

Surveying:

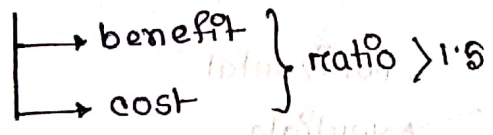
Object of surveying by aim of surveying:

To prepare a map to show the thesus of the area such as town, village, agricultural land, ponds, canal, hãll, river, highway, railway, tree, building

Importance of surveying:

It is vital importance in any engineering projects because →

- ① Without surveying not possible to select alignment of highway, railway, canal.
- ② Measurement of land and the fixation of bounderies.
- ③ Engineering feasibility impossible without surveying.



Classification of surveying:

① Primary classification:

- A. Plane : Arc consider कक्षा स्थान।
- B. Geodetic : " " " स्थान।

② Secondary classification:

A) Based on field:

- ① Land survey
- ② Marine survey
- ③ Astronomical survey.

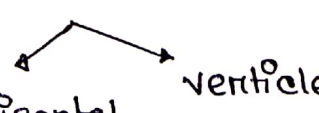
B) Based on instrument used:

- ① Plane table survey
- ② Compass survey
- ③ Theodolite survey
- ④ Chain survey.

Types of measurement:

① Linear measurement 

② Angular measurement


horizontal verticle

Unit of measurement:

- ① British Unit (inch, ft, yards, mile)
- ② Metric Unit (mm, cm, m, km...)

Methods of linear measurement:

- ① By pacing or stepping
- ② By speedometer
- ③ By chaining.

Types of chain:

Sl. No.	Types of chain	length of chain	No. of links	length of one link (m)
1.	Engineer's chain	100 ft.	100	1
2.	Gunter's chain	66 ft.	100	0.6
3.	Metric chain	20 m 30 m	100 150	0.2 0.2

$$\left. \begin{array}{l} 1 \text{ inch} = 2.5 \text{ cm} \\ 1 \text{ ft} = 30.48 \text{ cm} \end{array} \right\}$$

Types of tape:

- ① cloth or linen
- ② Metallic tape
- ③ Steel tape

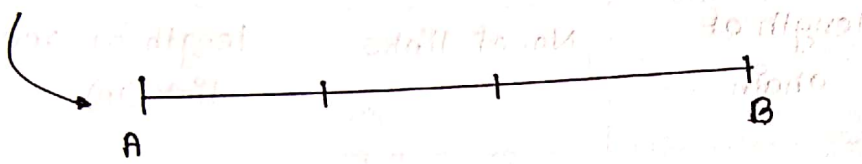
2.5 feet 80 cm
↳ one step of walking

Chaining: Chaining is a process measuring the linear distance between two stations with the help of chain.

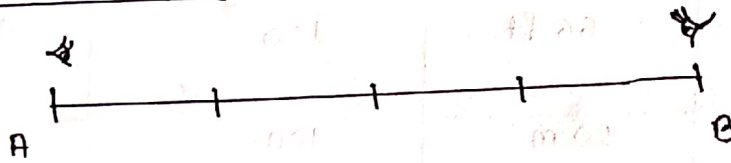
① Ranging

② Chaining

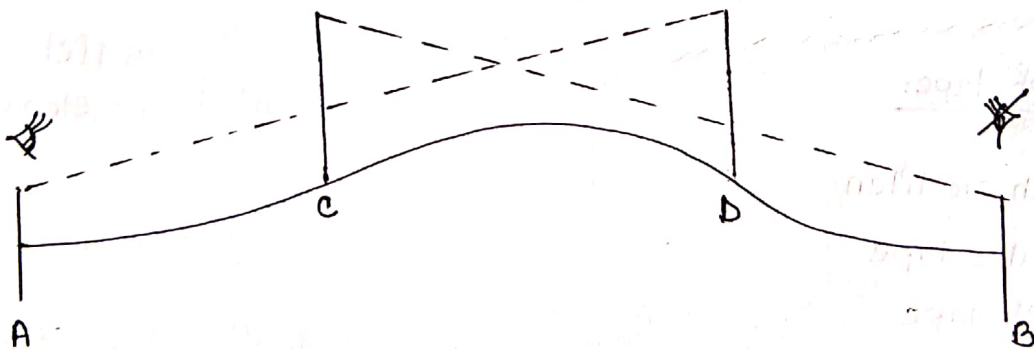
Ranging: Ranging is a process to make a line straight.



① Direct Ranging:



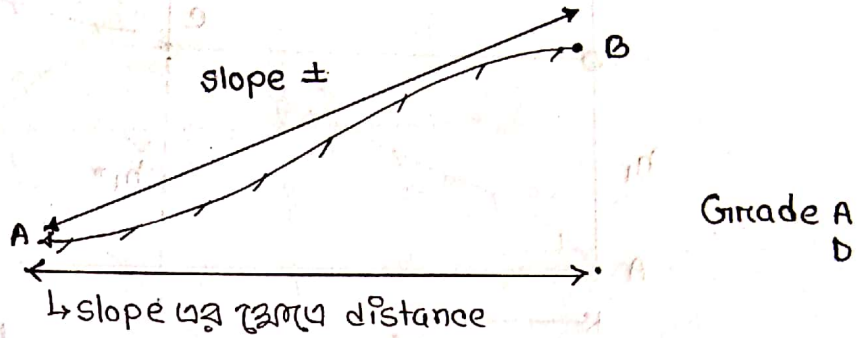
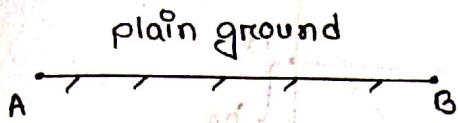
② Indirect Ranging:



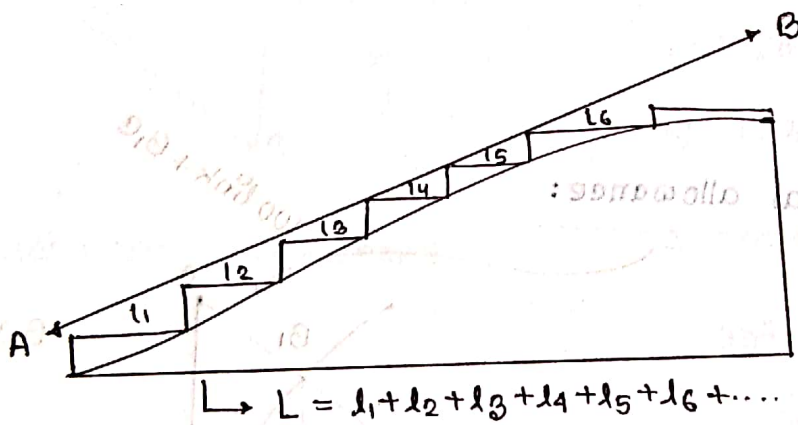
যদি A, C ও D কে same situation (একই line)

এবং B " " " " " এ ক্ষেত্রে তাই তা Indirect Ranging

Method of chaining on slopping ground:

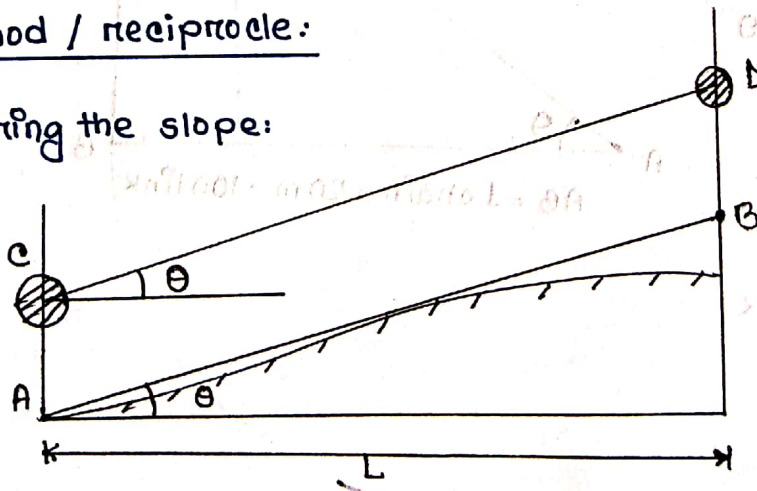


① Direct / Straight Methode:



② Indirect Method / reciprocal:

A) By measuring the slope:

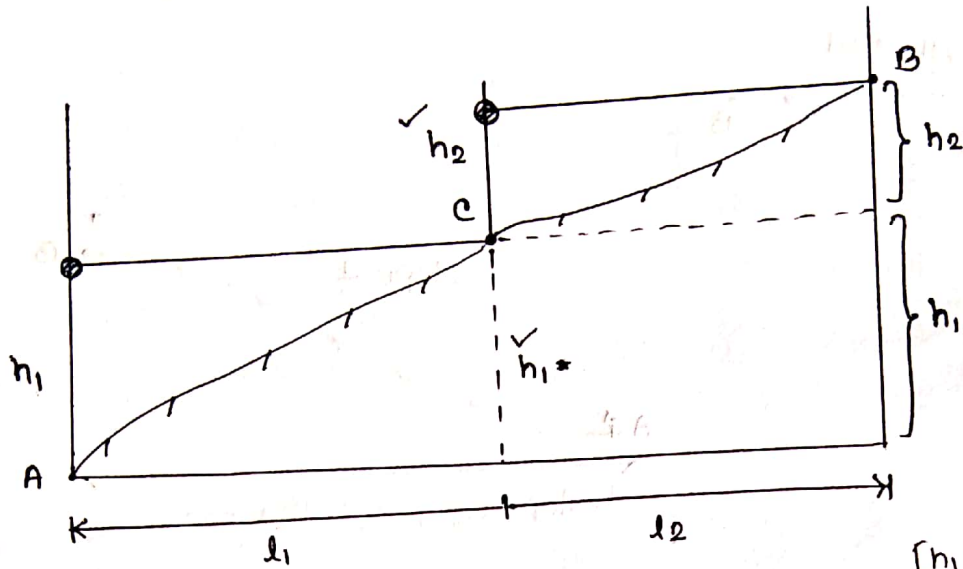


$$AC = BD$$

$$\frac{L}{AB} = \cos \theta$$

$$\Rightarrow L = AB \cos \theta$$

b) By knowing the difference of level:



[h_1 measured by telescope]

$$L = l_1 + l_2$$

$$L = \sqrt{AC^2 - h_1^2}$$

c) Applying Hypotenusal allowance:

$$AB = AB_1$$

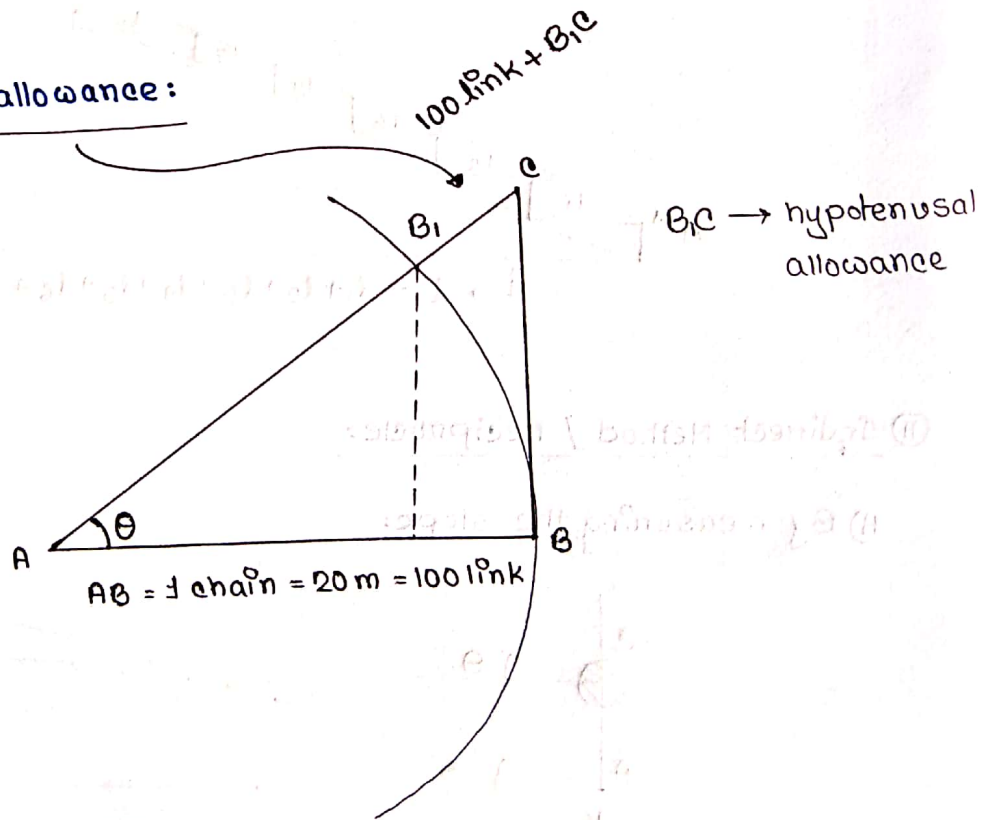
$$\Delta ABC \Rightarrow \frac{AC}{AB} = \sec \theta$$

$$\Rightarrow AC = 100 \sec \theta$$

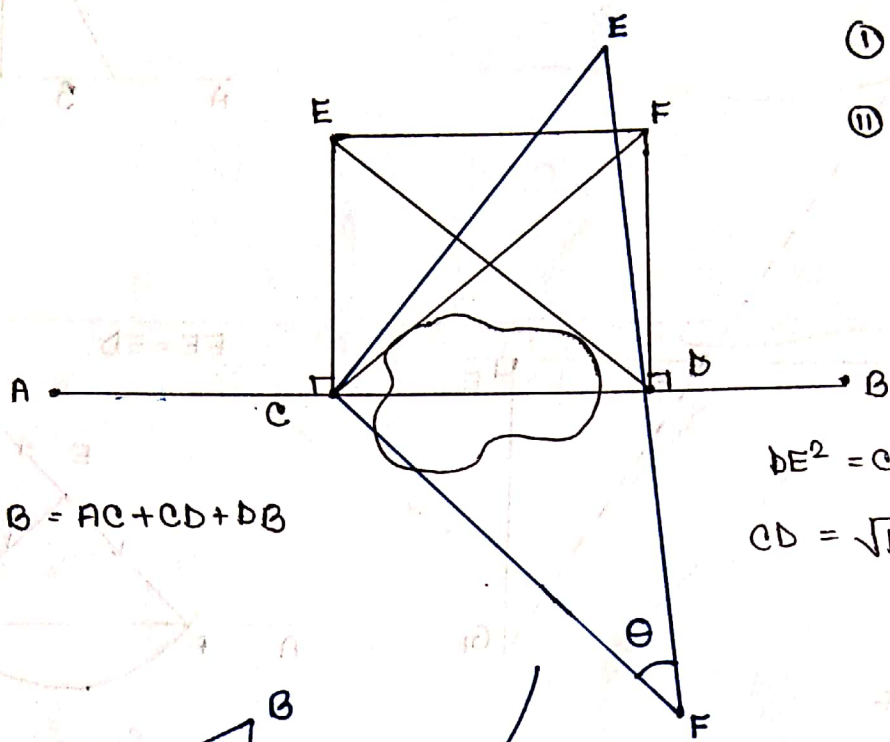
$$B_1C = AC - AB_1$$

$$= 100 \sec \theta - 100$$

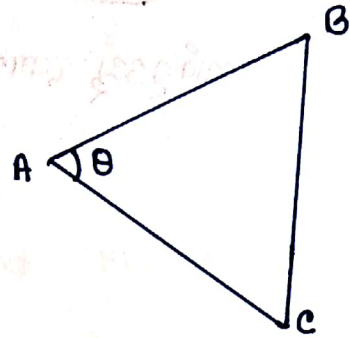
$$= 100 (\sec \theta - 1) \text{ link}$$



Obstacle in chaining:



$$AB = AC + CD + DB$$



$$BC^2 = AC^2 + AB^2 - 2 AC \cdot AB \cos \theta$$

$$\textcircled{I} \quad CD^2 = DE^2 - EC^2$$

$$\textcircled{II} \quad CD = EF$$

$$DE^2 = CE^2 + CD^2$$

$$CD = \sqrt{DE^2 - CE^2}$$

$$CE^2 = CF^2 + EF^2 - 2 CF \cdot EF \cos \theta$$

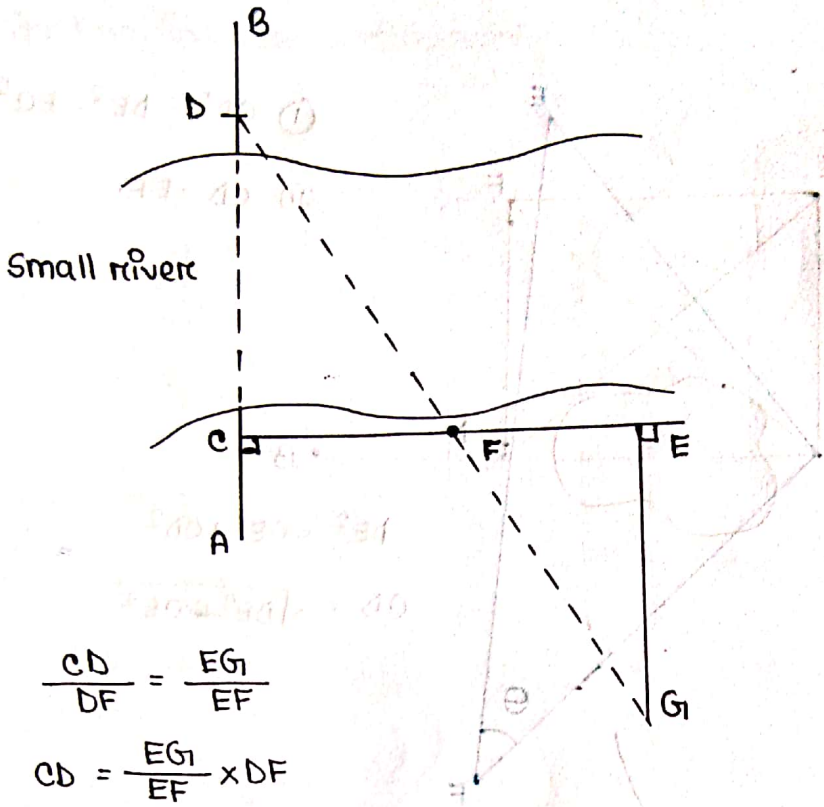
$$\Rightarrow \cos \theta = K$$

$\Delta CFD \Rightarrow$

$$CD^2 = CF^2 + DF^2 - 2 CF \cdot DF \cos \theta$$

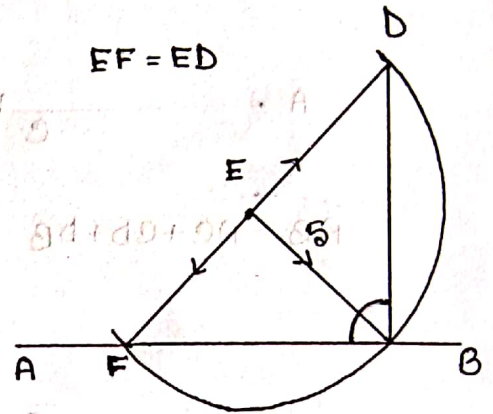
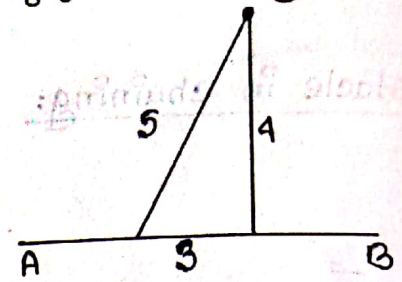
River এর width খনি কল্প হওয়া →

#



$$\frac{CD}{DF} = \frac{EG}{EF}$$

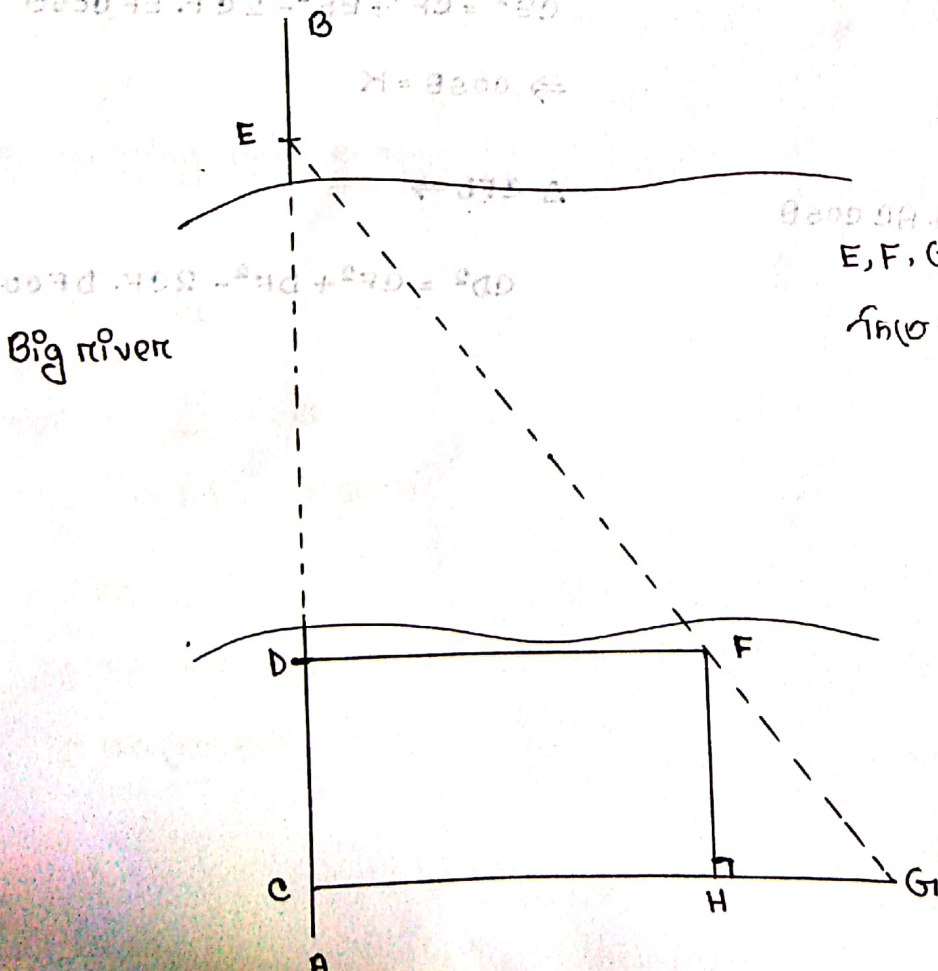
$$CD = \frac{EG}{EF} \times DF$$



অর্ধবৃত্তের কেন্দ্র

River এর width খনি বোঝা হওয়া →

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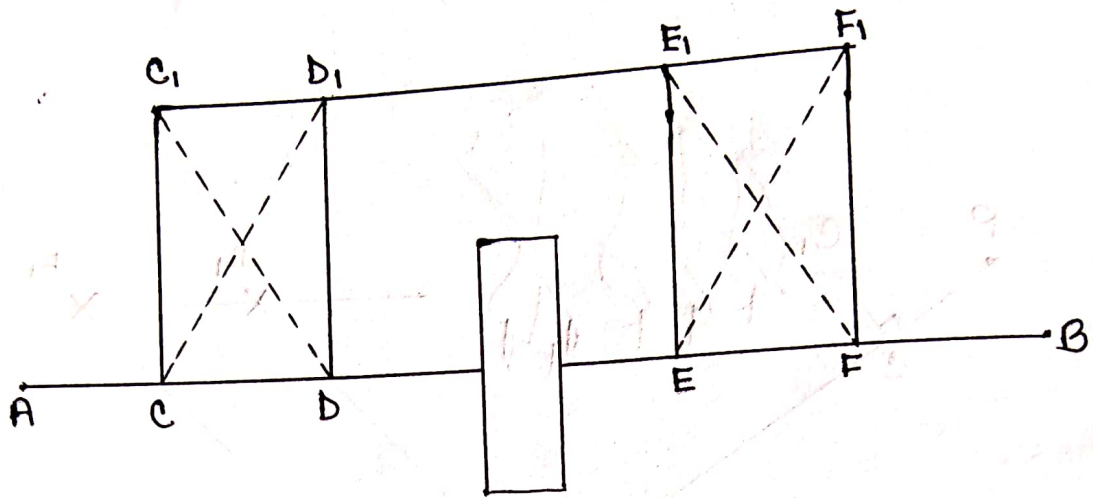
E, F, G - এ একসমতলে Ranging Rods

নির্ভর হলে এমন একই line-এ থাকবে

$$\frac{DE}{DF} = \frac{FH}{GH}$$

$$DE = \frac{FH}{GH} \times DF$$

Obstacle in chaining and Ranging:



chaining ভুল

personal ভুল

Errors and mistakes in chaining:

1. Compensating error

2. Cumulative error → chain ৭ link ৭য় কল্পবেষ্টি হওয়া

a) Positive (when chain is too short)

b) Negative (when chain is too long)

3. Mistake.

$L = 30 \text{ m}$ (Length of steel tape)

Weight, $w = 0.693 \text{ kg}$

Area, $A = 0.03 \text{ cm}^2$

Standard Temperature, $T_s = 20^\circ\text{C}$

" Pull (टोल), $P_s = 10 \text{ kg}$

$E = 2.1 \times 10^6 \text{ kg/cm}^2$

$\alpha = 11 \times 10^{-6} \text{ per } ^\circ\text{C}$

⇒ Field → Measured Temperature, $T_m = 32^\circ\text{C}$

" Pull, $P_m = 15 \text{ kg}$

$$C_t = \alpha (T_m - T_s) L \rightarrow \text{correction for } T = +0.00396 \text{ m}$$

$$C_p = \frac{(P_m - P_s) L}{AE} \rightarrow \text{correction for } P = +0.00238 \text{ m}$$

$$C_s = \frac{w^2 L}{24 n^2 P_m^2} \rightarrow \text{correction for Sag} = -0.00267 \text{ m}$$

always (-ve)

Total error, $l = C_t + C_p + C_s$

$$= 0.00396 + 0.00238 - 0.00267$$

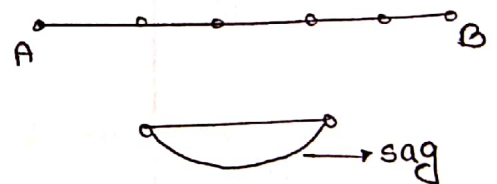
$$= 0.00367 \text{ m (+ve)}$$

∴

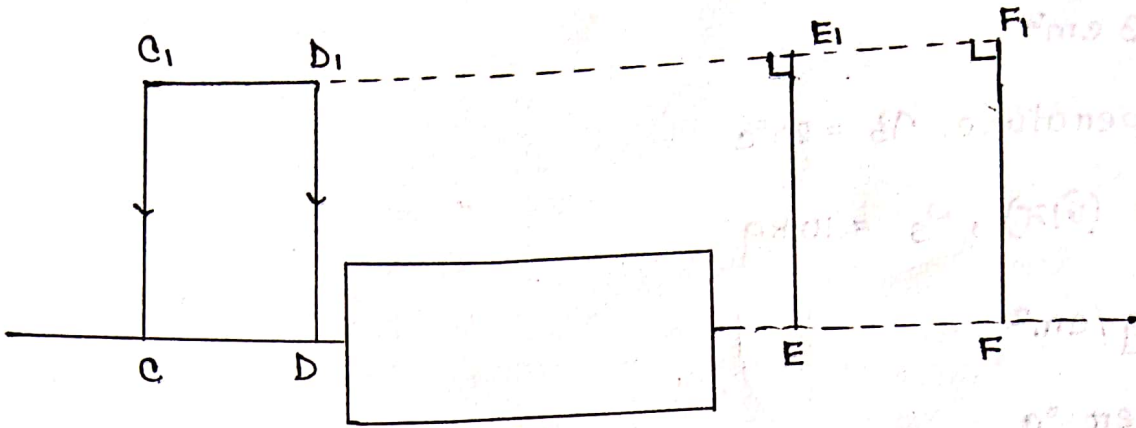
measured Length (ML) = 780 m

$$\begin{aligned} \text{Actual Length (Act. L)} &= \frac{L'}{L} \times \text{ML} \\ &= \left(\frac{L \pm e}{L} \right) \times \text{ML} \end{aligned}$$

$$= 780.095 \text{ m}$$

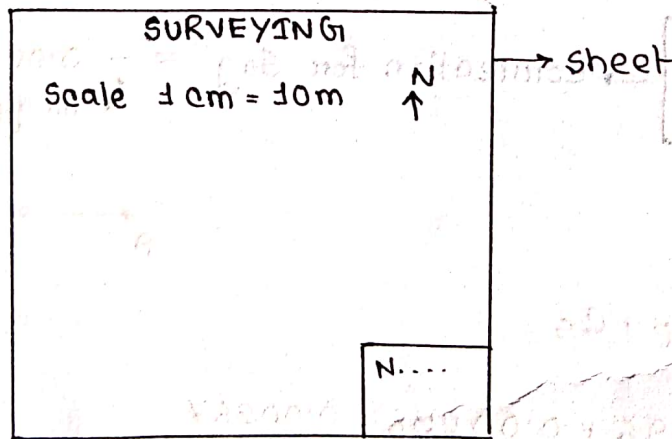


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Free survey:

- ① Field work
- ② office work
- ③ Adjustment of the instrument:

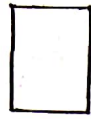


scale is a device used to measure the size of any object:

Scale is the fixed ratio, by which the actual length of the object is reduced or increased.

Type:

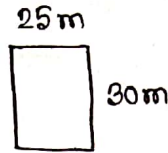
① Full size scale:



1cm = 1cm [Map ও object এর মাপ same]

② Reduced scale:

1cm = 5m



③ Increased or enlarged scale:



1cm = 0.01mm

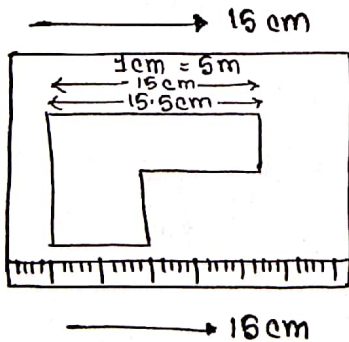
④ Engineering scale:

1cm = 5m or, 1cm = 10m

Representative fraction, $RF = \frac{1cm}{5m} = \frac{1cm}{5 \times 100cm} = \frac{1}{500}$ [map এর 1 unit field এর 500 unit]

⑤ Graphical scale:

map 5cm → 2.5m field



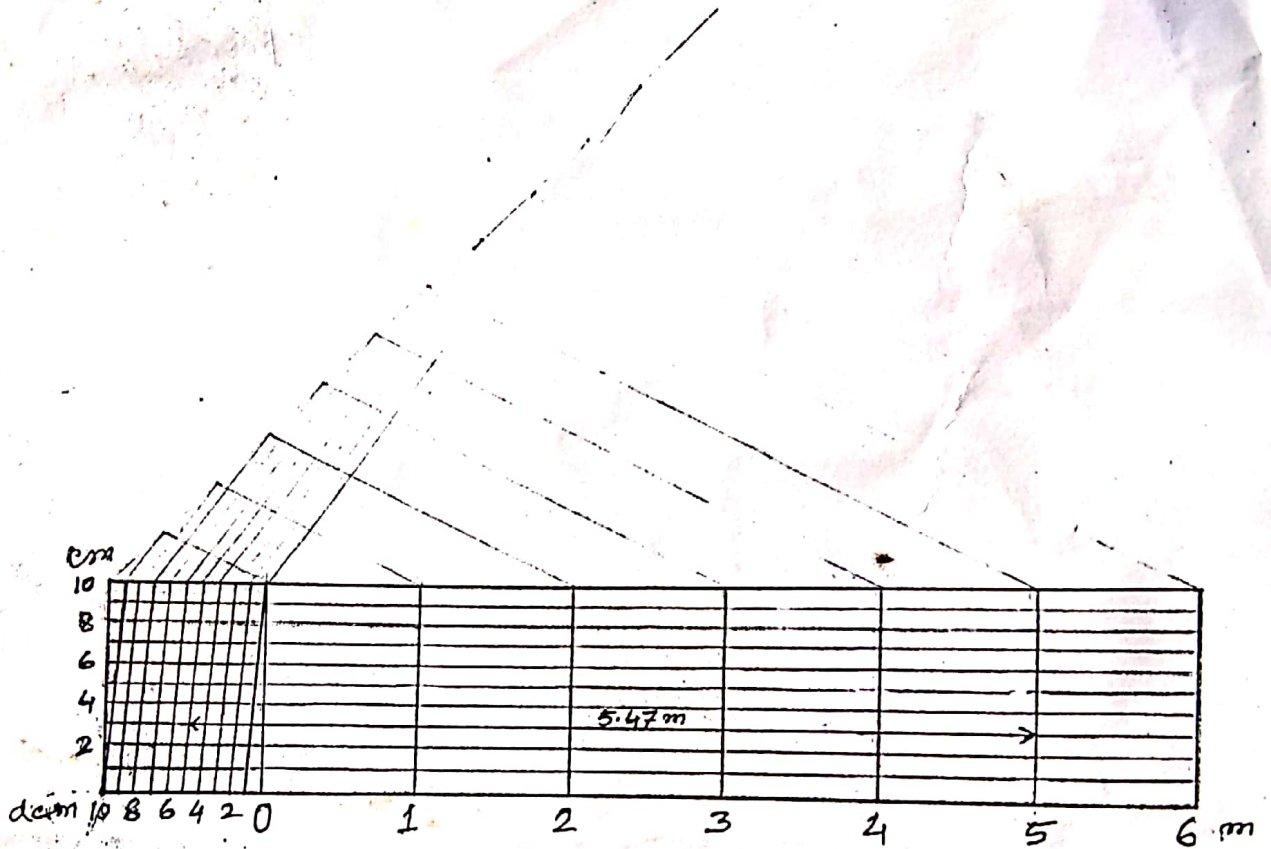
এই scale এর large এর ratio বসলেই অসম
 থাকে। 10 বছর পর বসলেই মতটুকু enlarge
 হবে, scale ও হবে, মাপ হবে অসম।

⑥ Plain scale: সাজাপাতি 2টা unit মাপে থাকে,

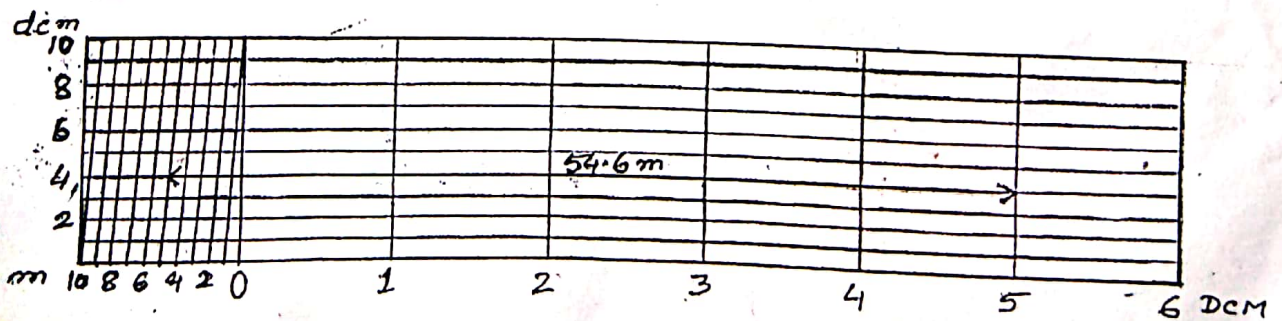
(m, inch; dm, cm; cm, mm) → 5.2

Diagonal scale: (m, dm, cm.) \rightarrow 5.74 or (dm, cm, mm)

vernier scale:



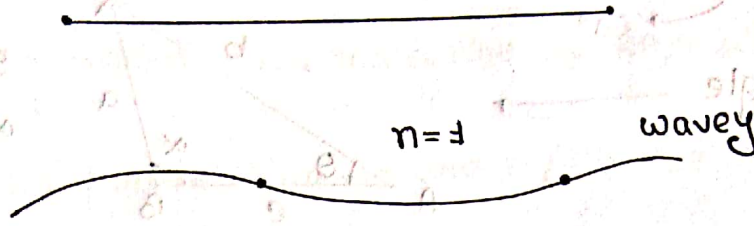
DIAGONAL SCALE



#

০৯ নাই
n এর স্থান নাই

plain



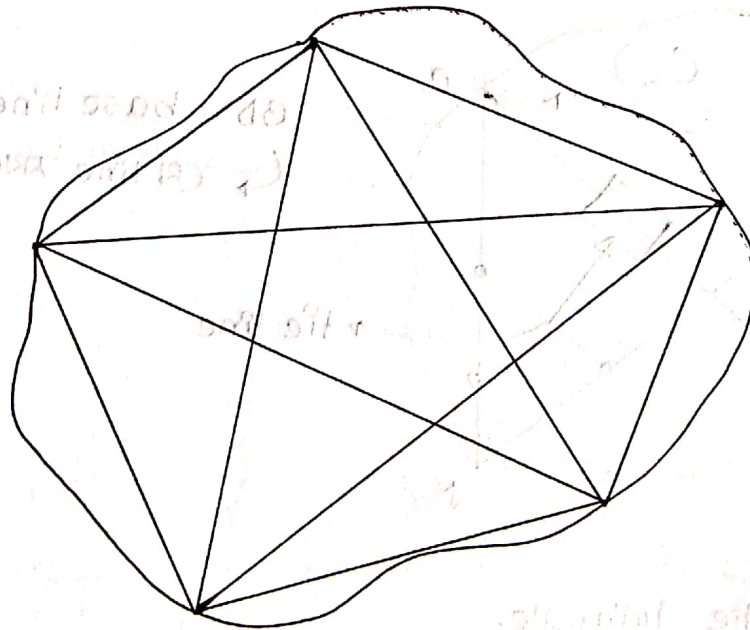
undulation

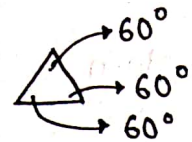
n=2

Chain surveying: (linear measurement)

Basic principle:

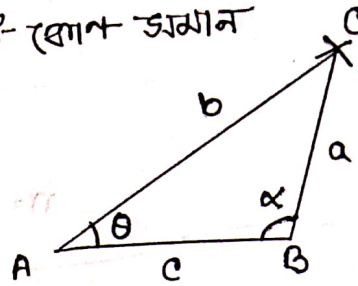
Triangulation (area covered by three sides)



① Ideal triangle \rightarrow 

৩টি কোণ সমান

② Well condition triangle \rightarrow



$\theta =$ সূত্রকোণ
 $\alpha =$ সূত্রকোণ

③ " " "

$\theta \rightarrow 30^\circ$ এর বরা
 $\alpha \rightarrow 120^\circ$ এর বেশি

$$\theta \geq 30^\circ$$

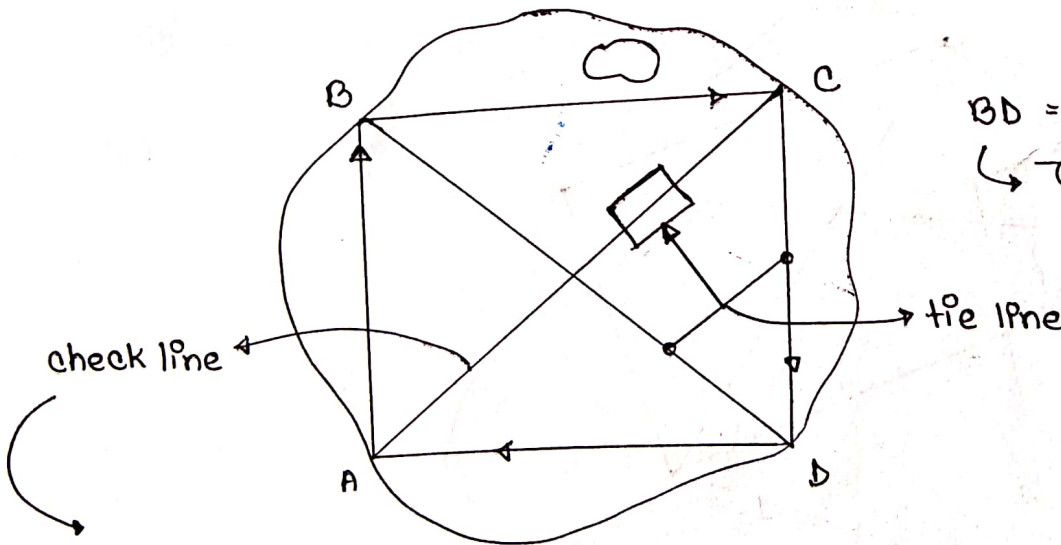
$$\alpha \leq 120^\circ$$

$$c^2 = a^2 + b^2 + 2ab \cos \theta$$

Reconnaissance: Primary survey

Reconnaissance survey (primary survey):

① Index sketch (key plan):



BD = base line

\hookrightarrow tie line সবচেয়ে বড়

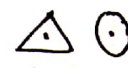
check line

tie line

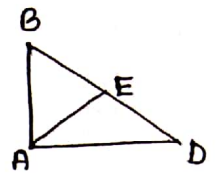
to check the accuracy of the triangle.

{ tie line is also a sag line
check line is also a tie line.

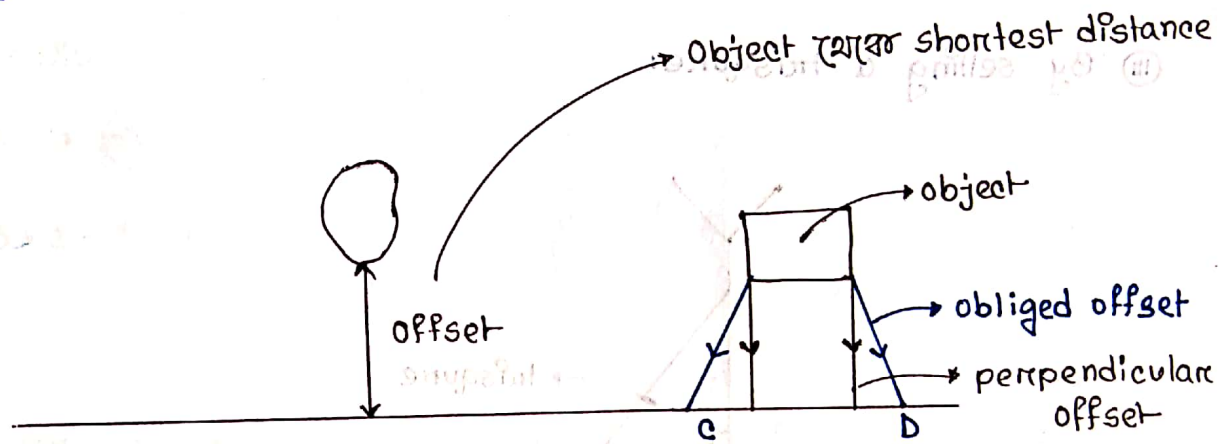
② Station is the points at the beginning and at the end of a chain.

a) main station → main line এর সাথে যুক্ত 

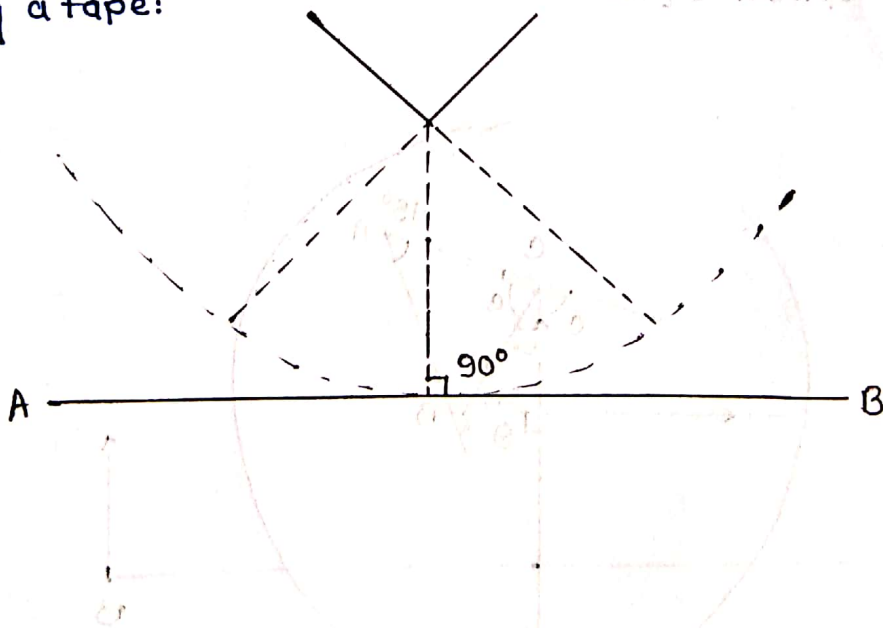
b) Subsidiary station → check / tie line এর সাথে যুক্ত



Object, offset:



① By swinging a tape:



$$\angle A + \angle B + \angle C = 180^\circ$$

$$\theta = 2\alpha + 2\beta$$

$$\Rightarrow \alpha + \beta = \frac{\theta}{2}$$

$$2\alpha + 2\alpha = 180^\circ$$

$$2\beta + 2\beta = 180^\circ$$

$$2\alpha + 2\alpha + 2\beta + 2\beta = 360^\circ$$

$$\Rightarrow \alpha + \beta + \alpha + \beta = 180^\circ$$

$$\Rightarrow \alpha + \beta = 180^\circ - (\alpha + \beta)$$

$$\Rightarrow \frac{\theta}{2} = 180^\circ - \alpha - \beta$$

$$\Rightarrow \frac{\theta}{2} = \alpha$$

For, $\theta = 90^\circ$, $\angle A$ must be 45° .

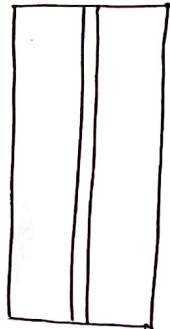
limiting length of offset:

not more than the length of tape or 15 m.

Equipment of chain surveying:

- ① Engineering chain (1), tape (1), ranging rod (at least 3), arrows (5), optical square (1), compass (Northline চিহ্ন করা -1).

fieldbook



, paper, pencil, eraser

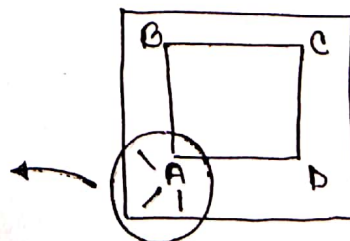
Procedure of field work:

- ① Reconnaissance
- ② Index sketch.
- ③ Marking the stations on the ground.
- ④ Reference sketches
- ⑤ Measurements noting in the fieldbook.

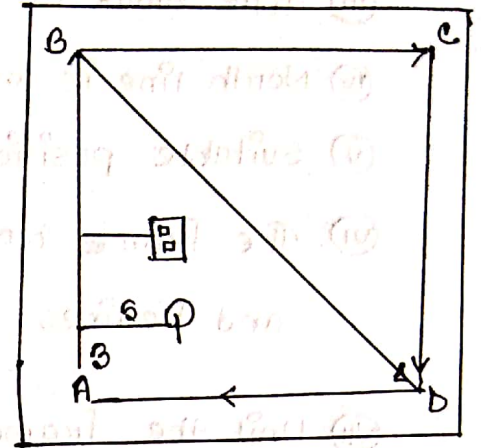
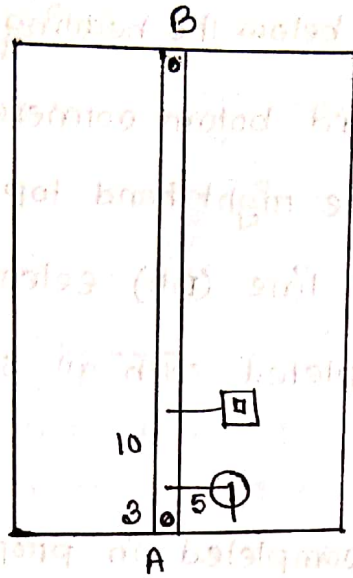
selection of survey station

- a) station should be intervisible.
- b) stations should form well condition triangle.
- c) Surveyline should be on level ground.
- d) chaining free from obstacle

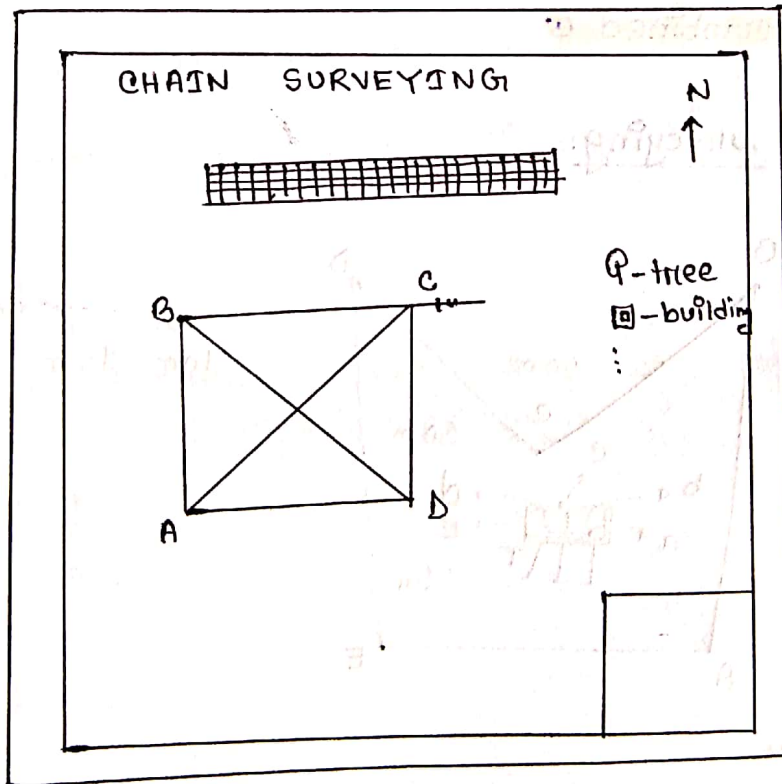
④ →



⑥ Field book recording:

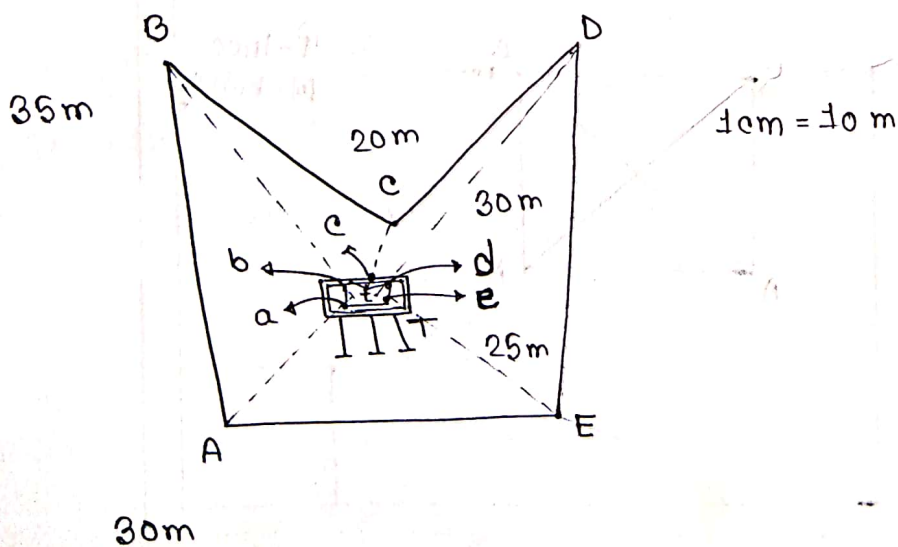


Procedure of plotting:



- ① A suitable scale
- ② Scale of the map is drawn below the heading.
- ③ Title block on the right hand bottom corner.
- ④ North line is marked on the right hand top corner.
- ⑤ Suitable position for base line (BD) selected on sheet.
- ⑥ The frame work is completed with all survey lines, check line and tie lines.
- ⑦ Unit the frame work is completed in proper form, the offsets should not be plotted.
- ⑧ Conventional symbols used in the map should be shown on the right hand side.


Plane table surveying:



Principal: Parallelism (vertical)

plane tabling is a graphical method of surveying. Field work and plotting are done simultaneously without the use of a fieldbook.

equipment: ① plane table


② alidade 

↳ a) plain alidade

b) telescopic "

③ spirit level

④ compass

⑤ U-fork  / plumbing fork ↓

Orientation: Orientation is the process of putting the plane table into some fixed direction so that the line representing a certain direction of the plan is parallel to that direction on the ground.

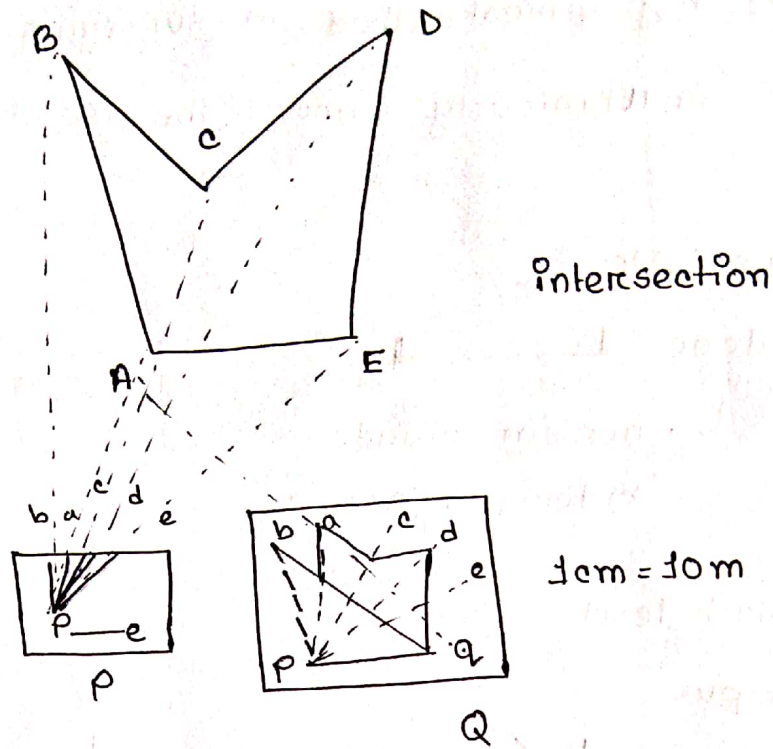
Method of Plane Tableing:

① Radiation → This method is more suitable when the distances are small (within a tape length).

② Intersection →

③ Traversing

④ Re-section



Procedure:

- ① fixing the table on the tripod stand.
- ② Leveling the table.
- ③ Centering the table.
- ④ Marking the dot line.
- ⑤ Orientation.

Errors:

- ① Instrumental
- ② Personal
- ③ Plotting

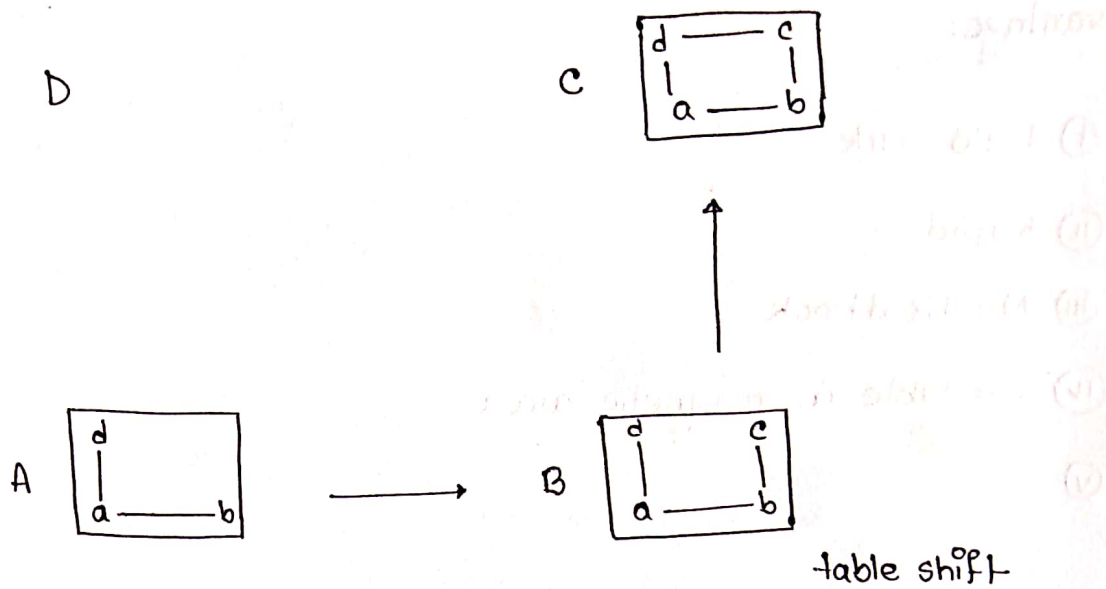
Advantage:

- ① Field work
- ② Rapid
- ③ No field book
- ④ Suitable in magnetic area.
- ⑤

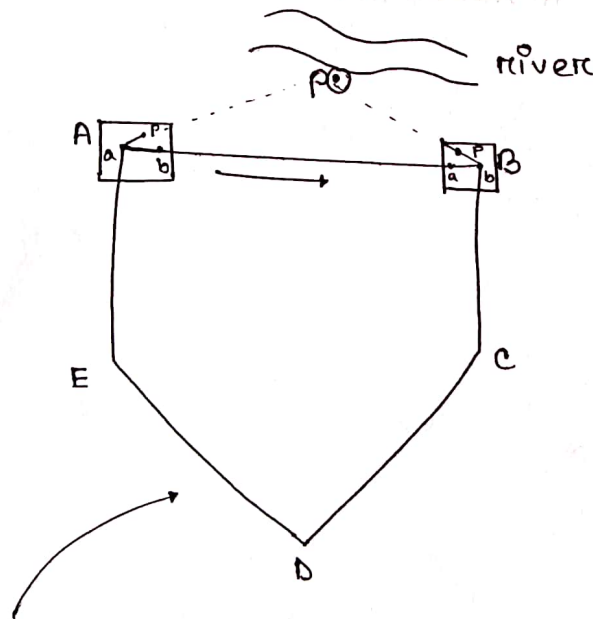
Disadvantage:

- ① Not suitable in wet magnet climate.
- ② " " for accurate work.
- ③ Difficult to carry the instrument.

#

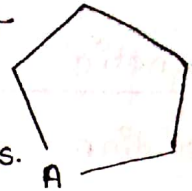


Resection: This method is suitable for establishing new stations at a place in order to locate missing details

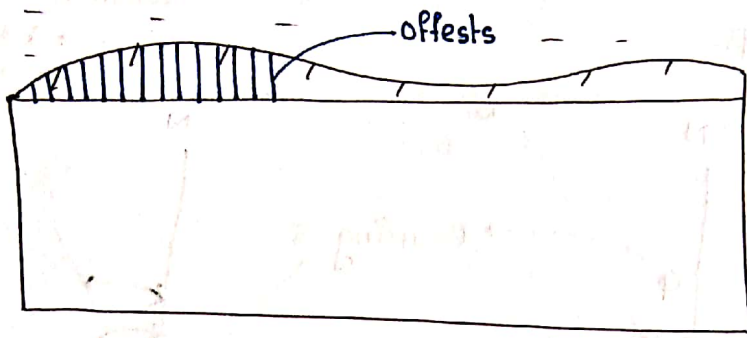


- a) Two-point problem.
- b) Three-point problem:

* # Traverse survey: A traverse is a frame work consist of a number of connected lines. Lengths are measured by chain or tape and the directions identified by compass.



① close traverse

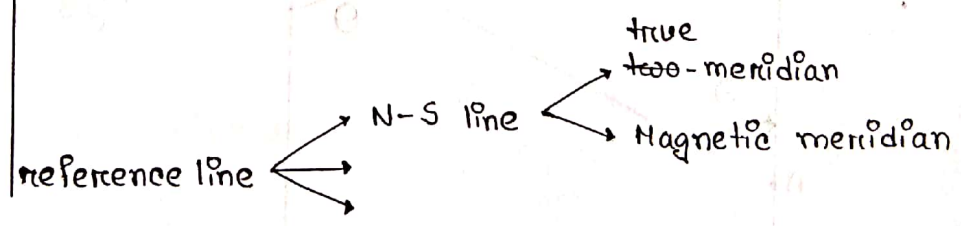
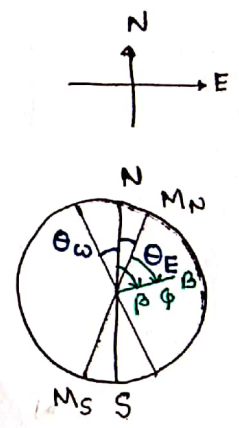
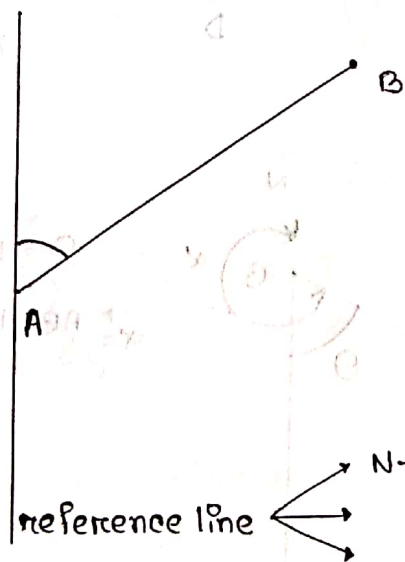


② open traverse

last end, first-end
এর মাঝে খিনবে না।

a) compass traverse:

b) Theodolite traverse:



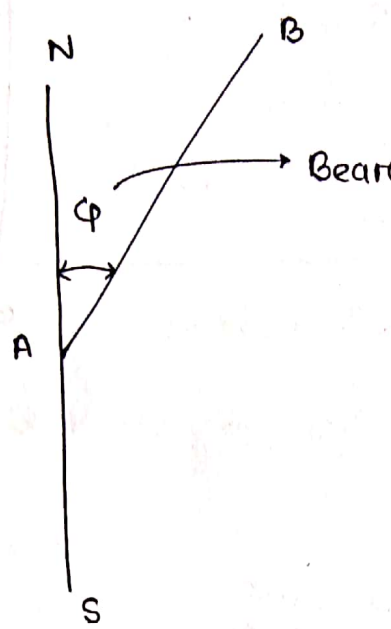
true meridian is the line passing through North pole and south pole and any point of the surface of the world.

Magnetic meridian is the line indicated by a 3D suspended and balanced magnetic needle.

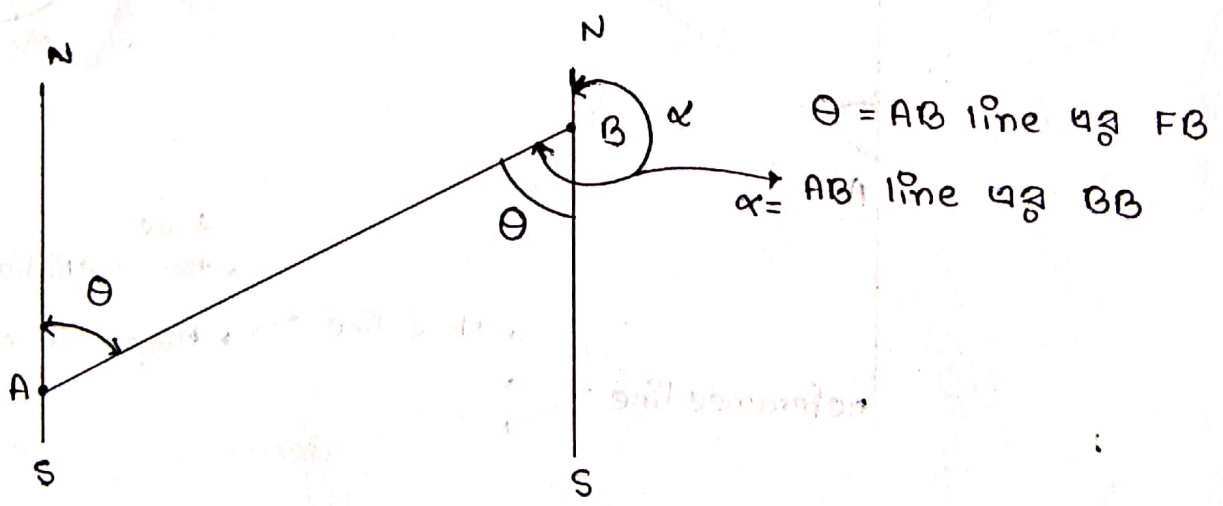
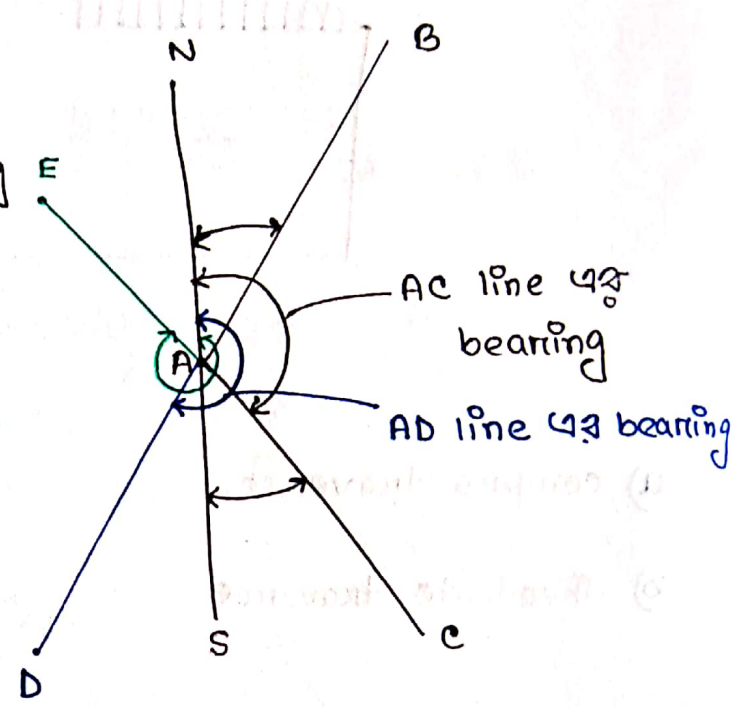
Magnetic declination: Angle between true meridian and magnetic meridian.

Magnetic Bearing / Bearing: ϕ

True Bearing: β



North pole থেকে clockwise
→ bearing



forward bearing → FB
backward bearing → BB

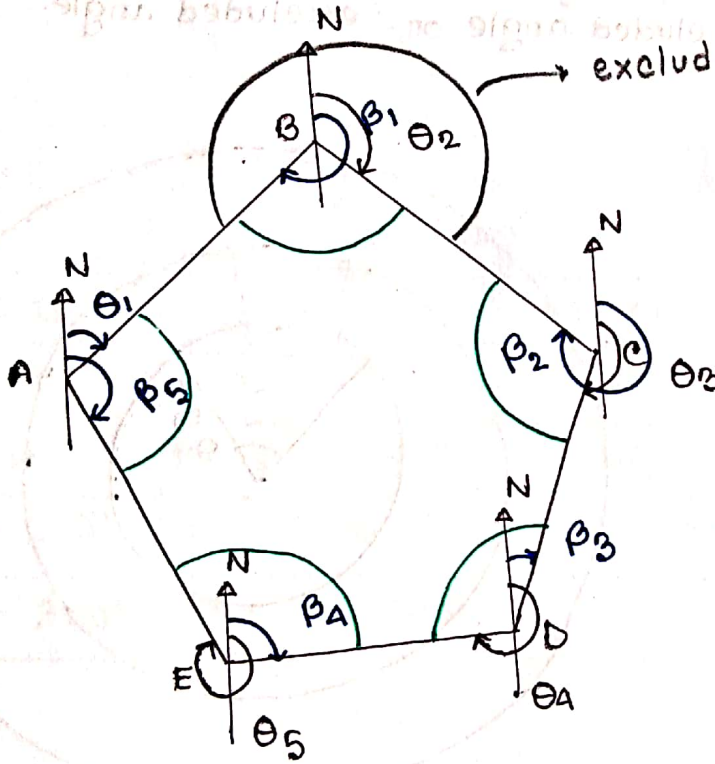
* Difference between FB and CB of any line is 180° .

$$\alpha - \theta = 180^\circ$$

* Traverse:

Two types of traverse are

- open traverse
- closed "



$\angle A + \angle B + \angle C + \angle D + \angle E \rightarrow$ included angle

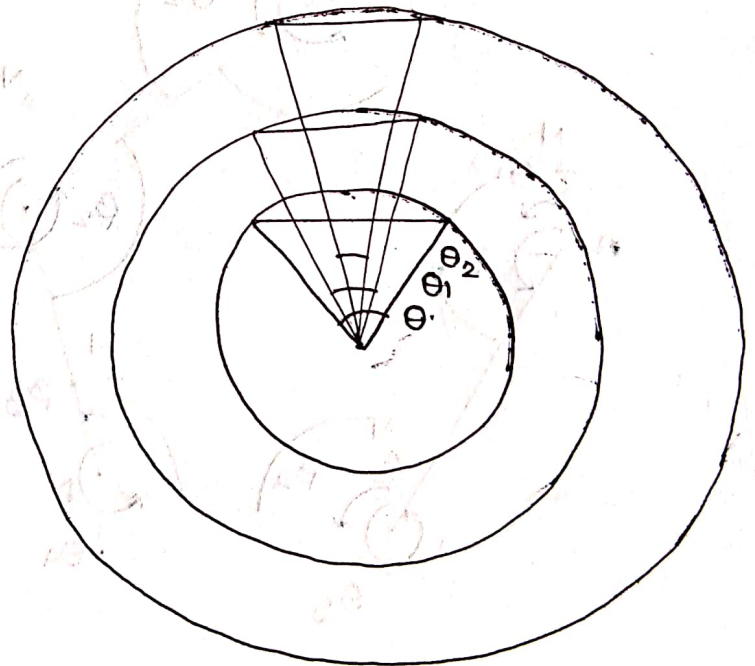
Characteristics of closed Traverse:

- ① Sum of included angle = $(2N-4) 90^\circ$ [N = No. of side]
- ② Sum of excluded angle = $(2N+4) 90^\circ$
- ③ F.B and B.B of a line must differ by 180° .
↳ $F.B - B.B = \pm 180^\circ$
- ④ Difference between two bearings of two lines at a point is —
∠included angle or, ∠excluded angle.

Designation of bearing:

$$R = \frac{1720}{b}$$

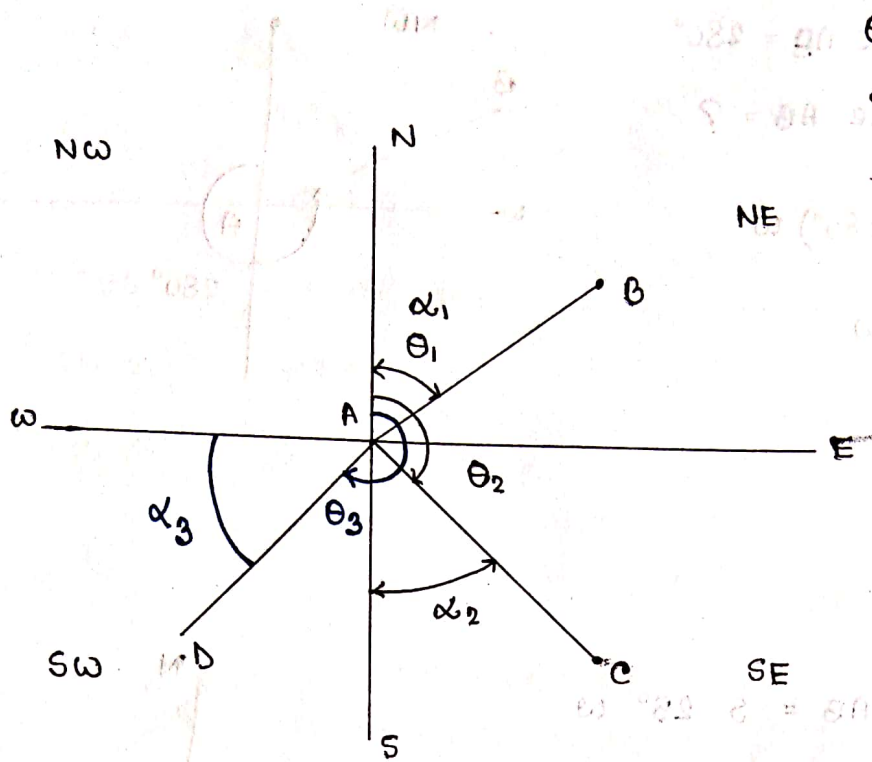
$$b.o.c =$$



① Whole circle bearing (WCB)

② Quadrantal bearing (QB/RB)

↳ Reduced



$$\theta_2 = W.C.B$$

$$\alpha_2 = Q.B$$

$$\begin{cases} W.C.B = 270^\circ \\ R.B = N.270^\circ E \end{cases}$$

W.C.B	R.B	Quadrant
$0^\circ - 90^\circ$	$R.B = W.C.B$	NE
$90^\circ - 180^\circ$	$R.B = 180^\circ - W.C.B$	SE
$180^\circ - 270^\circ$	$R.B = W.C.B - 180^\circ$	SW
$270^\circ - 360^\circ$	$R.B = 360^\circ - W.C.B$	NW

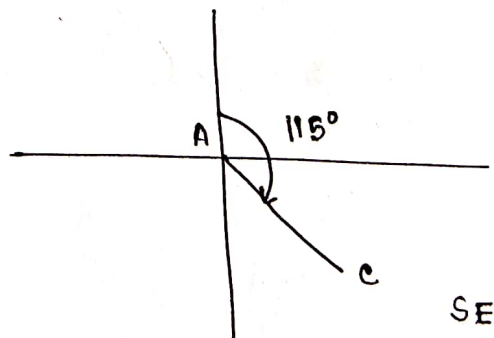
W.C.B of line AC = 115° .

R.B of line AC = ?

$$\rightarrow S (180 - W.C.B) E$$

$$\Rightarrow S (180 - 115) E$$

$$\Rightarrow S 65^\circ E$$

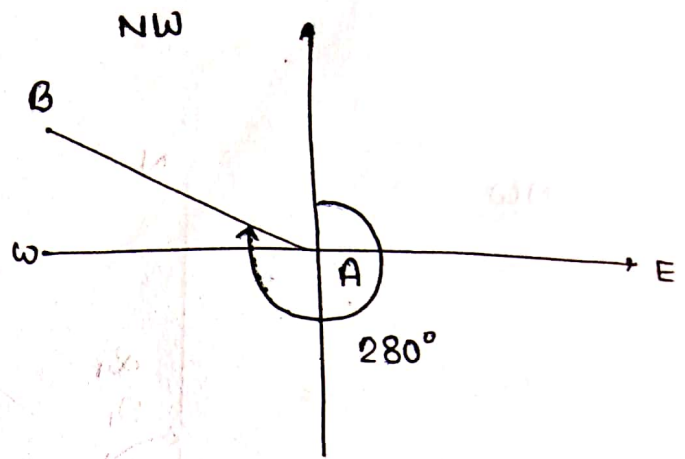


WCB of line AB = 280°

R.B of line AB = ?

$$\rightarrow N (360^\circ - 280^\circ) W$$

$$\Rightarrow N 80^\circ W$$



RB of line AB = S 25° W

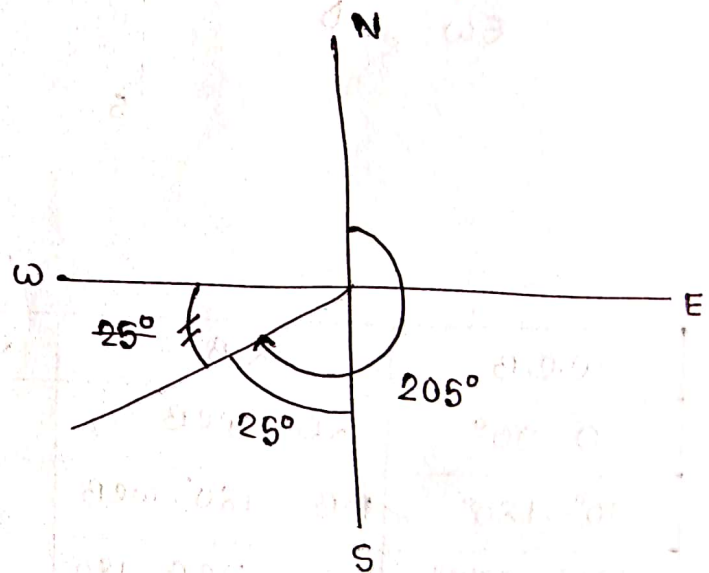
WCB = ?

$$\rightarrow 270^\circ - 25^\circ$$

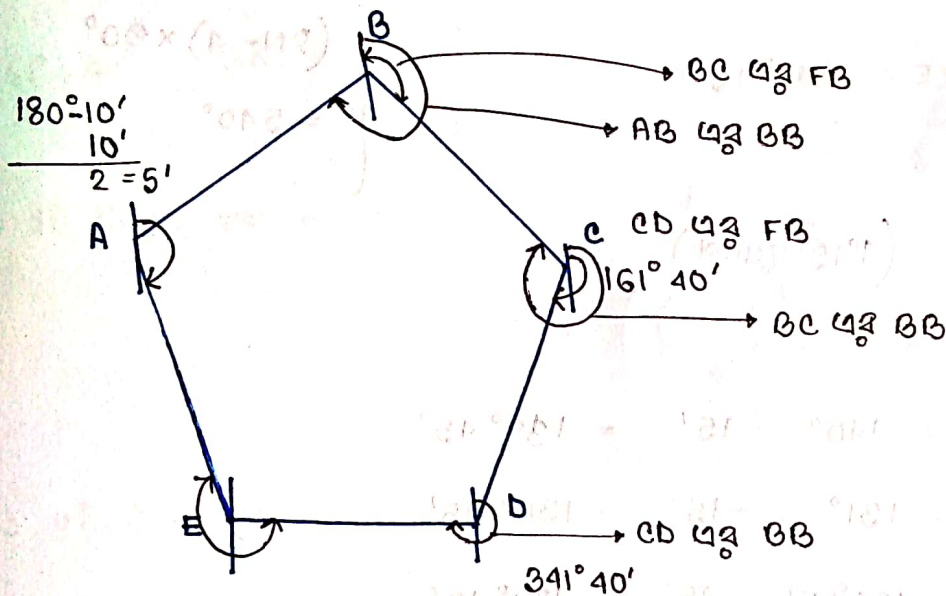
$$\Rightarrow 245^\circ$$

$$WCB - 180^\circ = 25^\circ$$

$$\Rightarrow WCB = 180^\circ + 25^\circ \\ = 205^\circ$$



#	Line	FB	BB
	AB	$71^{\circ}05'$	$250^{\circ}20'$
	BC	$110^{\circ}20'$	$292^{\circ}35'$
	CD	$161^{\circ}35' + 5'$	$341^{\circ}45' - 5'$
	DE	$220^{\circ}50'$	$40^{\circ}05'$
	EA	$300^{\circ}50'$	$221^{\circ}10'$



$$\angle B = \begin{array}{r} 250^{\circ}20' \\ - 110^{\circ}20' \\ \hline 140^{\circ} \end{array}$$

$$\angle C = \begin{array}{r} 292^{\circ}35' \\ - 161^{\circ}35' \\ \hline 131^{\circ} \end{array}$$

$$\angle D = \begin{array}{r} 341^{\circ}45' \\ 220^{\circ}50' \\ \hline 120^{\circ}55' \end{array}$$

$$\text{excluded } \angle E = \begin{array}{r} 300^{\circ}50' \\ 40^{\circ}05' \\ \hline 260^{\circ}45' \end{array}$$

$$\therefore \text{included } \angle E = 360^{\circ} - 260^{\circ}45' = 99^{\circ}15'$$

- ⇒ Difference:
- (i) $179^{\circ}15'$
 - (ii) $182^{\circ}15'$
 - (iii) $180^{\circ}10'$
 - (iv) $180^{\circ}45'$
 - (v) $179^{\circ}40'$

Cause: local attraction (magnetic substance - এর উপস্থিতি)

$$\angle A = \begin{array}{r} 221^{\circ}10' \\ - 71^{\circ}05' \\ \hline 150^{\circ}05' \end{array}$$

$$\textcircled{ii} \text{ FB of line DE} = 341^{\circ}40' - \angle D$$

$$= 241^{\circ}70'$$

$$\text{BB of line DE} = 241^{\circ}70' - 180^{\circ}$$

$$= 41^{\circ}70'$$

$$\textcircled{iii} \text{ FB of line EA} = 360^{\circ} - \angle E$$

$$= 261^{\circ}45'$$

$$= 261^{\circ} + 41^{\circ} = 302^{\circ}$$

$$\text{BB of line EA} =$$

$$= 122^{\circ}$$

$$\textcircled{iv} \text{ FB of line AB} = \text{BB of line EA} - \angle A$$

$$= 72^{\circ}10'$$

$$\text{BB of line AB} = 72^{\circ}10' + 180^{\circ}$$

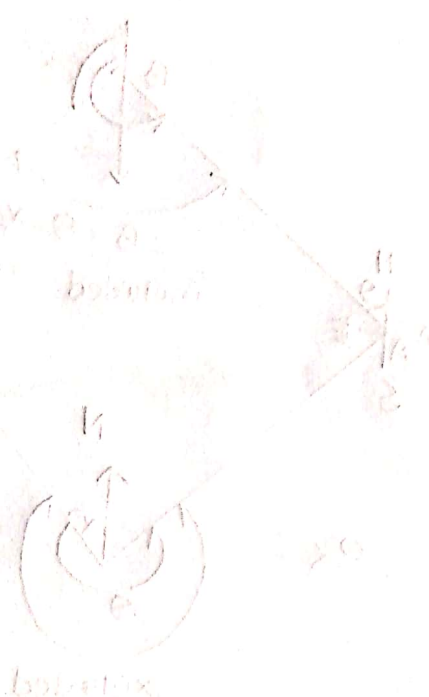
$$= 252^{\circ}10'$$

$$\textcircled{v} \text{ FB of line BC} = \text{BB of AB} - \angle B$$

$$= 112^{\circ}25'$$

$$\text{BB of line BC} = 112^{\circ}25' + 180^{\circ}$$

$$= 292^{\circ}25'$$

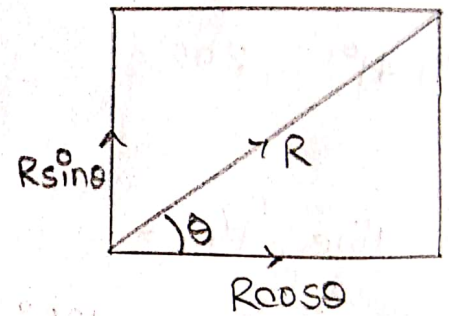
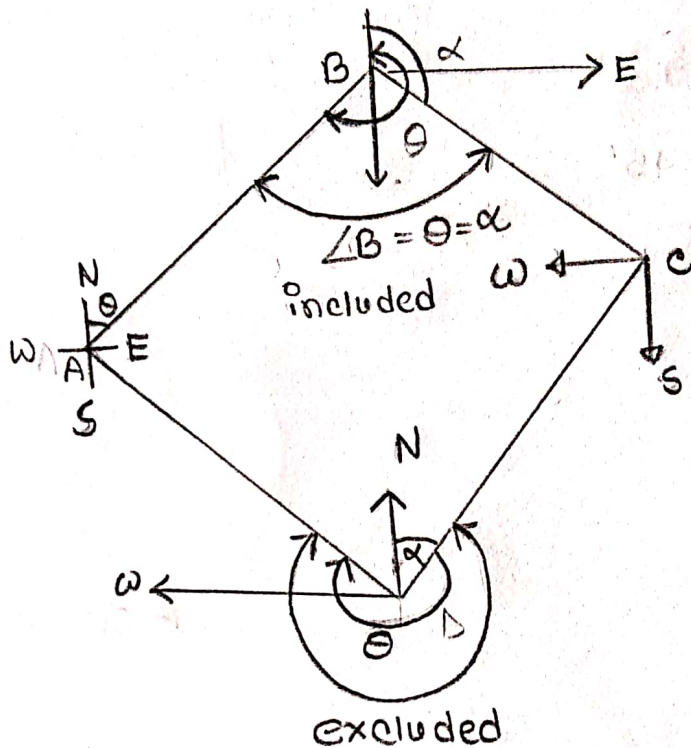


CHECK

$$FB \text{ of line } CD = BB \text{ of line } BC - \angle C$$

$$= 292^\circ 25' - 130^\circ 45'$$

$$= 161^\circ 40'$$

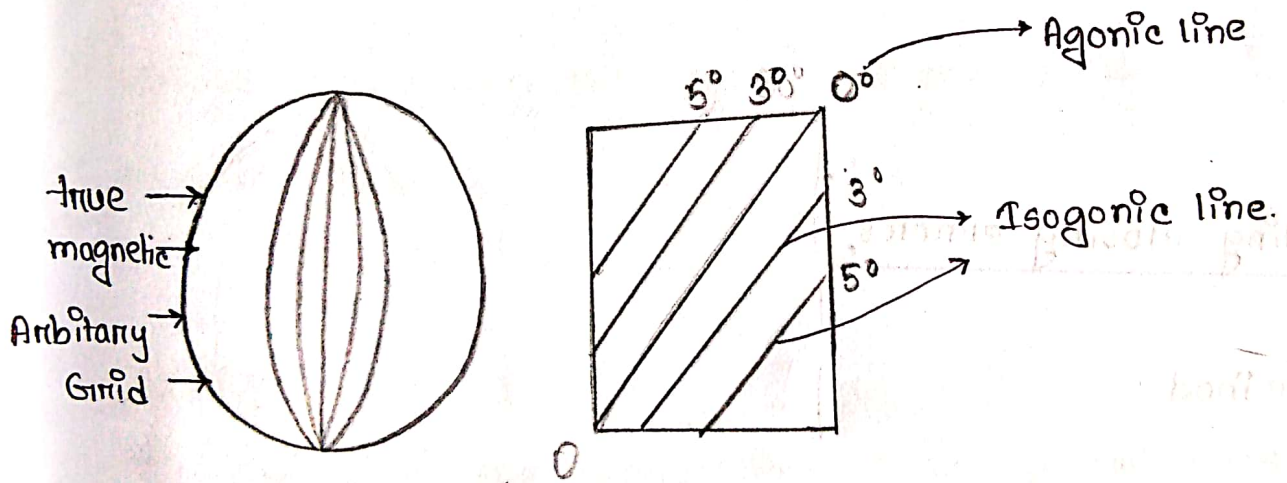


$\angle N = \angle S$ $\angle E = \angle W$

Methods of Traversing:

- (i) Compass traverse
- (ii) Theodolite "
- (iii) Plain Table "
- (iv) Chain "

Inclination of magnetic needle (DIP):

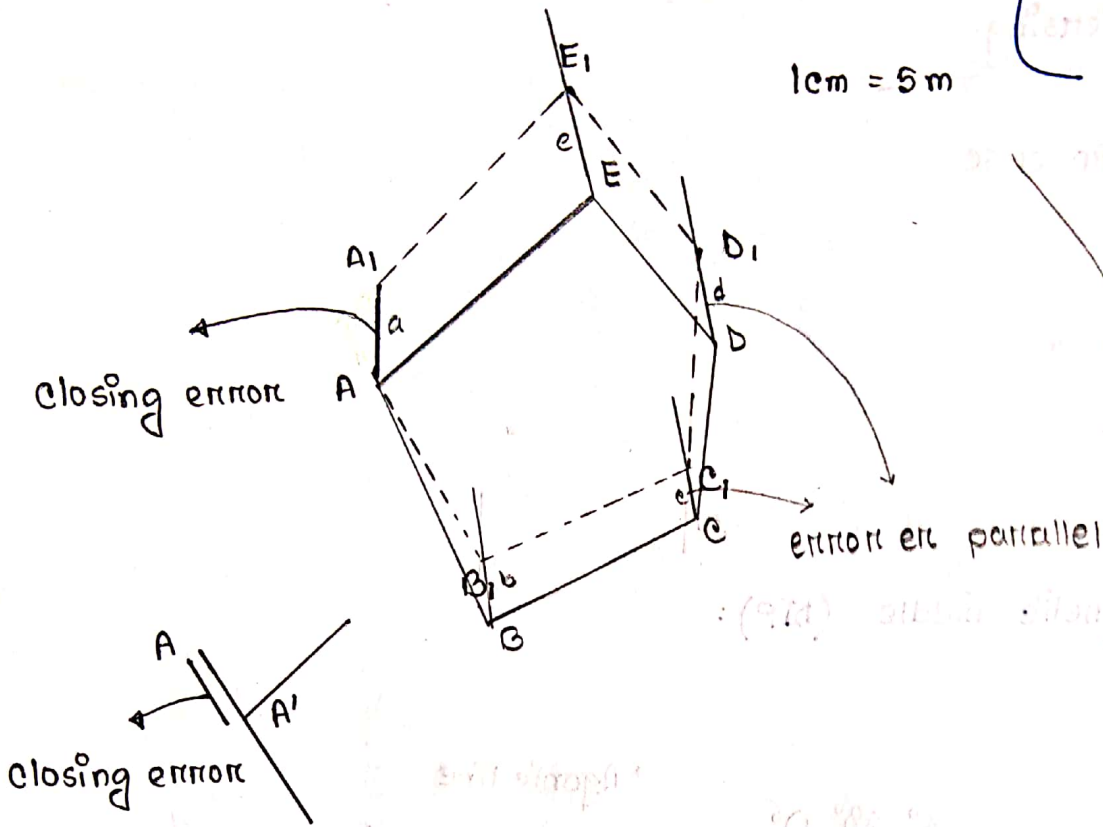


- True meridian →
- Magnetic " →
- Arbitrary →
- Grid →

720 sq. ft = 1 khata

435.6 sq. ft = 1 Decimal (කාලස)

1cm = 5m

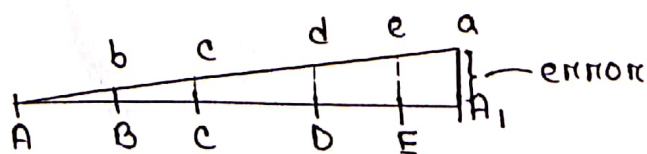


Method of correcting closing errors

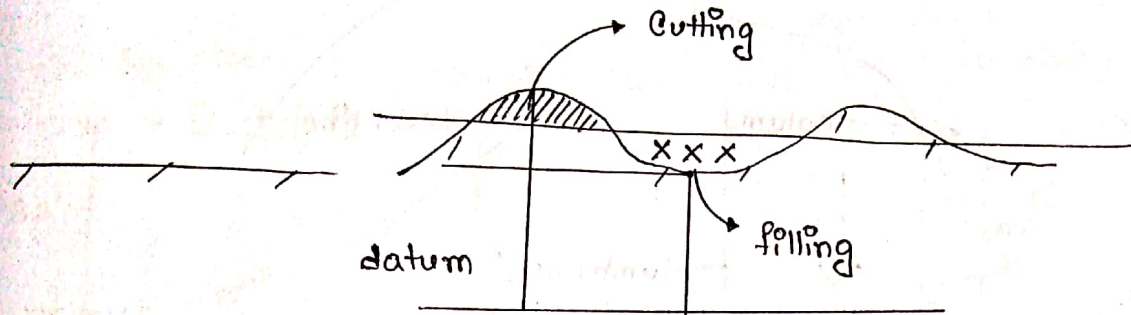
- ① Bowditch's method
- ② Transit's "
- ③ Graphical "
- ④ Axis "

Graphical method:

1cm = 10m



LEVELLING



levelling is the process by which the difference elevation (height) of various points on the earth surface is determined with respect to a given datum.

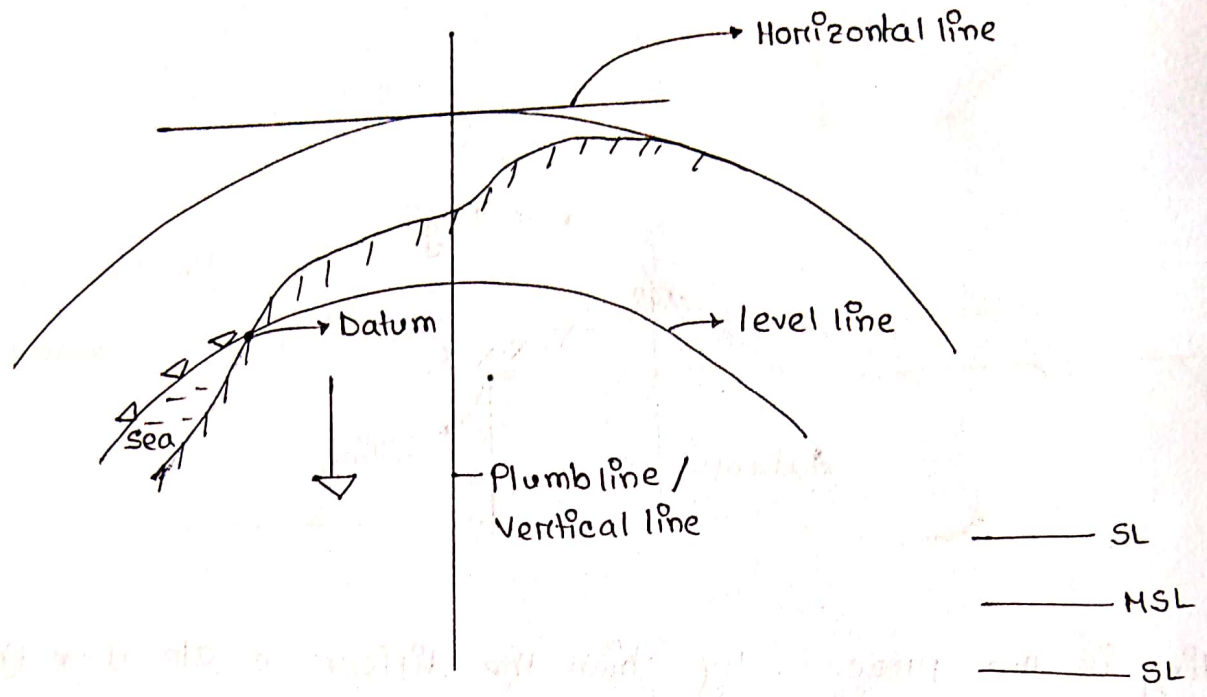
Objectives:

To determine the undulation of the ground surface by knowing the relative height of different objects on or below the earth surface.

Uses:

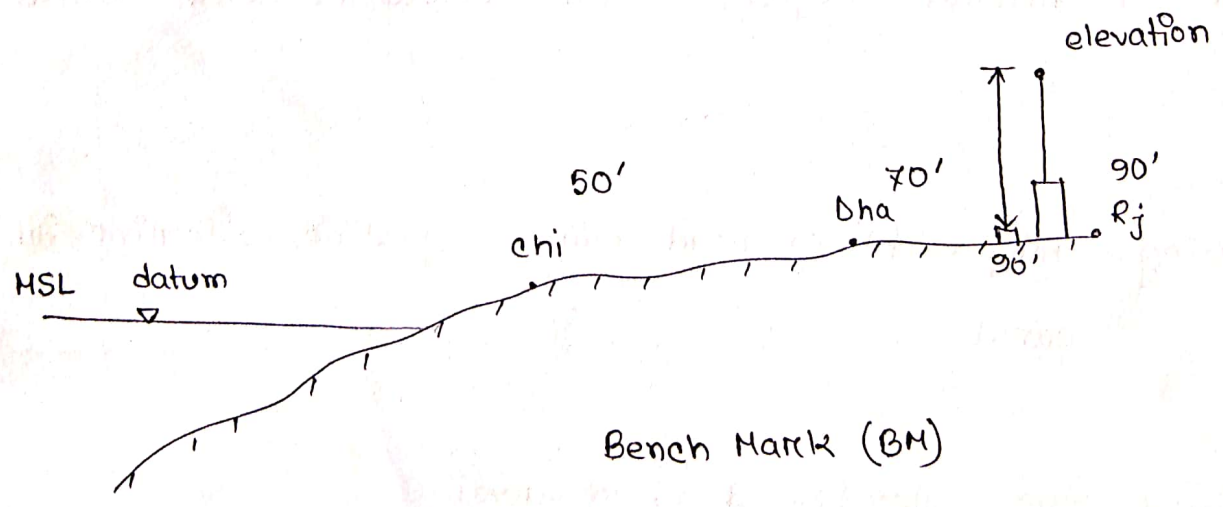
- ① For fixing - Alignment of road, railway, gasline, waterline, irrigation canal
- ② For fixing sites sites for dam, reservoir

LEVELLING

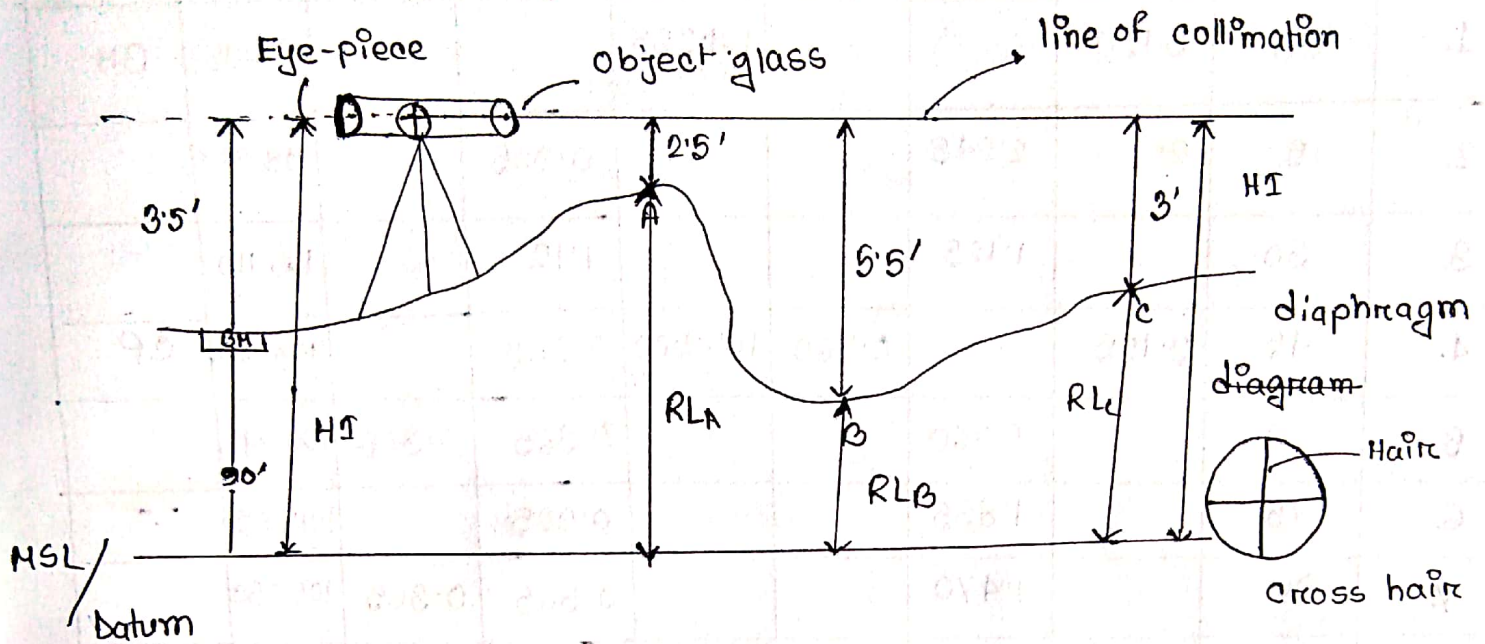


Datum is an imaginary level surface or level line from which elevation of different points are measured.

In Bangladesh we consider **Mean Sea Level** at Cox's Bazar, is considered as datum.



Bench Mark is the fixed point or mark of known elevation determined with reference to the datum line.



levelling staff

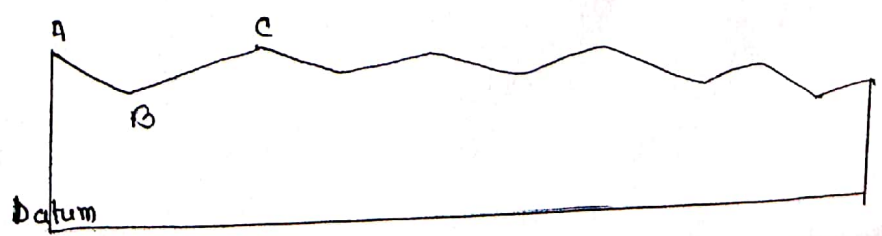
$$\begin{aligned}
 HI &= \text{Height of the instrument} \\
 &= 90' + 3.5' \\
 &= 93.5'
 \end{aligned}$$

$$\begin{aligned}
 RL_A &= 93.5' - 2.5' \\
 &= 91.0'
 \end{aligned}$$

$$\begin{aligned}
 RL_B &= 93.5' - 5.5' \\
 &= 88'
 \end{aligned}$$

$$\begin{aligned}
 RL_C &= 93.5' - 3' \\
 &= 90.5'
 \end{aligned}$$

RL = Reduced level

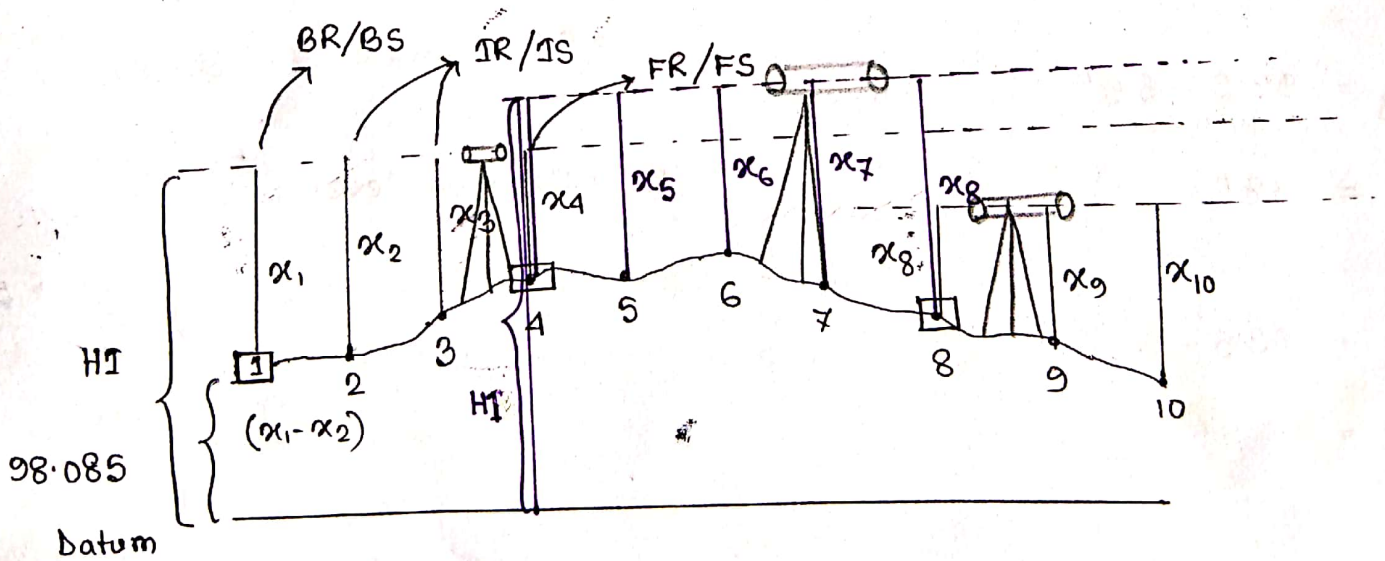


$$100.375 + 3.125 = 103.500$$

$$\alpha_1 - \alpha_2 \rightarrow \oplus \rightarrow \text{Rise}$$

$$\ominus \rightarrow \text{Fall}$$

Stations	Distance	Staff Readings (m)			HT	Difference		RL of Stations (m)	Remark
		BR	IR	FR		Rise	Fall		
1.	00	3.150			101.235 = 3.15 + 98.085			98.085	BM
2.	15		2.245			0.905		98.990	
3.	30		1.125			1.12		100.110	
4.	45	3.125		0.860	103.500	0.265		100.375	CP
5.	60		2.760			0.365	0.365	100.740	
6.	75		1.835			0.925		101.665	
7.	90		1.470			0.365		102.030	
8.	105	1.225		1.965	102.760 98.265		0.495	101.535	CP
9.	120		2.390				1.165	100.37	
10.	135			3.035			0.645	99.725	



Line of collimation Method / Height of the Instrument Method

$$\sum BR - \sum FR = \text{last RL} - \text{1st RL}$$

$$\Rightarrow 7.5 - 5.86 = 99.725 - 98.083$$

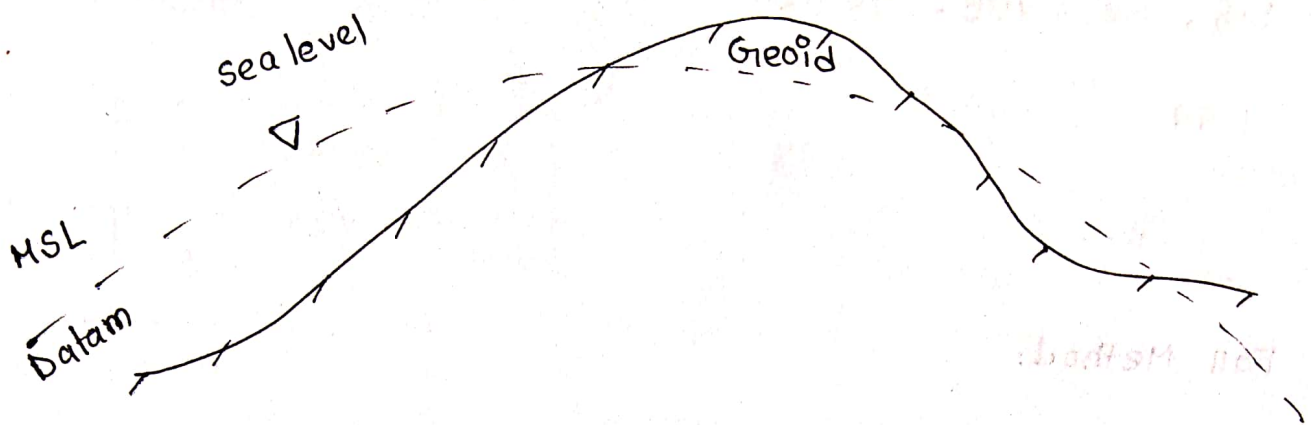
$$\Rightarrow 1.64 = 1.64$$

Rise and Fall Method:

$$\sum \text{Rise} - \sum \text{Fall} = \text{last RL} - \text{1st RL}$$

$$\Rightarrow \begin{array}{l} 3.945 - 2.305 \\ \cancel{3.58} - \cancel{2.67} \end{array} = 99.725 - 98.085$$

$$\Rightarrow 1.64 = 1.64$$



Geoid: .. is the surface of the earth at mean sealevel.

Parallax: The apparent movement of the image relative to the cross hairs is known as parallax.

Types of level:

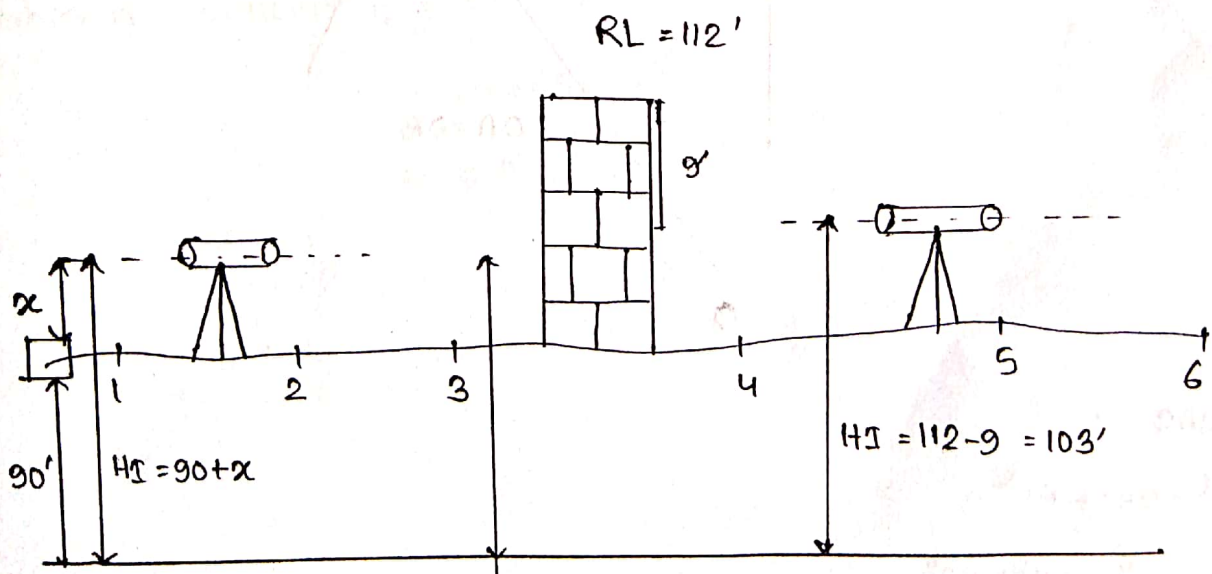
- (i) Dumpy level.
- (ii) Y-level (wye level)
- (iii) Reversible level
- (iv) Tilting level
- (v) Automatic level

Types of levelling staff:

- ① One-length staff
- ② Folding "
- ③ Telescopic "
- ④ Target "

Types of benchmark:

- ① GTS (Great Trigonometrical survey) (mean sea level)
- ② Permanent
- ③ Arbitrary



$$\Rightarrow R^2 + 2RC_c + C_c^2 = R^2 + D^2$$

$$\Rightarrow 2RC_c = D^2$$

$$\Rightarrow C_c = \frac{D^2}{2R}$$

$$R = 6370 \text{ km}$$

$$\therefore C_c = \frac{D^2}{2 \times 6370} \text{ (km)}$$

$$= \frac{1000 \times D^2}{2 \times 6370} \text{ (m)}$$

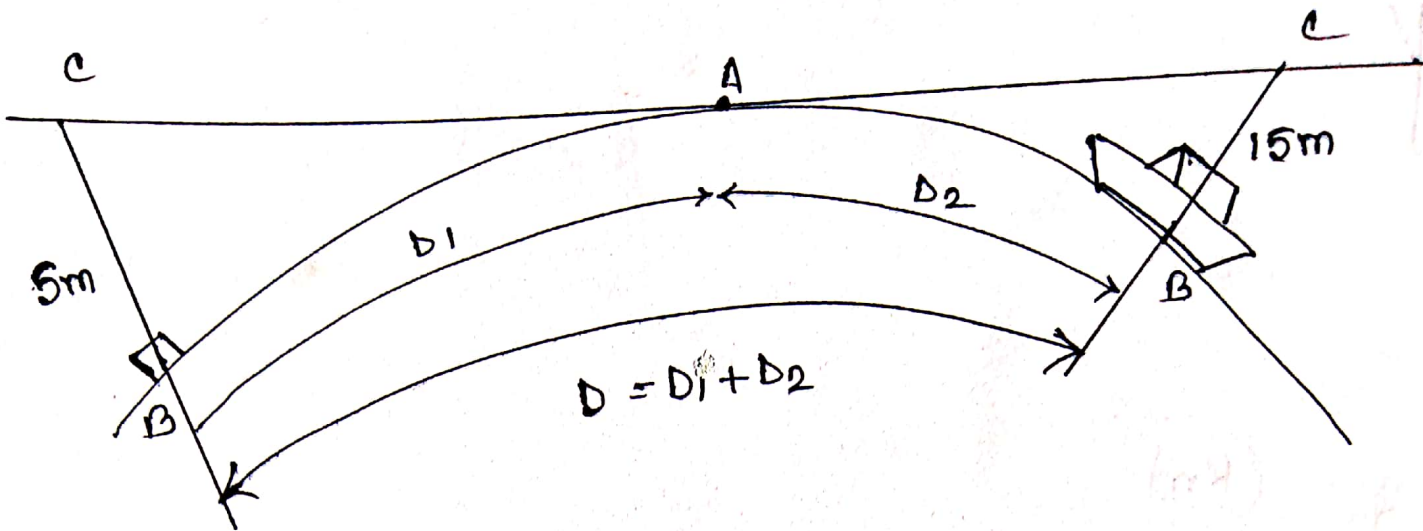
$$C_c = 0.0785 D^2$$

Correction for refraction, $C_r = \frac{1}{7} \cdot C_c$

$$\text{Combined correct, } c = C_c - C_r$$

$$= 0.06728 D^2$$

$$\approx 0.0673 D^2$$



$$C = 0.0673 D^2$$

$$h_1 = 0.0673 D^2$$

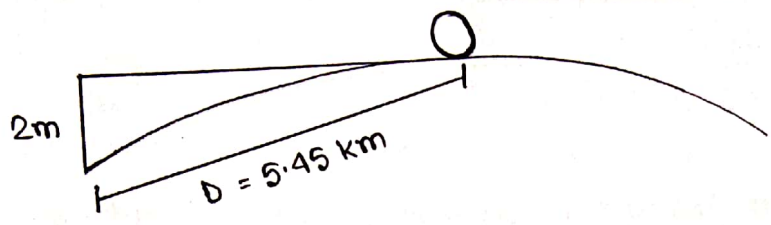
$$\Rightarrow 5 = 0.0673 D^2$$

$$\Rightarrow D_1 =$$

$$h_1 = 0.0673 D_1^2$$

$$\Rightarrow D_1 = \sqrt{\frac{h_1}{0.0673}}$$

Find the distance of the sun from the observer when just see the sunset.
 When the height of the observer is 2m.



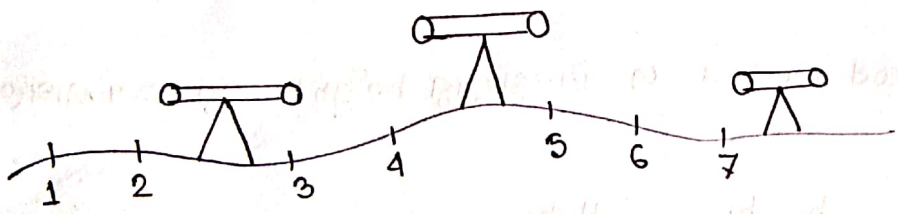
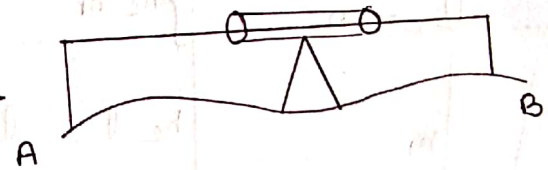
$$h = 0.0673 D^2$$

$$D = \left(\frac{h}{0.0673} \right)^{1/2}$$

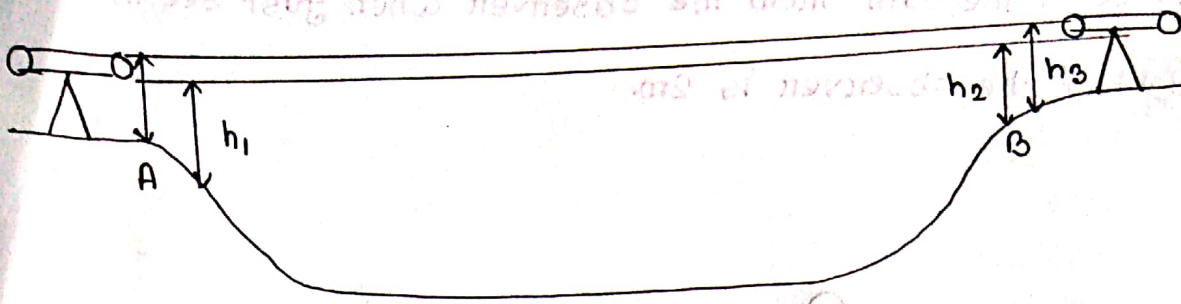
$$= 883.140 \text{ m}$$

Types of levelling operation:

1. Simple levelling
2. Differential levelling



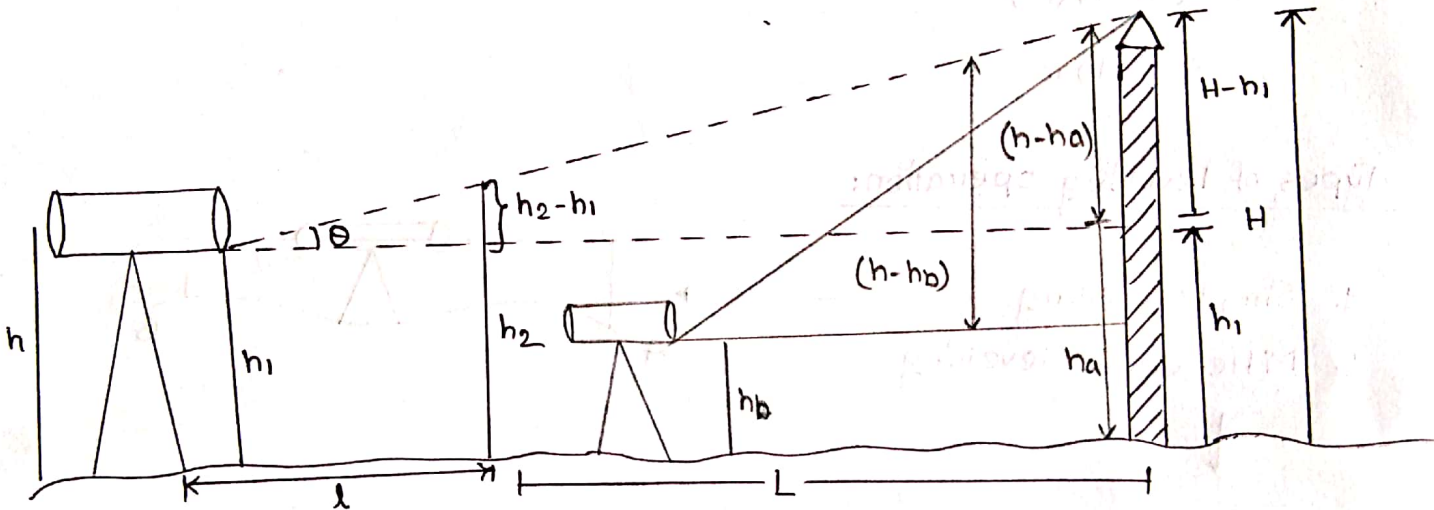
3. Fly levelling
4. longitudinal / Profile "
5. Cross sectional "
6. Reciprocal "
7. Trigonometrical "
8. Contouring levelling



$$d_1 = h_1 - h_2$$

$$d_2 = h_3 - h_4$$

$\left(\frac{d_1 + d_2}{2}\right) \rightarrow$ A থেকে ও এর height এর গড় (A ও B station বাতুলু বুলু ঠিক না কি নাটু)



[Tape and ranging rod method - এ টাওয়ারের height বের করা মাগলে]

$$h_1 =$$

$$h_2 =$$

$$l =$$

$$L =$$

$$\frac{h_2 - h_1}{l} = \frac{H - h_1}{L + l}$$

$$\therefore H = ?$$

[অঙ্কবোঝি নিভুল]

২মি- theodolite use করা হয়, এখন এ জুয়া হবে,

$$h = \text{known}$$

$$L = h$$

$$\theta =$$

$$\tan \theta = \frac{H - h}{L}$$

যদি obstruction থাকে, তাহলে প্রারম্ভে theodolite use করা হয়।

$h_a, h_b, l, \theta, \alpha \rightarrow \text{known}$

$H, L \rightarrow \text{unknown}$

$$\tan \theta = \frac{H - h_a}{l + L}$$

$$\tan \alpha = \frac{H - h_b}{L}$$

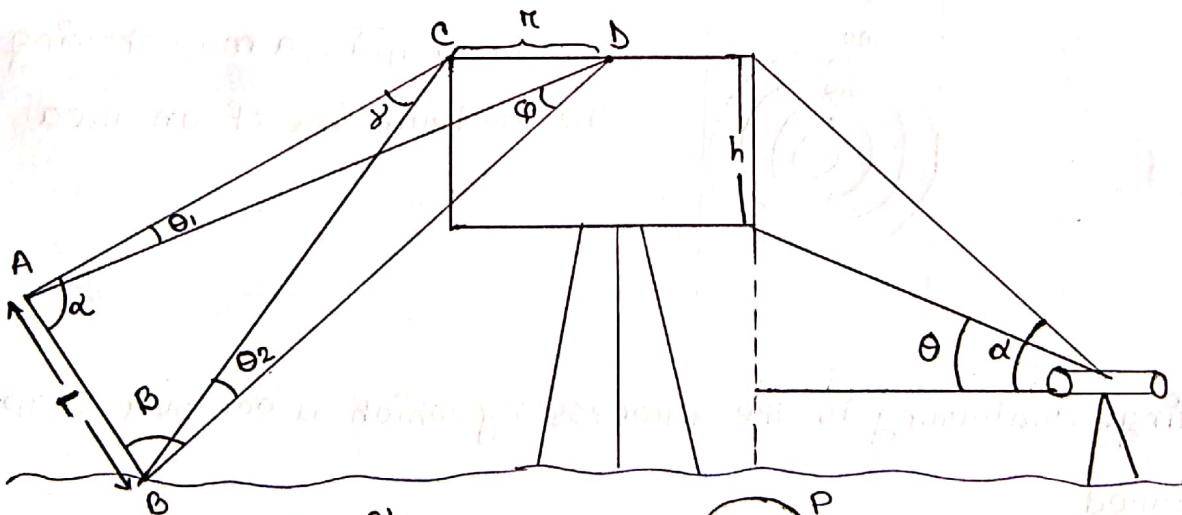
now, $L = \frac{H - h_b}{\tan \alpha}$ [L এর মান $\tan \theta$ এর eqn এ বসিয়ে]

এখানে ৩টি পদ্ধতি ব্যবহার করা হয়েছে-

(i) Ranging rod হিসেবে (h_1, h_2)

(ii) One theodolite (h, θ)

(iii) Two " ($h_a, h_b, \alpha, \theta, l$)



$$V = Ah$$

$$= \pi \pi^2 h$$

$$\frac{P}{b} = \frac{P}{D} = \pi = 3.14159265659$$

$$\pi = CD$$

$$CD^2 = AC^2 + AD^2 - 2AC \cdot AD \cdot \cos \theta_1 \quad \text{--- (1)}$$

From ΔABC ,

$$\frac{AC}{\sin(\beta - \theta_2)} = \frac{AB}{\sin \gamma}$$

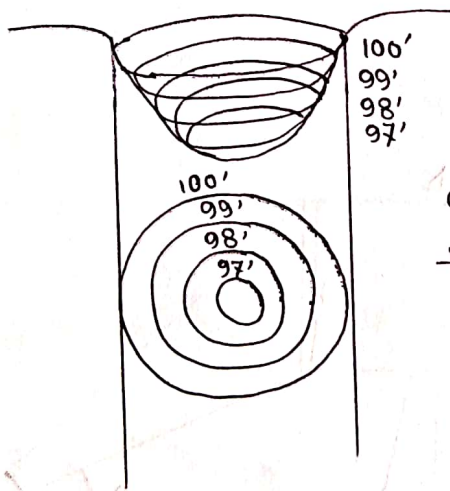
ΔADB ,

$$\frac{AD}{\sin \beta} = \frac{AB}{\sin \phi}$$

$\therefore AD = ?$

① Contour / Contour line: Contour line is a imaginary line joining points of equal RL with respect to datum.

② Contour map:



contour map is a map showing only the contour line of an area.

③ Contouring: Contouring is the process by which a contour map is prepared.

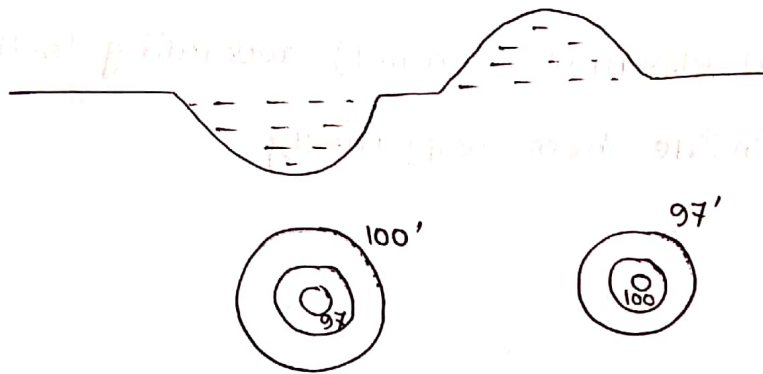
Contour interval: Vertical distance between any two consecutive contours.

Horizontal equivalent: Horizontal distance between two consecutive contours.

Objectives: (i) To know the nature of the ground.

(ii) Suitable alignments can be selected and volume of earthwork can be estimated.

Uses: (i) The possible route (the direction) can be demarcated.

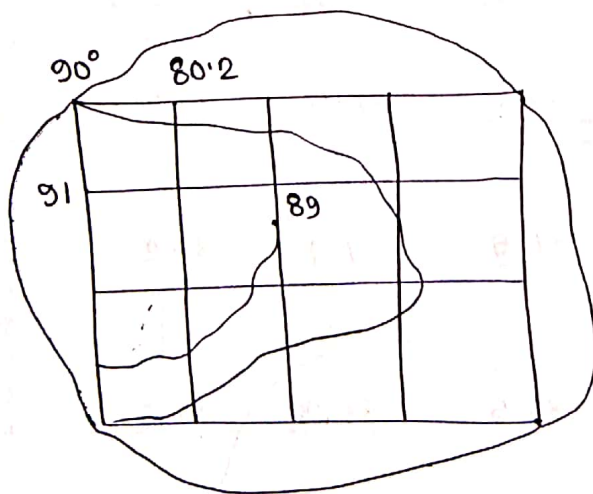


(ii) Economical alignment can be selected.

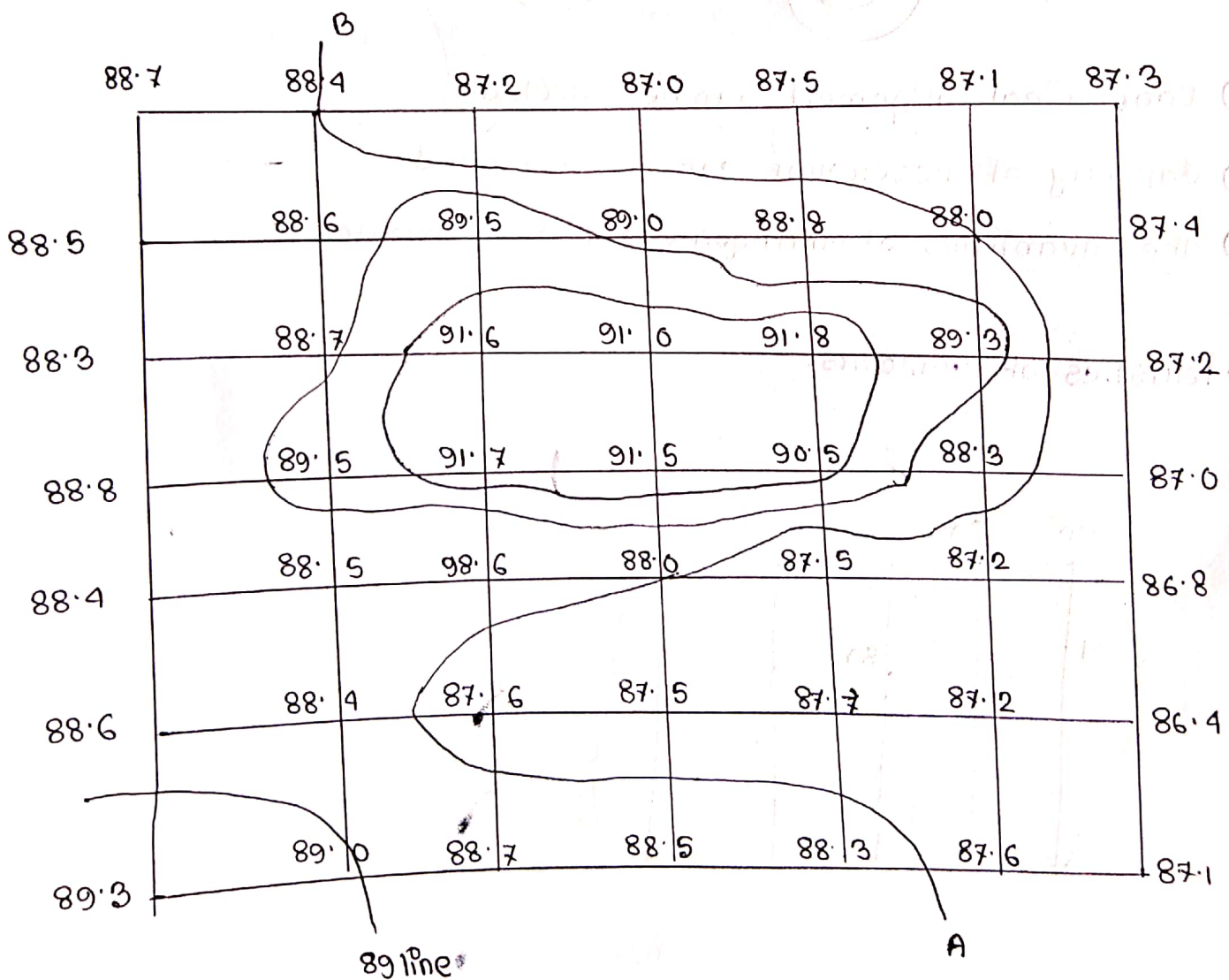
(iii) Capacity of reservoir can be computed.

(iv) The quantities of earthwork can be computed.

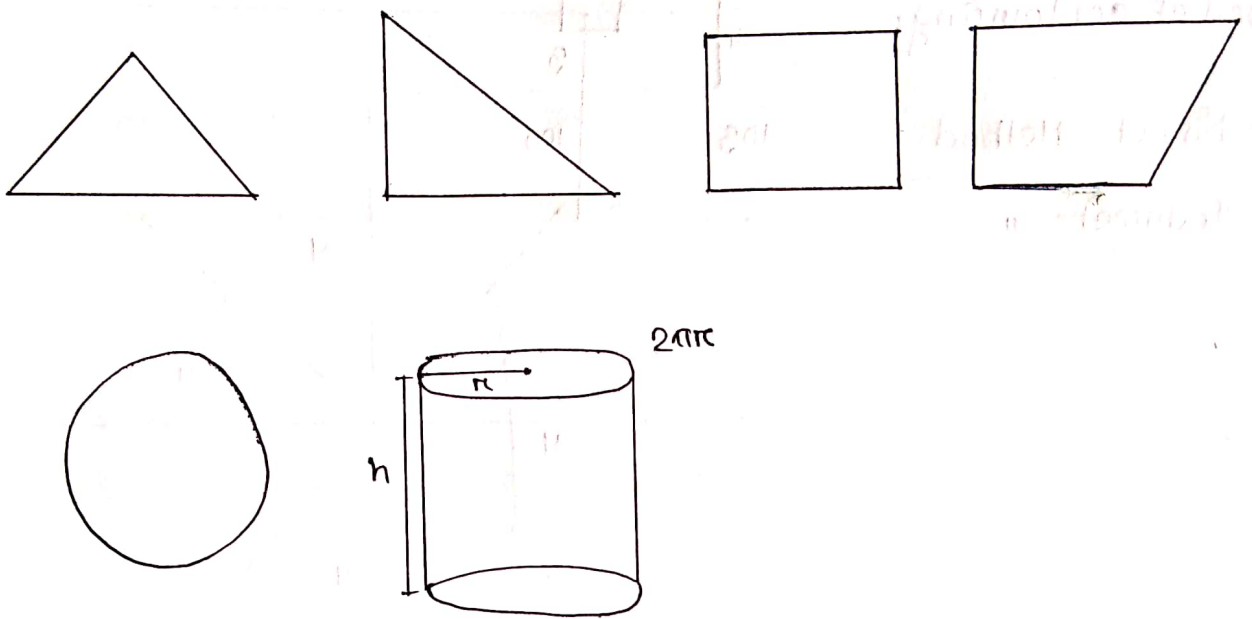
Characteristics of contours:



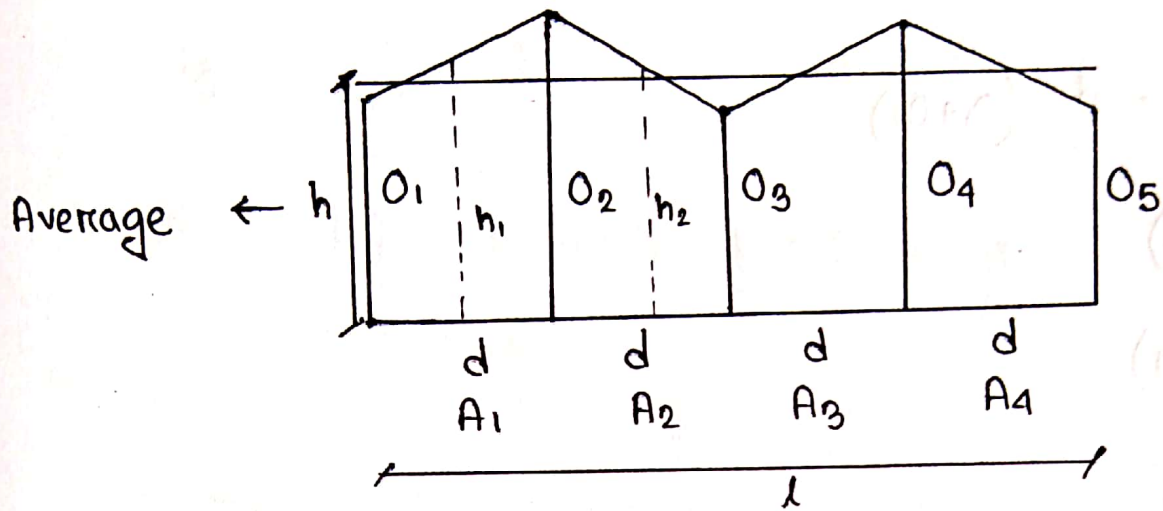
1. Contour line can not cross one another except overhanging cliff.
2. Each contour lines must be a continuous, until it either goes off the map or closes itself.
3. Uniformly spaced contour lines indicate a uniform slope.
4. A series of closed contours on the map indicates a depression or an elevation (summit) according to the lower or higher values inside them respectively.



Contour gradient: During preliminary survey for roads in a hill area, the required points are first established along the gradient. The line joining these points is known as the contour gradient or grade contour.



Computation of Area



Rule: 01 → Mid-Ordinate Rule

$$A_1 = h_1 d$$

$$A_2 = h_2 d$$

$$A_3 = h_3 d$$

$$\therefore A = d(h_1 + h_2 + h_3 + \dots)$$

Rule: 02 → Average-Ordinate Rule

$$h = \frac{O_1 + O_2 + O_3 + O_4 + O_5}{5}$$

$$A = h \times 4d$$

$$\therefore A = \left(\frac{O_1 + O_2 + O_3 + \dots + O_n}{n} \right) (n-1) d$$

$$\text{OR, } A = \left(\frac{O_0 + O_1 + \dots + O_n}{n+1} \right) (n-1) d$$

Rule → 03

Trapezoidal Rule:

$$A_1 = \frac{1}{2} (O_1 + O_2) d = \frac{d}{2} (O_1 + O_2)$$

$$A_2 = \frac{d}{2} (O_2 + O_3)$$

$$A_3 = \frac{d}{2} (O_3 + O_4)$$

$$A_4 = \frac{d}{2} (O_4 + O_5)$$

$$A = \frac{d}{2} [O_1 + O_2 + O_2 + O_3 + O_3 + O_4 + O_4 + O_5]$$

$$= \frac{d}{2} [O_1 + O_5 + 2(O_2 + O_3 + O_4)]$$

$$\text{Area, } A = \frac{\text{common distance}}{2} \left[\text{1st Ordinate} + \text{last Ordinate} + 2 \left(\text{Sum of other ordinates} \right) \right]$$

Rule → 04

Simpson's Rule:

$$ACBEFA = ACBEFA + \cancel{A}DEFEB$$

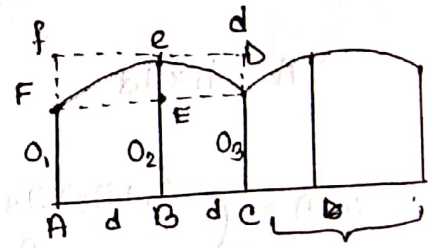
$$= \frac{1}{2} (O_1 + O_3) 2d + \frac{2}{3} \left(O_2 - \frac{O_1 + O_3}{2} \right) 2d$$

$$= d(O_1 + O_3) + \frac{4d}{3} \left(\frac{2O_2 - O_1 - O_3}{2} \right)$$

$$= d(O_1 + O_3) + \frac{2d}{3} (2O_2 - O_1 - O_3)$$

$$= \frac{d}{3} [3O_1 + 3O_3 + 4O_2 - 2O_1 - 2O_3]$$

$$= \frac{d}{3} [O_1 + 4O_2 + O_3]$$



$$EB = \left(\frac{O_1 + O_3}{2} \right)$$

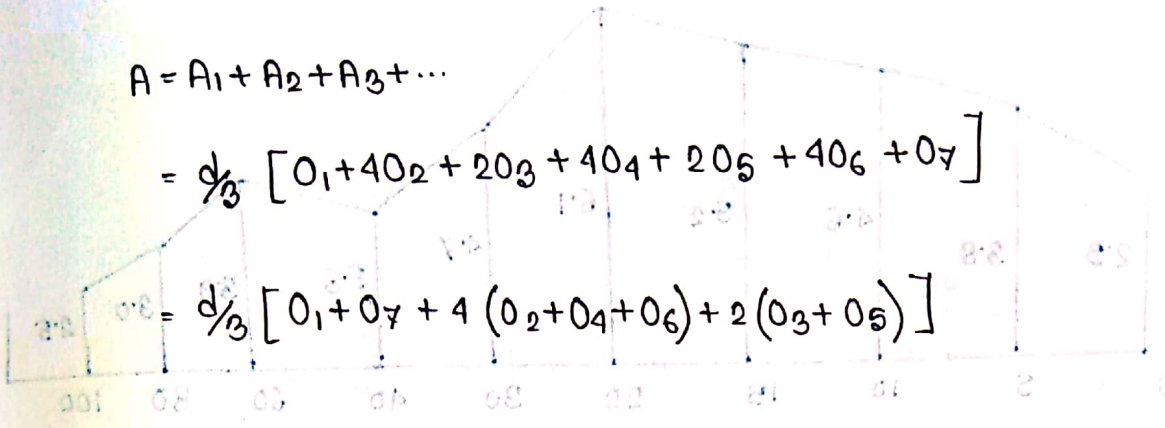
$$eE = O_2 - \left(\frac{O_1 + O_3}{2} \right)$$

$$A_3 = \frac{d}{3} (O_5 + 4O_6 + O_7)$$

$$A = A_1 + A_2 + A_3 + \dots$$

$$= \frac{d}{3} [O_1 + 4O_2 + 2O_3 + 4O_4 + 2O_5 + 4O_6 + O_7]$$

$$= \frac{d}{3} [O_1 + O_7 + 4(O_2 + O_4 + O_6) + 2(O_3 + O_5)]$$



$$\text{Area, } A = \frac{\text{common distance}}{3} \left[\text{1st Ordinate} + \text{last Ordinate} + 4 (\text{Sum of even ordinates}) + 2 (\text{Sum of odd remaining odd ordinates}) \right]$$

[This rule is applicable for only ~~even~~ ^{odd} ordinates.]

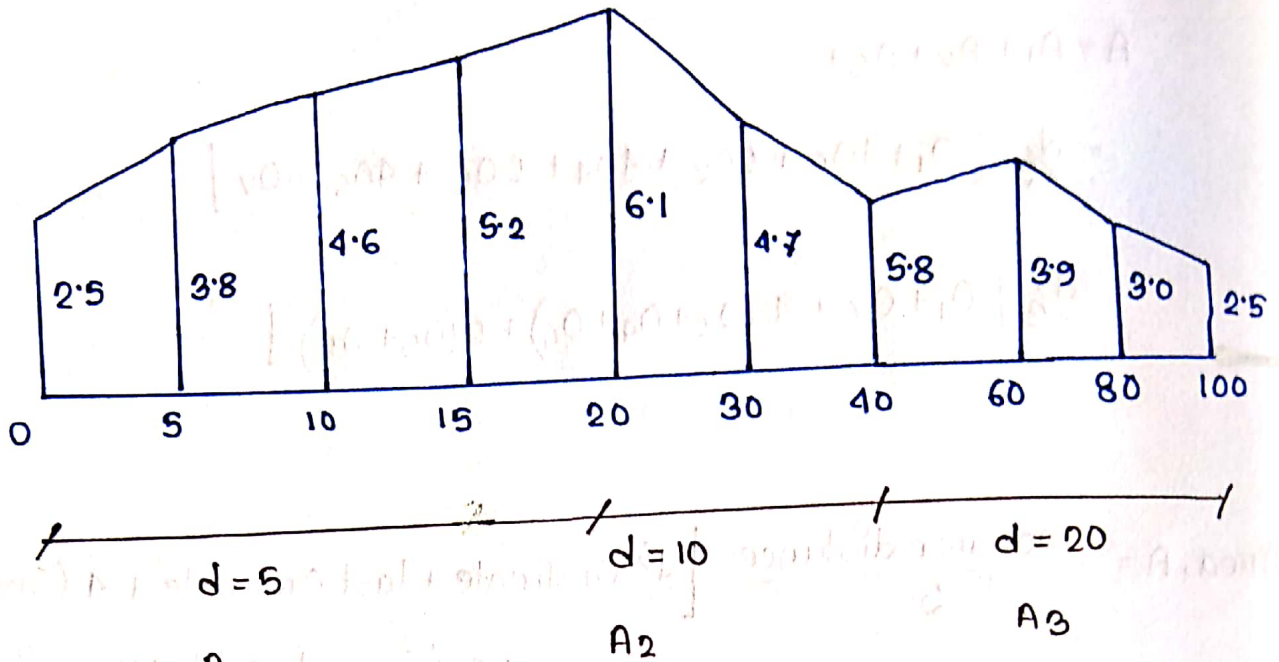
[This rule is applicable for only odd ordinates.]

$$\text{For area} = \left[(0.8 + 0.4 + 0.2) \times \frac{1}{3} + 1.2 \times \frac{1}{3} \right] \times \frac{1}{3} = 1.1$$

$$\text{For area} = \left[(0.8) \times \frac{1}{3} + 1.2 \right] \times \frac{1}{3} = 0.6$$

$$\text{For area} = \left[(0.2 + 0.2) \times \frac{1}{3} + 0.2 \times \frac{1}{3} \right] \times \frac{1}{3} = 0.2$$

#



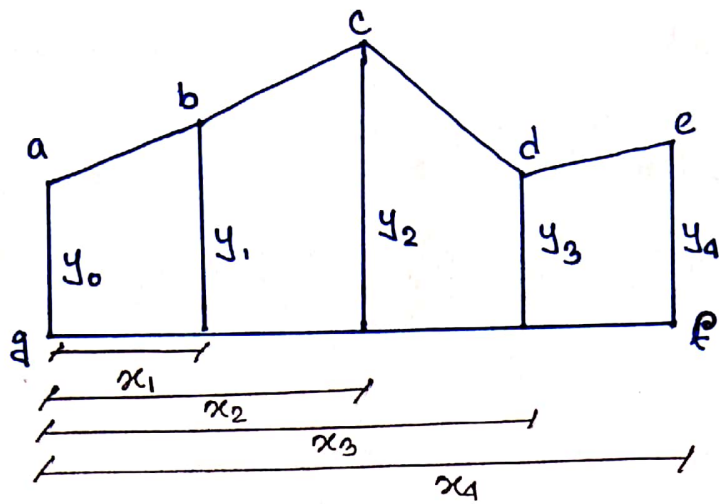
Trapezoidal Rule:

$$\text{Area} = \frac{\text{common distance}}{2} \left[\text{1st ord.} + \text{last ord.} + 2 (\text{Sum of other ord.}) \right]$$

$$A_1 = \frac{5}{2} \left[2.5 + 6.1 + 2(3.8 + 4.6 + 5.2) \right] = 89.5 \text{ m}^2$$

$$A_2 = \frac{10}{2} \left[6.1 + 5.8 + 2(4.7) \right] = 106.5 \text{ m}^2$$

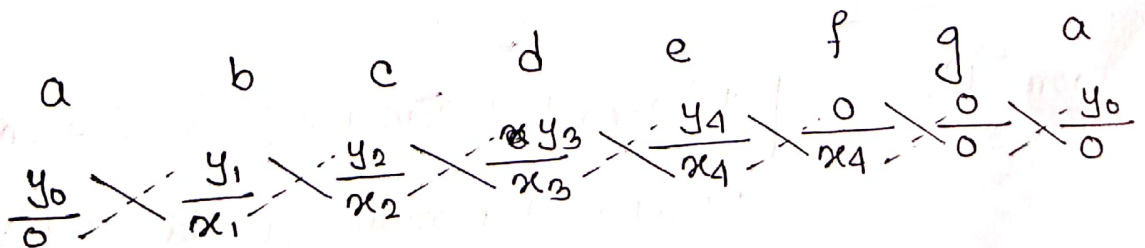
$$A_3 = \frac{20}{2} \left[5.8 + 2.5 + 2(3.9 + 3.0) \right] = 221 \text{ m}^2$$



Rule → OS

Co-ordinate Rule:

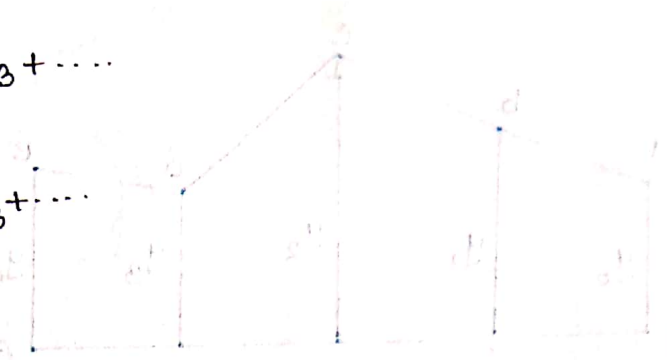
Points	Co-ordinate	
	x	y
a	0	y_0
b	x_1	y_1
c	x_2	y_2
d	x_3	y_3
e	x_4	y_4
f	x_4	0
g	0	0
a	0	y_0



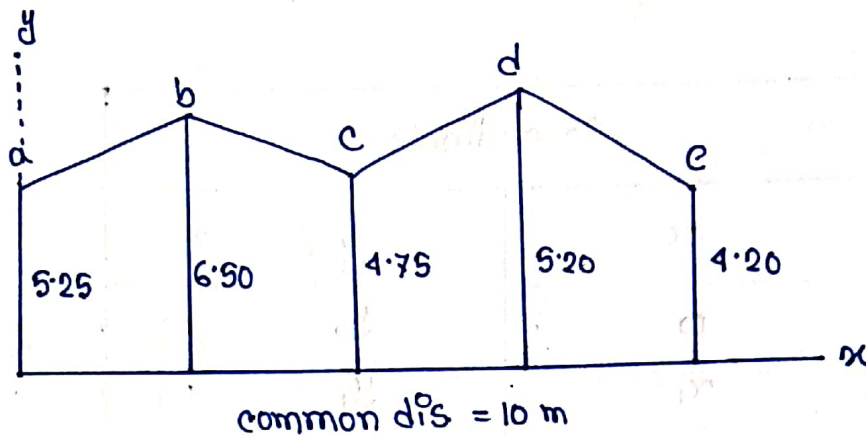
$$\sum P = y_0 x_1 + y_1 x_2 + y_2 x_3 + \dots$$

$$\sum Q = x_0 y_1 + x_1 y_2 + x_2 y_3 + \dots$$

$$A = \frac{1}{2} (\sum P - \sum Q)$$



#



① Trapezoidal Rule:

$$\text{Area} = \frac{\text{common dis.}}{2} \left[\text{1st Corr.} + \text{last Corr.} + 2 (\text{Sum of other ordinates}) \right]$$

$$A = \frac{10}{2} \left[5.25 + 4.20 + 2 (6.50 + 4.75 + 5.20) \right]$$

$$= 211.75 \text{ m}^2$$

② Simpson's Rule:

$$\text{Area} = \frac{\text{com. dis}}{3} \left[\text{1st Corr} + \text{last Corr.} + 4 (\text{Sum of even ord.}) + 2 (\text{sum of remaining odd ord.}) \right]$$

$$= \frac{10}{3} \left[5.25 + 4.20 + 4 (6.50 + 5.20) + 2 \times 4.75 \right]$$

$$= 219.17 \text{ m}^2$$



③ Co-ordinate Rule:

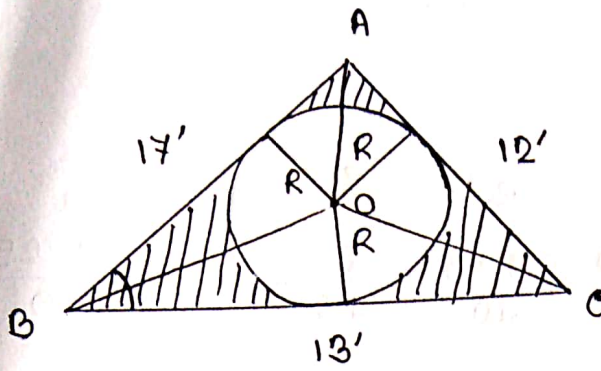
a	b	c	d	e	f	g	a
$\frac{5.25}{0}$	$\frac{6.50}{10}$	$\frac{4.75}{20}$	$\frac{5.20}{30}$	$\frac{4.20}{40}$	$\frac{0}{4.20}$	$\frac{0}{0}$	$\frac{5.25}{0}$

$$\begin{aligned} \sum P &= 5.25 \times 10 + 6.50 \times 20 + 4.75 \times 30 + 5.20 \times 40 + 4.20 \times 4.20 + 0 \times 0 + 0 \times 0 \\ &= 550.64 \quad 701 \end{aligned}$$

$$\begin{aligned} \sum Q &= 0 \times 6.50 + 10 \times 4.75 + 20 \times 5.20 + 30 \times 4.20 + 40 \times 0 + 4.20 \times 0 + 0 \times 5.25 \\ &= 277.5 \end{aligned}$$

$$\begin{aligned} A &= \frac{1}{2} (\sum P - \sum Q) \\ &= 211.75 \text{ m}^2 \end{aligned}$$

TR	SR
1. 	1. 
2. any number of ordinates	2. only for ^{even} odd number of ordinate.
3. Approximate result	3. Accurate result



$$s = \sqrt{s(s-a)(s-b)(s-c)}$$

$$\Delta AOC = \frac{R}{2} \times 12$$

$$\Delta AOB = \frac{R}{2} \times 17$$

$$\Delta BOC = \frac{R}{2} \times 13$$

$$A = \frac{R}{2} (12 + 17 + 13)$$

[A এর মান বজালে R এর মান বের করা যাবে, তাহলে
circle এর area পাওয়া যাবে।

∴ shade এর area পাওয়া যাবে]