

" Dhaka wasa - 2017 "

Q.1 The unconfined compression of the soil is 180 kPa. Determine the undrained cohesion.

Ans:

Given that,  $q_u = 180 \text{ kPa}$ .

$$C_u = ?$$

$$\therefore C = \frac{q_u}{2} = \frac{180}{2} = 90 \text{ kPa}$$

Q.2 Sieve Analysis were performed for a coarse sand & it has been found 20% sample retained on <sup>each</sup> #4, #8, #30, #40, #50, #100 & #200 sieve. Find the F.M. of sand.

Sol<sup>n</sup>:

Sieve No.	Percentage of Retained	Percentage of cumulative Retained
#4	20	20 ✓
#8	20	40 ✓
#30	20	60 ✓
#40	20	80 ✓
#50	20	100 ✓
#100	20	120 ✓
#200	20	140

$$\therefore \text{FM} = \frac{20 + 40 + 60 + 80 + 100 + 120}{100} = \frac{320}{100} = 3.2$$

(Ans)

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Q.3 What is the theoretical relationship between speed, density and volume for a highway.

Ans:

Under uninterrupted flow conditions, speed, density and flow are related by the following equation:

$$Q = K * V$$

where,

- Q = Flow (vehicles/hour)
- K = Density (vehicles/mile, vehicles/km)
- V = Speed (miles/hour, km/hour)

Q.4 Define BOD & COD. Why  $COD > BOD$ ?

Ans: Try yourself.

Q.5 Given that

# Singly Reinforced Beam.

Q.5

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A rectangular Beam has a width of 12" and effective depth 17.5". It is reinforced with 3 #7 bars. If  $f_y = 60 \text{ ksi}$ , &  $f_c = 4 \text{ ksi}$ ,  $f_s = 24 \text{ ksi}$ , &  $f_c' = 12 \text{ ksi}$ ,  $n = 9$  find the allowable Working Moment Capacity.

Sol<sup>n</sup>.

Given that,  $f_s = 24 \text{ ksi}$

$f_c' = 12 \text{ ksi}$

$A_s = 3 \times 0.6 = 1.8 \text{ in}^2$

$n = 9$

[N.B: if "h" not given then,  
 $n = \frac{E_s}{E_c}$ ,  $E_s = \text{Modulus of Elasticity of steel}$ .

$E_c = 57000 \sqrt{f_c'}$ ]

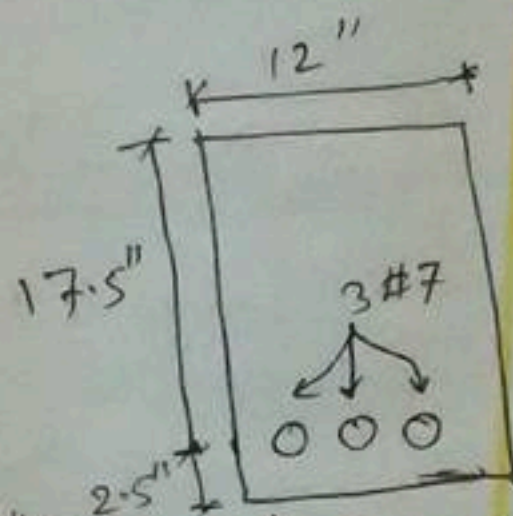
$$r = \frac{f_s}{f_c} = \frac{24}{1.2} = 20$$

$$k = \frac{n}{n+r} = \frac{9}{9+20} = 0.31$$

$$j = 1 - \frac{k}{3}$$

$$= 1 - \frac{0.32}{3}$$

$$= 0.90$$



Here,

$$k = \sqrt{(rn)^2 + 2pn} - rn$$

$$p = \frac{A_s}{bd}$$

$$\therefore p = \frac{1.8}{12 \times 17.5} = 857 \times 10^{-3}$$

$$\therefore k = \sqrt{\dots}$$

$$\therefore k = 0.32$$

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$$\text{Now, } \rho_b = \frac{n}{2b(n+n)} = \frac{9}{2 \times 20(9+20)} \\ = 7.76 \times 10^{-3}$$

$$n = \frac{f_s}{f_c} = \frac{24}{12} = 20$$

$\therefore \rho > \rho_b$ ; Hence, Compression failure occurs.

$$\therefore M = \frac{1}{2} f_c j k b d^2$$

$$= \frac{1}{2} \times 1.2 \times 0.9 \times 0.32 \times 12 \times 17.5^2$$

$$= 635.04 \text{ kip-inch. (Ans).}$$

[N.B.: if  $\rho < \rho_b$ ; then tension failure occurs,  
then,  $M = A_s f_s j d$ .  
&  $\rho = \rho_b$ ; Balanced reinforced }  
then,  $M = A_s f_s j d + \frac{1}{2} f_c j k b d^2$  ]

[ If it is said to do "Design" that means  
moment given, but,  $b, d$  &  $A_s$  value will be calculated.

Then, Procedure:  $k = \frac{n}{n+1}$ ,  $j = 1 - \frac{k}{3}$ ,  $\rho = \frac{f_s}{f_c}$ .

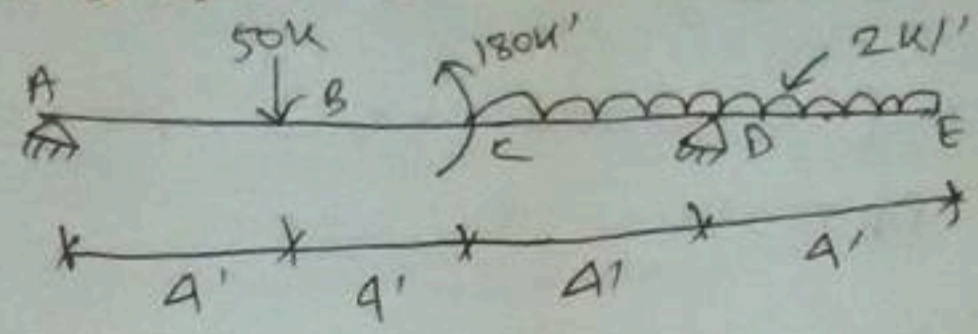
$$\rho_b = \frac{n}{2b(n+n)}, \quad R = \frac{1}{2} f_c j k, \quad M = R b d^2, \quad A_s = \rho_b b d$$

$$\text{or, } A_s = \frac{M}{f_s j d}$$

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Q.6

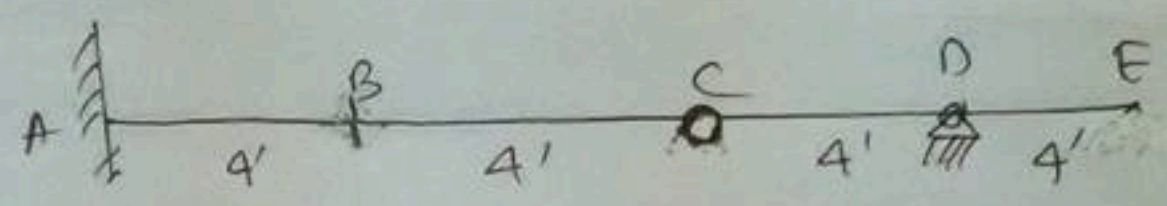
Draw SFD & BMD of the following beam:



Soln: Try yourself.

Q.7

Draw IL Diagram for  $R_A$ ,  $V_B$  &  $M_A$

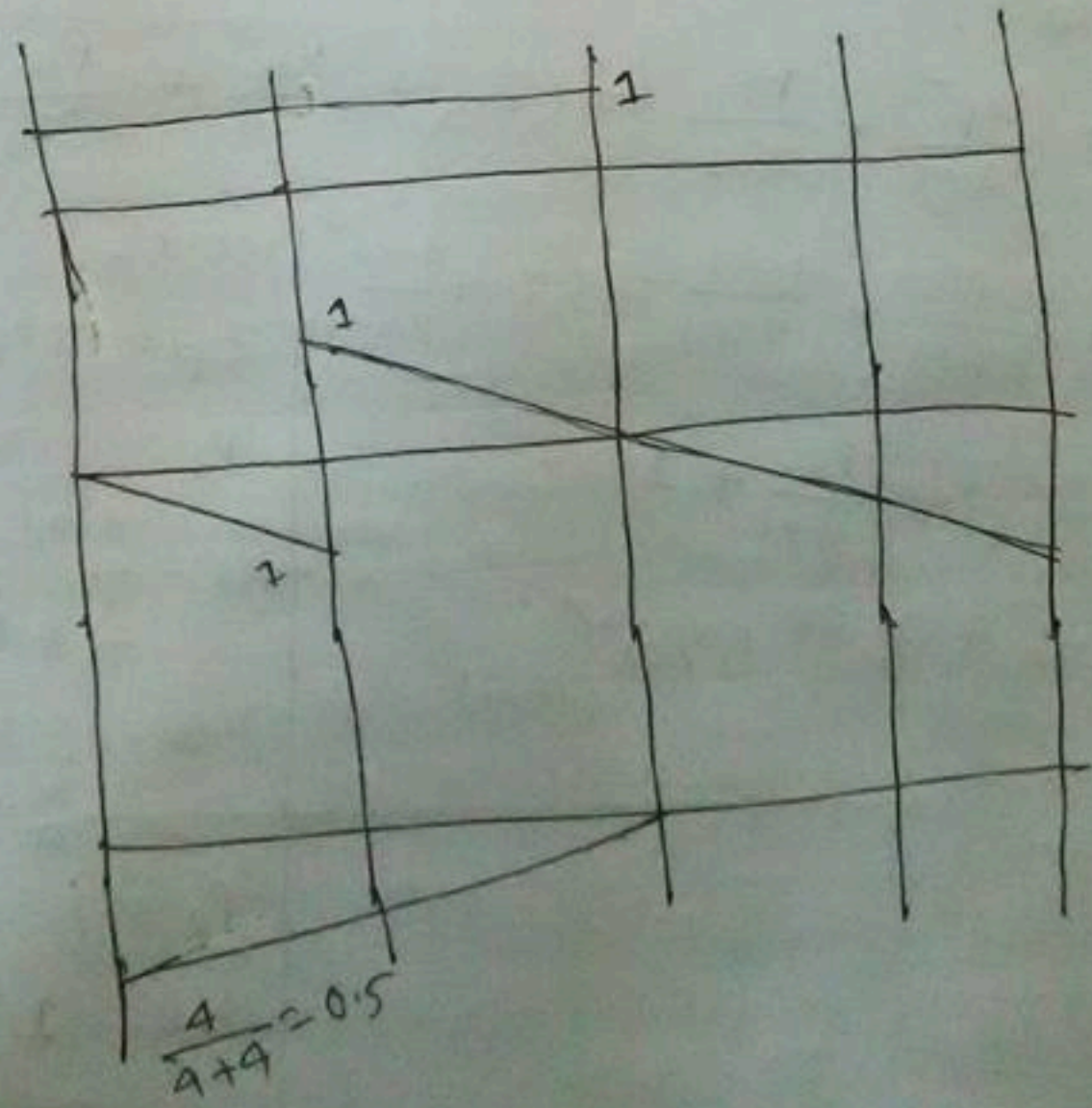


Soln:

$R_A \rightarrow$

$V_B \rightarrow$

$M_A \rightarrow$



$\frac{4}{4+4} = 0.5$

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Q.8 A pipe is laying at head of 525 m at one end, P and at head of 580 m at other end, Q.  $D_P = 50$  cm,  $D_Q = 75$  cm. Discharge from P to Q is  $0.02$  m<sup>3</sup>/sec. If pressure at point P is  $1000$  kN/m<sup>2</sup>, Determine the pressure at point Q. Assume head loss =  $1$  m. ~~Also consider pressure is~~

Sol<sup>n</sup>:

we know,

$$Z_P + \frac{V_P^2}{2g} + \frac{P_P}{\gamma_w} = \left( Z_Q + \frac{V_Q^2}{2g} + \frac{P_Q}{\gamma_w} \right) + H.L$$

$$\Rightarrow 525 + \frac{0.1^2}{2 \times 9.81} + \frac{1000}{9.81} = 580 + \frac{0.05^2}{2 \times 9.81}$$

$$+ \frac{P_Q}{9.81} + 1$$

$$\Rightarrow P_Q = 450.64 \text{ kN/m}^2 \text{ (Ans.)}$$

Here,

$$Z_P = 525 \text{ m.}$$

$$P_P = 1000 \text{ kN/m}^2$$

$$V_P = \frac{0.02}{\frac{\pi}{4} \times 0.5^2} = 0.1 \text{ m/s}$$

$$V_Q = \frac{0.02}{\frac{\pi}{4} \times 0.75^2} = 0.05 \text{ m/s}$$

$$P_Q = ?$$

$$H.L = 1 \text{ m}$$

$$g = 9.81 \text{ m/s}^2$$

$$\gamma_w = 9.81 \text{ kN/m}^3$$

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Q.9 Calculate volume of road embankment by trapezoidal rule:

Interval (feet)	100	200	300	400	500
Area (ft <sup>2</sup> )	10	25	30	50	60

Sol<sup>n</sup>.

Here, common interval,  $d = 100'$

$$\therefore \text{Volume} = \left( \frac{10+60}{2} + 25+30+50 \right) \times 100$$

$$= 14000 \text{ ft}^3$$

$$= 1301.31 \text{ m}^3 \text{ (Ans.)}$$

Q.10 A pipe diameter of 50 cm. is flowing a flow. The value of  $n = 0.016$ ,  $S = 0.001$ . Determine the discharge.

Sol<sup>n</sup>:

we know,  $Q = \frac{1}{n} A R^{2/3} S^{1/2}$

$$= \frac{1}{0.016} \times 0.196 \times (0.125)^{2/3} \times (0.001)^{1/2}$$

$$= 0.097 \text{ m}^3/\text{sec. (Ans.)}$$

Here,

$$n = 0.016$$

$$S = 0.001.$$

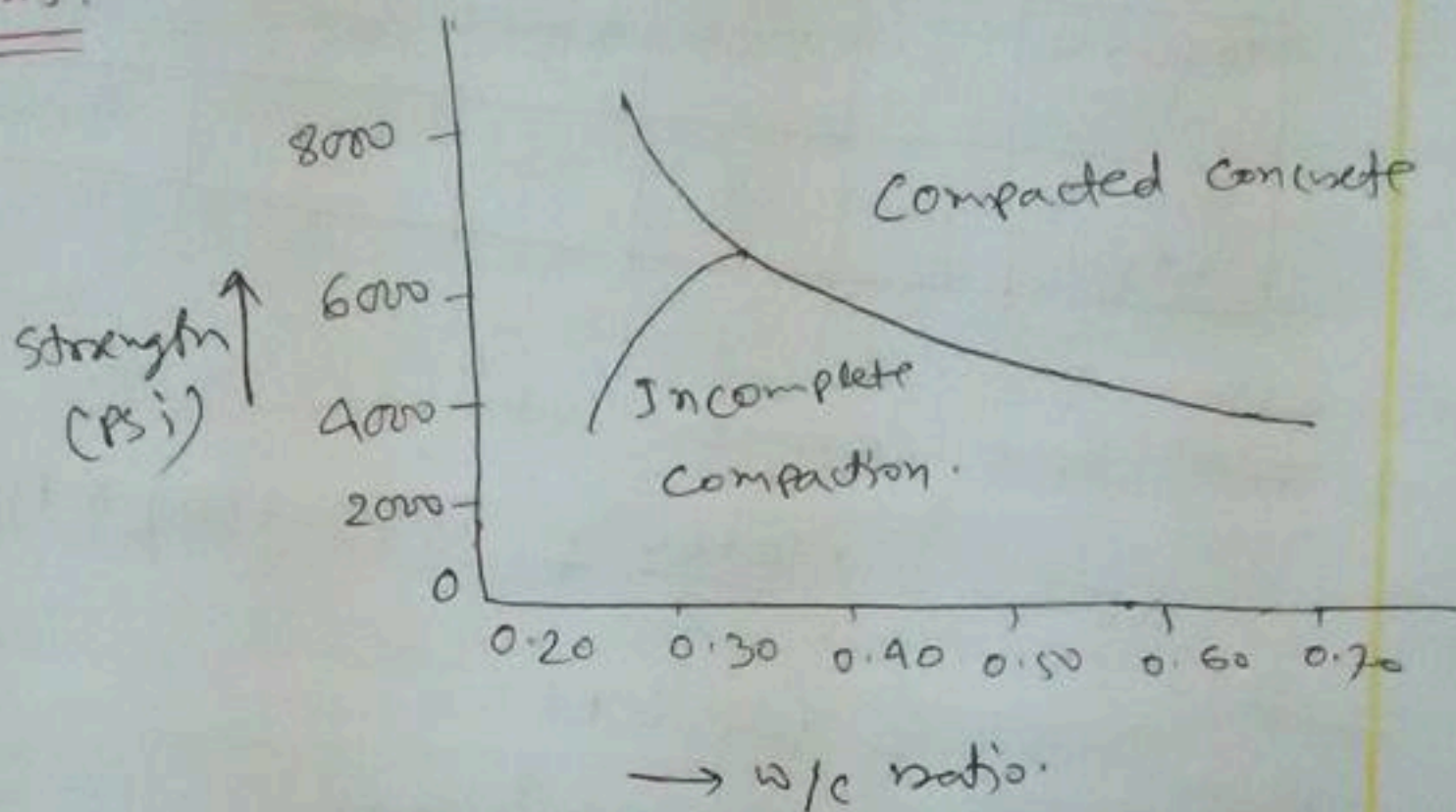
$$Q = ?$$

$$A = \frac{\pi}{4} \times 0.5^2 = 0.196 \text{ m}^2$$

$$R = \frac{A}{P} = \frac{A}{\pi d} = \frac{d}{4} = 0.125 \text{ m}$$

Q.11 Draw the graph between Compressive Strength of concrete and water cement ratio.

Ans:



Q.12 Axle load of 27k and 36 kips are repeated 30 and 15 times respectively on pavement of 24 hrs. What will be the Equivalent single Axle Load in a year?

Sol<sup>n</sup>:  $365 \text{ day} = 365 \times 24 = 8760 \text{ hr.}$

Sl. No.	Axle Load (k)	No. of Load Repetition (n)	EALF (F <sub>i</sub> )	F <sub>i</sub> n <sub>i</sub>
1.	27	$30 \times 365 = 10950$	$\left(\frac{27}{18}\right)^4 = 5.06$	55407
2.	36	$15 \times 365 = 5475$	$\left(\frac{36}{18}\right)^4 = 16$	87616
			Total =	143023 (Ans)

Q.13

Draw Diagram for attaining superelevation pavement rotating about centerline.

Ans:

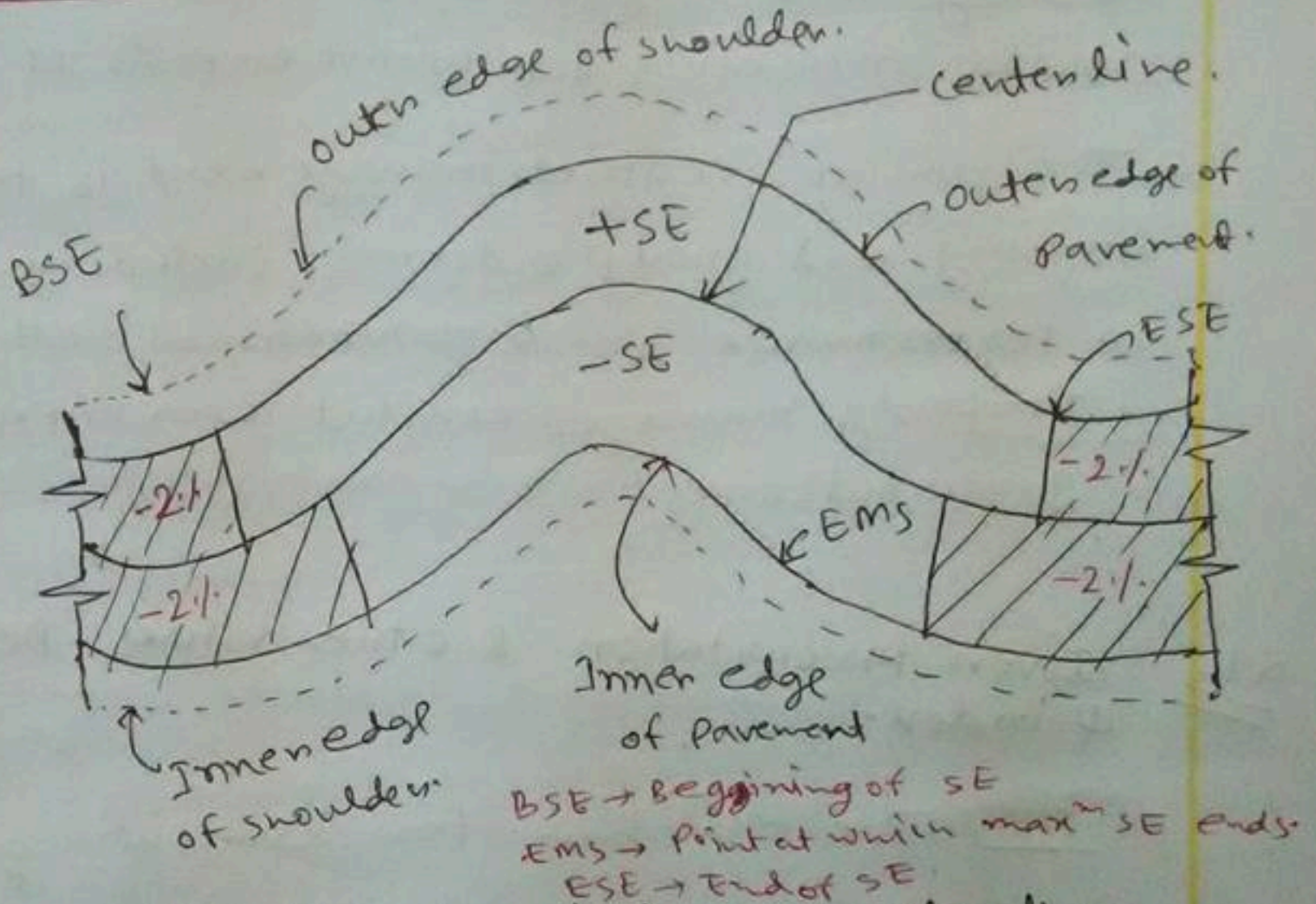
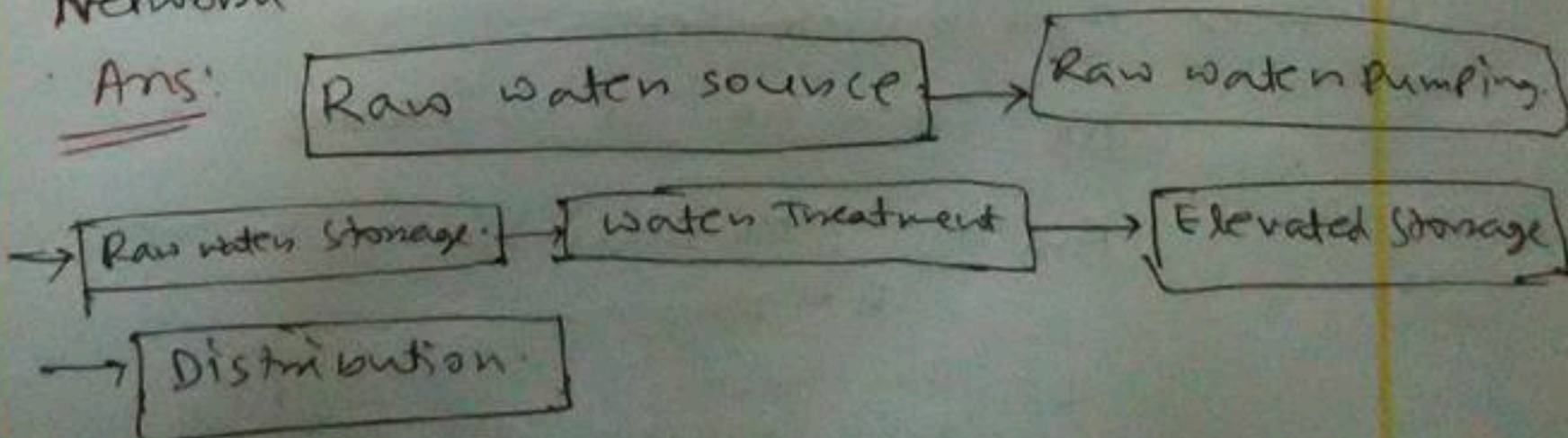


Fig: Plan of Typical Superelevation Transition.

Q.14

Sketch the flow diagram of water supply network.

Ans:



Q.15 Characteristics of Conflict Management System.

Ans: Conflict management is the process of limiting the negative aspects of conflict while increasing the positive aspects of conflict.

→ The aim of conflict management is to enhance learning and group outcomes, including effectiveness or performance in organizational setting.

→ Properly managed conflict can improve group outcomes.

Q.16 Given precipitation & other values - Determine  $\Phi$  index.

Ans: The equation for computing  $\Phi$ -Index:

$$P_d = \sum_{m=1}^m (R_m - \Phi \Delta t)$$

Hence,  $P_d$  = runoff depth.

$m$  = Total no. of ordinates.

$R_m$  = "m"th ordinate in the total rainfall hyetograph.

$\Phi$  =  $\Phi$ -index.

$\Delta t$  = time step for each  $a_i$ .

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~~Another equation,~~

$$\text{Here, } \bar{Q}_d = \frac{\sum Q_i \times \Delta t}{A}$$

$Q_i$  = direct runoff ordinate at  $i^{\text{th}}$  time step

$A$  = watershed Area.

Another Equation:

$$\phi\text{-index} = \frac{P-R}{t_e}$$

where,  $P$  = total storm precipitation (cm)

$R$  = total surface runoff (cm)

$t_e$  = elapsed time period (in hours)

$$\& \text{w-index} = \frac{P-R-I_a}{t_e}$$

Here,  $I_a$  = initial losses.

The w-index is more accurate than the  $\phi$ -index because it subtracts initial losses.

### Example of $\phi$ index

A 12-hour storm rainfall with the following depths in cm occurred over a basin: 2.0, 2.5, 7.6, 3.8, 10.6, 5.0, 7.0, 10.0, 6.4, 3.8, 1.4 and 1.4. The surface runoff resulting from the above storm is equivalent to 25.5 cm of depth over the basin. Determine the average infiltration index ( $\phi$ -index) for the basin.

Ans:

$$\begin{aligned}\text{Total rainfall in 12 hours} &= (2.0 + 2.5 + \dots + 1.4) \\ &= 61.5 \text{ cm.}\end{aligned}$$

$$\text{Total runoff in 12 hours} = 25.5 \text{ cm.} \quad (P-R)$$

$$\begin{aligned}\text{Infiltration in 12 hours} &= (61.5 - 25.5) \text{ cm} \\ &= 36 \text{ cm}\end{aligned}$$

$$\therefore \text{Avg. Infiltration} = \frac{36}{12} = 3.0 \text{ cm/hr.} \quad (P-R) \rightarrow te$$

$\therefore$  Avg. rate of infiltration during the central 8 hours.

$$8\phi + 2.0 + 2.5 + 1.4 + 1.4 = 36$$

$$\Rightarrow \phi = 3.6 \text{ cm/hr. (Ans)}$$

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Q.17

If,  $q_u = 50 \text{ kPa}$ ,  $N_c = 9$ ,  $\gamma = 18 \text{ kN/m}^3$ ,  $N_q = 2$ ,  $N_\gamma = 1$ ,  
 $B = 1 \text{ m}$ , Calculate ultimate bearing capacity.

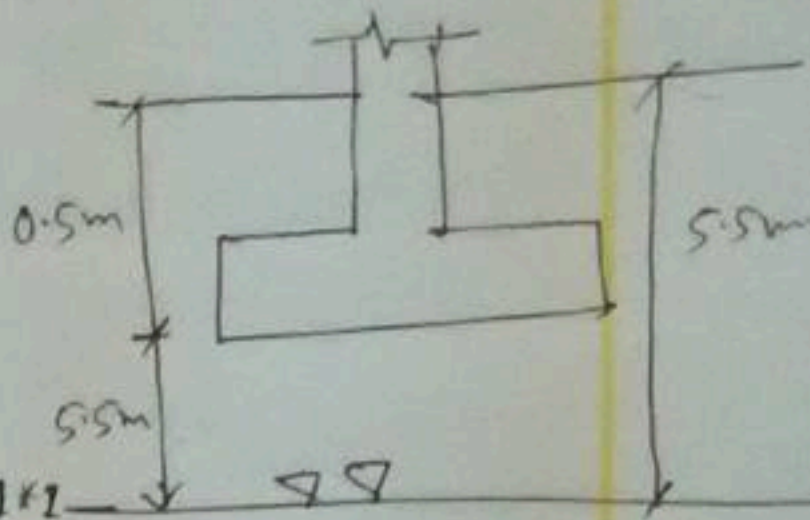
Solution:

we know,

$$q_u = cN_c + qN_q + \frac{1}{2}\gamma B N_\gamma$$

$$= 25 \times 9 + 6 \times 9 + \frac{1}{2} \times 18 \times 1 \times 1$$

=



Here,

$$q = \frac{q_u}{2} = \frac{50}{2} = 25 \text{ kPa}$$

$$N_c = 9$$

$$q = \gamma D_f = 18 \times 0.5 = 9$$

9 or 6

confused

Q.18

A problem about the calculation of field capacity. (can't memorize).

(11) Find the field capacity of soil for the given data.

Solution:

$$\text{water applied} = 600 \text{ m}^3.$$

$\therefore$  water retained

$$= (600 \times 0.90) \text{ m}^3 \times 9.81 \text{ kN/m}^3$$

$$= 5297.4 \text{ kN}.$$

Again, dry weight of irrigated

land = Area  $\times$  Depth  $\times \gamma_d$  of

$$\text{soil} = (800 \times 2) \times 1400$$

$$= 2240000 \text{ kg}$$

$$= \frac{2240000 \times 9.81}{1000} \text{ kN}$$

$$= 21974.4 \text{ kN}$$

Now,

21974.4 kN soil retains 5297.4 kN water

$$\therefore 100 \text{ kN " " } \frac{5297.4}{21974.4} \times 100$$

$$= 24.11 \% \text{ water}$$

We know,

Field capacity = (i) Existing water content + (ii) retained water content

$$= (7 + 24.11) \% = 31.11 \%$$

[Ans]

Given,

i) Depth of root zone = 2 m.

$$\gamma_w = 9.81 \text{ kN/m}^3.$$

ii) Existing water content = 7%

iii)  $\gamma_d$  of soil = 1400 kg/m<sup>3</sup>

iv) Total water volume

$$= 600 \text{ m}^3.$$

v) water lost = 10%

$$\therefore \text{water retained} = (100 - 10) \% = 90 \%$$

vi) Area of irrigated land = 800 m<sup>2</sup>.

Field Capacity = ?