 \text{ m}$ .

[GTCL-2016]

Solution:

$$\tau = \mu \frac{v}{y}$$

$$\Rightarrow \tau = 1 \times \frac{0.9}{0.45} \left( \frac{\text{N}\cdot\text{s}}{\text{m}^2} \times \frac{\text{m}}{\text{s}} \times \frac{1}{\text{m}} \right)$$

$$\Rightarrow \tau = 2 \frac{\text{N}}{\text{m}^2} = 2 \text{ Pa} \quad (\text{Ans.})$$

Hints:

$\nu$  = Kinematic viscosity  $\Rightarrow \nu = \frac{\mu}{\rho}$   $\left( \frac{\text{cm}^2}{\text{s}}, \text{Stoke} \right)$

$\tau$  = Shear stress  $\left( \frac{\text{N}}{\text{m}^2}, \text{Pa} \right)$

$\mu$  = Coefficient of Dynamic Viscosity  $\left( \frac{\text{N}\cdot\text{s}}{\text{m}^2}, \text{Pa}\cdot\text{s}, \text{poise} \right)$

$v$  = Velocity distribution  $\left( \text{m/s} \right)$

$y$  = Separated distance  $\left( \text{m} \right)$

$\frac{v}{y}$  = Velocity Gradient  $\left( 1/\text{s} \right)$

$\mu$  = Viscosity  $\left( \text{Pa}\cdot\text{s}, \text{poise} \right)$

$$1 \text{ Poise} = \frac{1}{10} \frac{\text{N}\cdot\text{s}}{\text{m}^2}$$

$$1 \frac{\text{cm}^2}{\text{s}} = 1 \text{ Stoke}$$

21]

$y = 0.5 \text{ mm} = 0.5 \times 10^{-3} \text{ m}$

$v = 0.50 \text{ m/s}$

$\tau = 4 \text{ N/m}^2$

$\mu = ?$

Solution:

$$\tau = \mu \frac{v}{y}$$

$$\Rightarrow 4 = \mu \times \frac{0.50}{0.5 \times 10^{-3}}$$

$$\Rightarrow \mu = 4 \times 10^{-3} \frac{\text{N}\cdot\text{s}}{\text{m}^2}$$

$$\therefore \mu = 4 \times 10^{-3} \text{ Pa}\cdot\text{s} \quad (\text{Ans.})$$

22]

$\mu = 1 \frac{\text{N}\cdot\text{sec}}{\text{m}^2}$

$v = 0.9 - y^2$

$\tau = ?$

$y = 0.45$

Solution:

$$\tau = \mu \frac{v}{y}$$

$$\Rightarrow \tau = 1 \times \frac{0.9 - y^2}{0.45}$$

$$= 1 \times \frac{0.9 - 0.45^2}{0.45}$$

$$\therefore \tau = 1.55 \frac{\text{N}}{\text{m}^2} \quad (\text{Ans.})$$

\* 261  $v = ?$   $f = 981 \text{ kg/m}^3 \text{ cm}^3 = 1$

$\tau = 0.2452 \text{ N/m}^2$

$\frac{v}{f} = 0.2 \text{ /sec}$

$\tau = \mu \frac{v}{f}$

$\Rightarrow \tau = (f v) \frac{v}{f}$

$\Rightarrow 0.2452 = (981 \times v) \times 0.2$

$\Rightarrow v^2 = 1.25 \times 10^{-3} \text{ m}^2/\text{s}$

$\Rightarrow v = 1.25 \times 10^{-3} \times 100^2 \text{ cm}^2/\text{s}$

$\therefore v = 12.5 \text{ stoke (Am.)}$

$\mu = f v$   
 $v = \frac{\mu}{f}$

$1 \frac{\text{cm}^2}{\text{s}} = 1 \text{ stoke}$

$\frac{f \cdot 0 - 0}{3 \cdot 0} \times 2 = f \cdot 0$

$\frac{v}{f} = 0.2$

100.0 - 200.0 ... 30.0 - 20.0 ... make ...

... 100.0 = 200.0 ...

$100.0 = 2$

$B = A \dots$

$(P, T, 0.1)$

$100.0 \times 1 = 100.0$

# 28] Dimensionless critical shear = 0.45, slope = 0.001, sedimentation size 0.20 mm. Find out the critical shear stress of incipient depth. Channel is a wide channel. Assume necessary data. Determine the incipient depth. [PGC B-2017]

Solution:  $\tau_{DC} = 0.45$

$$S = 0.001$$

$$d_{mm} = 0.20 \text{ mm}$$

$$\therefore d_m = 2 \times 10^{-4} \text{ m}$$

$$\text{Wide channel} \rightarrow A = Y_0, R = Y_0$$

$$\text{Let, } \gamma_s = 2.5$$

$$\gamma_w = 9.81 \text{ KN/m}^3$$

(i) Critical shear stress,  $\tau_c \rightarrow \gamma_w$

$$\tau_c = \tau_{DC} (\gamma - \gamma_w) d_m$$

$$= \tau_{DC} (\gamma_s \gamma_w - \gamma_w) d_m$$

$$= 0.45 \times (2.5 \times 9.81 - 9.81) \times (2 \times 10^{-4})$$

$$= 1.32 \times 10^{-3} \text{ KN/m}^2$$

(ii)  $\tau_c = \gamma_w R S$

$$\Rightarrow \tau_c = \gamma_w Y_0 S$$

[wide channel,  $R = Y_0$ ]

$$\Rightarrow (1.32 \times 10^{-3}) = 9.81 \times Y_0 \times 0.001$$

$$\therefore Y_0 = 0.135 \text{ m}$$

# 29]  $\tau_{DC} = 0.045 \text{ N/m}^2$

$$V = 2$$

$$S = 0.0001$$

$$\gamma_s = 2.65$$

Solution: (ii)  $\tau_c = \tau_{DC} (\gamma - \gamma_w) d_m$

$$= \tau_{DC} (\gamma_s \gamma_w - \gamma_w) d_m$$

$$= 0.045 \times (2.65 \times 9.81 - 9.81) \times (2 \times 10^{-4})$$

$$= 1.46 \times 10^{-4} \text{ KN/m}^2$$

$$\tau_c = \gamma_w R S$$

$$\Rightarrow \tau_c = \gamma_w \times Y_0 \times S$$

[wide River channel]

$$\Rightarrow 1.46 \times 10^{-4} = 9.81 \times Y_0 \times 0.0001$$

$$\therefore Y_0 = 0.149 \text{ m}$$

(Ans.)

(P.T.O.)

(i) Effective velocity,

$$V = \frac{1}{n} R^{2/3} S^{1/2}$$

$$\Rightarrow V = \frac{1}{n} (y)^{2/3} S^{1/2}$$

$$\Rightarrow V = \frac{1}{0.02} \times (0.149)^{2/3} \times (0.0001)^{1/2}$$

$$\therefore V = 0.141 \text{ m/s}$$

(Am.)

Let,  $\eta = 0.02$

[wide River Channel,  $A = y$ ,  $R = y$ ]

28] If depth is 5m and slope is 1:2, Then find critical shear velocity. Wide channel. (ii)

Solution: Critical shear velocity,

$$V_c = \sqrt{g R S}$$

$$y = 5m$$

$$S = 1:2$$

$$\Rightarrow V_c = \sqrt{g y S}$$

[Wide Channel,  $\frac{A}{R} = \frac{1}{2}$ ]

$$= \sqrt{9.81 \times 5 \times \frac{1}{2}}$$

$$\therefore V_c = 4.95 \text{ m/s}$$

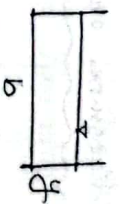
29]

$$y = 4m$$

$$b = 6m$$

$$S = 0.0001$$

$$V_c = ?$$



Solution:  $R = \frac{A}{P} = \frac{by}{b+2y}$

$$\Rightarrow R = \frac{6 \times 4}{6 + 2 \times 4} = 1.71 \text{ m}$$

shear velocity,

$$V_c = \sqrt{g R S}$$

$$= \sqrt{9.81 \times 1.71 \times 0.0001}$$

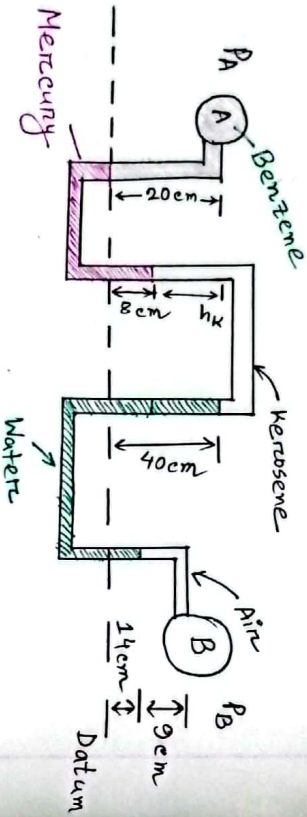
$$= 0.041 \text{ m/s}$$

(Am.)

## # Pressure on U-Shape Manometers

25

Determine  $\Delta P$  between points A and B. All fluids are at  $20^\circ\text{C}$ . Take the specific weights of Benzene  $8640 \text{ N/m}^3$ , Mercury  $133100 \text{ N/m}^3$ , Kerosene  $7885 \text{ N/m}^3$ , water  $9790 \text{ N/m}^3$  and Air  $12 \text{ N/m}^3$ .



20

For the manometer of figure, all fluids are at  $20^\circ\text{C}$ . If  $P_B - P_A = 97 \text{ kPa}$ , Determine the height  $H$  in centimeters. Take the specific weights of mercury  $133100 \text{ N/m}^3$  and water  $9790 \text{ N/m}^3$ .

$\gamma = \rho g$

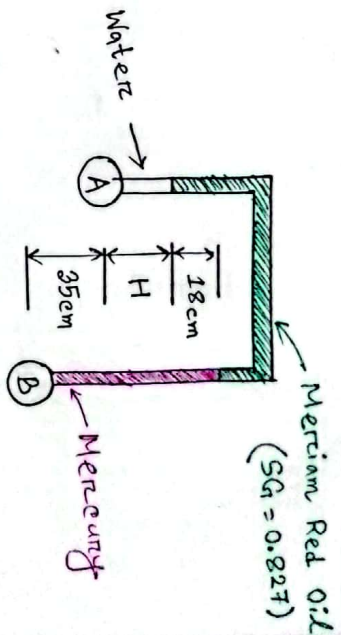
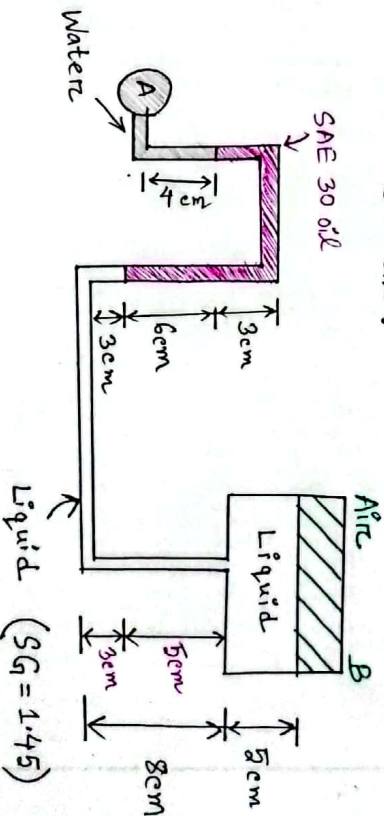


Fig: Inverted manometer.

In figure the pressure at point A is  $25 \text{ psi}$ . All fluids are at  $20^\circ\text{C}$ . What is the air pressure in the closed chamber B?  $\gamma = 9810 \text{ N/m}^3$  for water,  $8720 \text{ N/m}^3$  for SAE 30 oil.



# Pressure on U-Shape Manometer:

$$P = h \gamma \quad \text{(Concept)}$$

$$= h \rho g$$

$$= h \times (G_s \gamma_w)$$

\* Q6] Solution:

$$P_A + h_B \gamma_B - h_m \gamma_m - h_K \gamma_K + h_w \gamma_w = P_B + h_A \gamma_A$$

$$\Rightarrow P_A + 0.2 \times 8640 - 0.08 \times 133100 - 0.32 \times 7885 + 0.26 \times 9790$$

$$= P_B + 0.09 \times 12$$

$$\Rightarrow P_A - P_B = 8839 \quad \text{N/m}^2 \quad \text{(Ans.)}$$

\* Q7] Solution:

Given,  $P_B - P_A = 97 \text{ kPa}$

$$H = ?$$

For inverted manometer,

$$P_A - h_w \gamma_w - h_{mro} \gamma_{mro} = P_B - h_m \gamma_m$$

$$\Rightarrow P_B - P_A = \gamma_m h_m - H \times \gamma_w - h_{mro} \gamma_{mro}$$

$$\Rightarrow 97 \times 10^3 = 133100 \times (1.8 + H + 0.35) - H \times 9790 - 0.18 \times (9790 \times 0.827)$$

$$\Rightarrow H = 0.2264 \text{ m}$$

$$\therefore H = 22.64 \text{ cm} \quad \text{(Ans.)}$$

\* Q9]  $P_A = 25 \text{ psi} = 172414 \text{ N/m}^2$

$$P_{air} = ?$$

Solution:

$$P_A - h_w \gamma_w + h_{SAE} \gamma_{SAE} = h_L \gamma_L + P_{air}$$

$$\Rightarrow 172414 - 0.04 \times 9810 + 0.06 \times 8720 = (0.05 + 0.05) \times (9810 \times 1.45) + P_{air}$$

$$\therefore P_{air} = 171122 \quad \text{N/m}^2 \quad \text{(Ans.)}$$

\* Q10] Solution:

$$P_A + h_1 \gamma_1 + h_2 \gamma_2 = P_B + h_3 \gamma_3$$

~~Solution:~~

$$P_A + h_1 \gamma_1 = h_2 \gamma_2$$

$$\Rightarrow P_A + 0.05 \times 9.81 = 0.15 \times (9.81 \times 13.6)$$

$$\therefore P_A = 19522 \text{ KN/m}^2$$

$$= 19522 \text{ N/m}^2$$

$$\text{(P.T.O.)}$$

$$1 \text{ Mpa} = 145 \text{ psi}$$

$$\Rightarrow 1 \times 10^6 \frac{\text{N}}{\text{m}^2} = 145 \text{ psi}$$

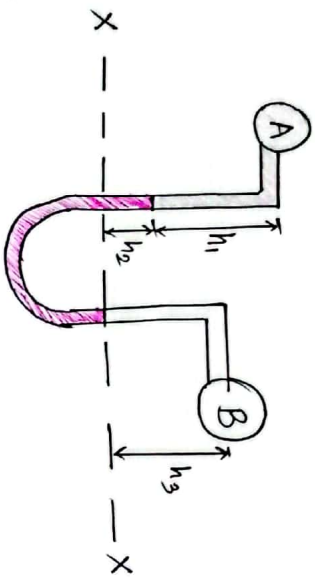
$$\therefore 1 \text{ psi} = \frac{10^6}{145} \frac{\text{N}}{\text{m}^2}$$

$$\gamma_S = 13.6$$

$$P_A = ?$$

$$\gamma_w = 9.81 \text{ KN/m}^3$$

Q21



$$G_2 (A) = 1.5$$

$$G_2 (B) = 0.9$$

What is the Pressure Equation?

Solution:

$$P_A + h_1 \gamma_1 + h_2 \gamma_2 = P_B + h_3 \gamma_3$$

$$\Rightarrow P_A + h_1 \rho_1 g + h_2 \rho_2 g = P_B + h_3 \rho_3 g$$

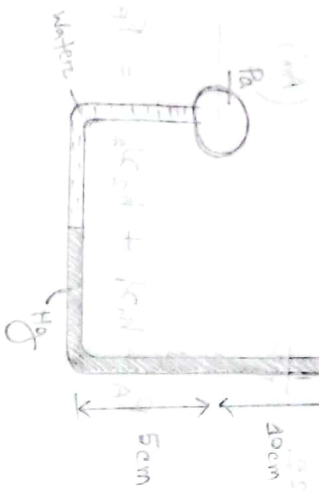
(Ans.)

Q22

A 15 cm diameter pipe carries 70 L/s of oil ( $G = 0.75$ ). At a section 15 cm above the datum, the pressure is 2 cm vacuum of mercury. If  $\alpha = 1.1$  then calculate the total head in A of the oil section.

[BIWTA-2019]

Q21 A U-type manometer containing Hg ( $G_s = 13.6$ ) has its right limb opened to the atmosphere. The left limb is full of water and is connected to a pipe containing water under pressure. Find the pressure of water in pipe above atmosphere for the manometer reading. [BMDB-2019]



Solution:

$$P_a + h_w \gamma_w = h_{Hg} \gamma_{Hg}$$

$$\Rightarrow P_a + h_w \gamma_w = h_{Hg} \times (\gamma_w \times G_s)$$

$$\Rightarrow P_a + 0.05 \times 9.81 = 0.15 \times (9.81 \times 13.6)$$

$$\therefore P_a = 19.522 \text{ KN/m}^2$$

(Ans.)

$$\gamma_w = 9.81 \text{ KN/m}^3$$

# Extra

Q21 Dia,  $d = 15 \text{ cm} = 0.15 \text{ m}$

$$Q = 70 \text{ L/s} = 70 \times 10^{-3} \text{ m}^3/\text{s}$$

$$Z = 15 \text{ cm} = 0.15 \text{ m}$$

$$G_s = 0.75, \quad \alpha = 1.1$$

$P = 2 \text{ cm}$  vacuum of mercury

Total Head,  $H = ?$  |  $G_s(\text{mercury}) = 13.6$

Solution:

$$\text{Area, } A = \frac{\pi}{4} d^2 = 0.0176 \text{ m}^2$$

$$\text{Velocity, } v = \frac{Q}{A} = 3.977 \text{ m/s}$$

Pressure,  $P = 2 \text{ cm}$  vacuum of mercury

$$P = h_m \gamma_m$$

$$= h_m (\gamma_w \times G_s(\text{mercury}))$$

$$= \frac{2}{100} \times 9.81 \times 13.6$$

$$= 2.668 \text{ KN/m}^2$$

$$\gamma_w = \gamma_w \times G_s(\text{air}) = 7.36 \text{ KN/m}^3$$

$$\text{Total Head, } H = \frac{P}{\gamma} + \alpha \frac{v^2}{2g} + Z$$

$$= \frac{2.668}{7.36} + 1.1 \times \frac{3.977^2}{2 \times 9.81} + 0.15$$

$$= 1.4 \text{ m}$$

(Ans.)

\* Q81 You have planned to irrigate a garden by a lush

well using a centrifugal pump at a rate of 10 gpm through 1" pipe. At the lower point of the water level is 50' below the garden level. You will need a 75' long plastic pipe (including horizontal length). According to the chart, the frictional head loss per 100 ft length of 1" plastic pipe is 6.3 ft and the same one 90° below connector is 16.655 ft. Calculate water HP and motor HP for efficiency 0.5.

[NPBCL-2019]

Solution:  $Q = 10 \text{ gpm}$

$$d = 1 \text{ in (No use)}$$

$$h = 50'$$

$$L = 25'$$

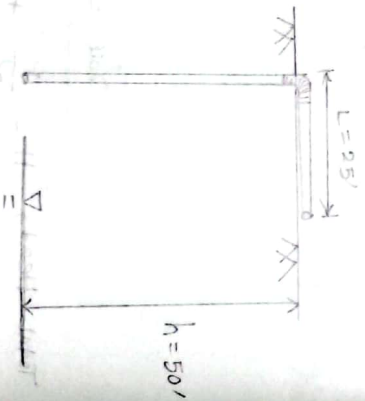
$$L_{\text{Total}} = 50 + 25 = 75'$$

$$\eta = 0.5$$

$$\text{Water HP} = ?$$

$$\text{Motor HP} = ?$$

Friction head loss = 6.3' per 100' length



Now, Frictional head loss,

$$h_f = \left( \text{Head loss for } \right) + \left( \text{Head loss below connector} \right)$$

$$= \left( 6.3' \times \frac{75}{100} \right) + \left( 16.655' \right)$$

$$= 21.38'$$

$$\text{Total Head, } H = h + h_f$$

$$= 50 + 21.38$$

$$= 71.38 \text{ ft}$$

$$\text{Water HP} = \frac{HQ}{3960}$$

$$= \frac{71.38 \times 10}{3960}$$

$$= 0.18 \text{ (WHP)}$$

$$\text{Motor HP} = \frac{WHP}{\eta}$$

$$= \frac{0.18}{0.5}$$

$$= 0.36 \text{ (BHP)}$$

(Ans)

Q → gpm  
H → ft