



Heaven 's Light is Our Guide

DEPARTMENT OF CIVIL ENGINEERING

RAJSHAHI UNIVERSITY OF ENGINEERING & TECHNOLOGY

Prepared by

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Course Title : Structural Analysis and Design-I

Course Code: CE 3111

Lecture: 3 hrs/ week ,

Credit: 3.00

Prereq. CE 2111

- **Content of Course:**

- Stability and determinacy of structures, analysis of statically determinate arches. Influence lines for statically determinate structure: **moving loads** on beams, frames and **trusses**. Cable supported structures and space trusses..

Reference Books

- 1. Theory of Simple Structures by T.C. Shedd and J.Vawter (2nd Edition)
- 2. Elementary Structural Analysis by Utku, Norris & Wilber (4th Edition)
- 3. Analysis of structures by V N Vazirani (Vol. 1) , M M Ratwani
- 4. Indeterminate Structural Analysis by J. Sterling Kinney

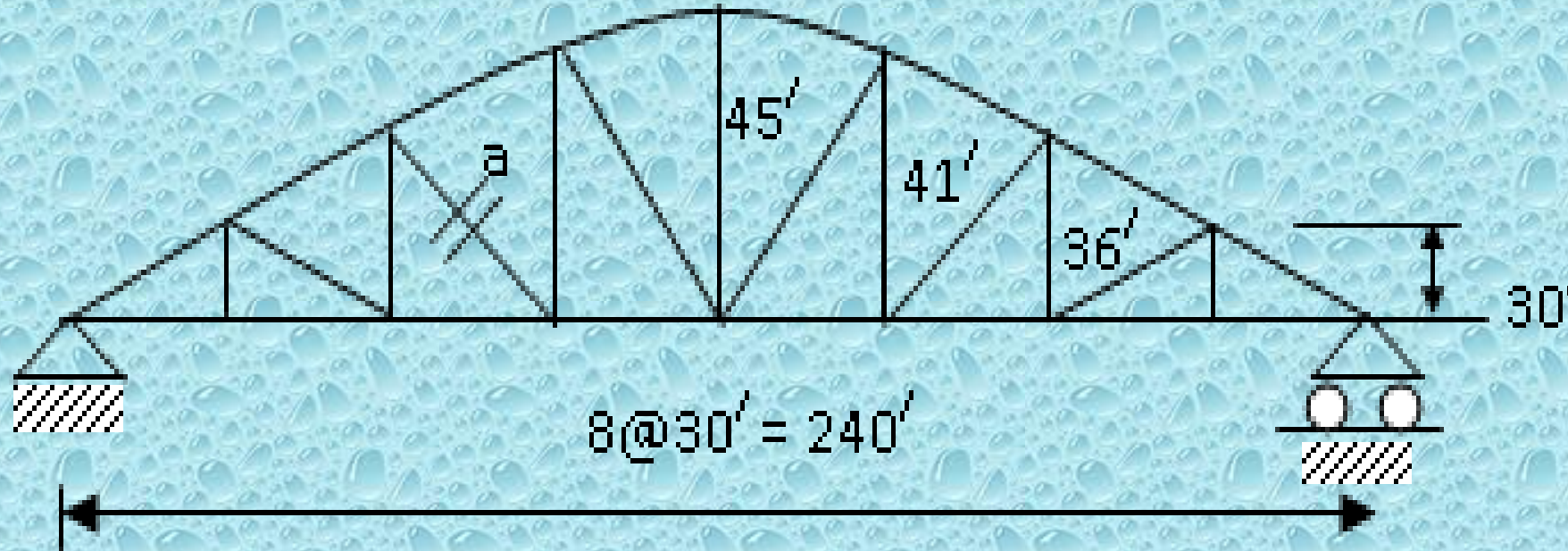
Topics:

Lane load on Trusses

Topics to be Covered

- Introduction
- Define Suspension Bridges
- Example of Suspension Bridges
Advantages and disadvantages of
Suspension Bridges
- Derive Formula for Analysis of
Suspension Bridges
- Sample Example to Analysis and Design
of Suspension Bridges

Introduction



Truss

The truss or framed structure are composed of interconnecting members and are supported in such a manner that they are capable of holding applied external force in static equilibrium. It must hold in equilibrium gravity forces that are applied as a consequence of its own weight.

1.2 DEFINITION OF TRUSS

A truss is a configuration, which is designed to sustain inclined, vertical, horizontal loads accruing at or between its points of support.

1.3 CHARACTERISTIC OF A TRUSS

- a) The general configuration of a truss is triangle.
- b) The members are connected of their end by frictionless pin.
- c) The loads are applied only at joints and not at the intermediate points of a member.

ADVANTAGES OF TRUSS OVER BEAM

Manufactured roof trusses provide a structurally efficient alternative to beams. They place greater emphasis on axial loading of members and less on bending. Associated advantages of trusses include:

- Strong but light to erect
- Can be made to suit most roof shapes
- Less onsite fabrication, therefore less site labour and less effected by bad weather
- Factory production allows automated production Better quality control is possible
- Trusses make maximum structural use of the timber
- Trusses are capable of long spans Internal walls are usually non-load bearing therefore lighter weight internal walls are possible

1. TYPES OF TRUSS

They are mainly three types

- a) Simple,
- b) Compound,
- c) Complex.

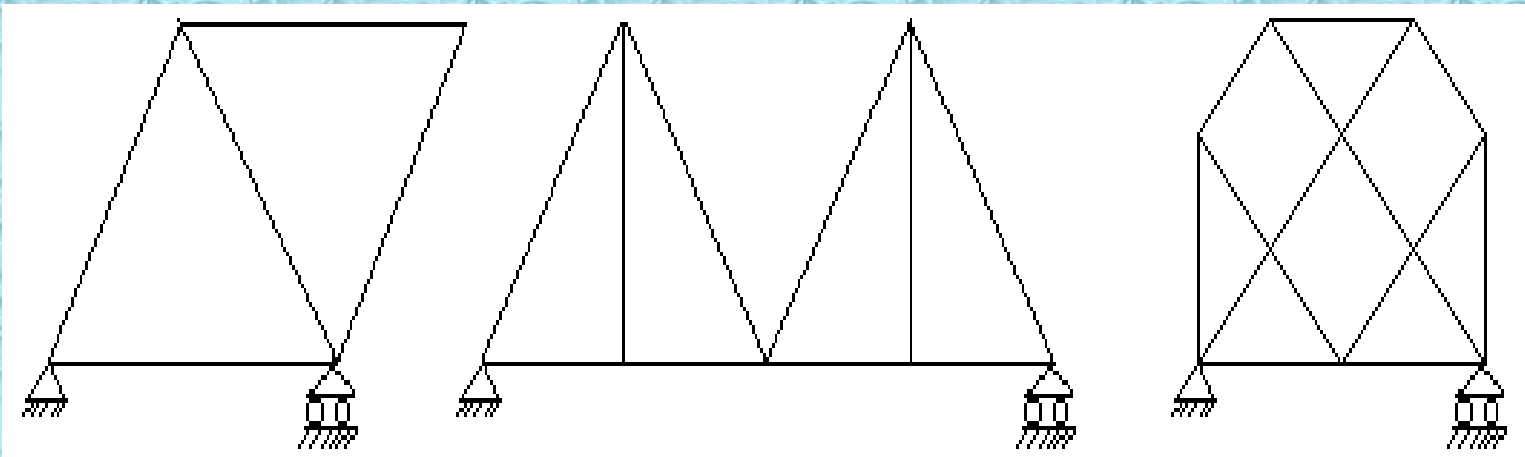
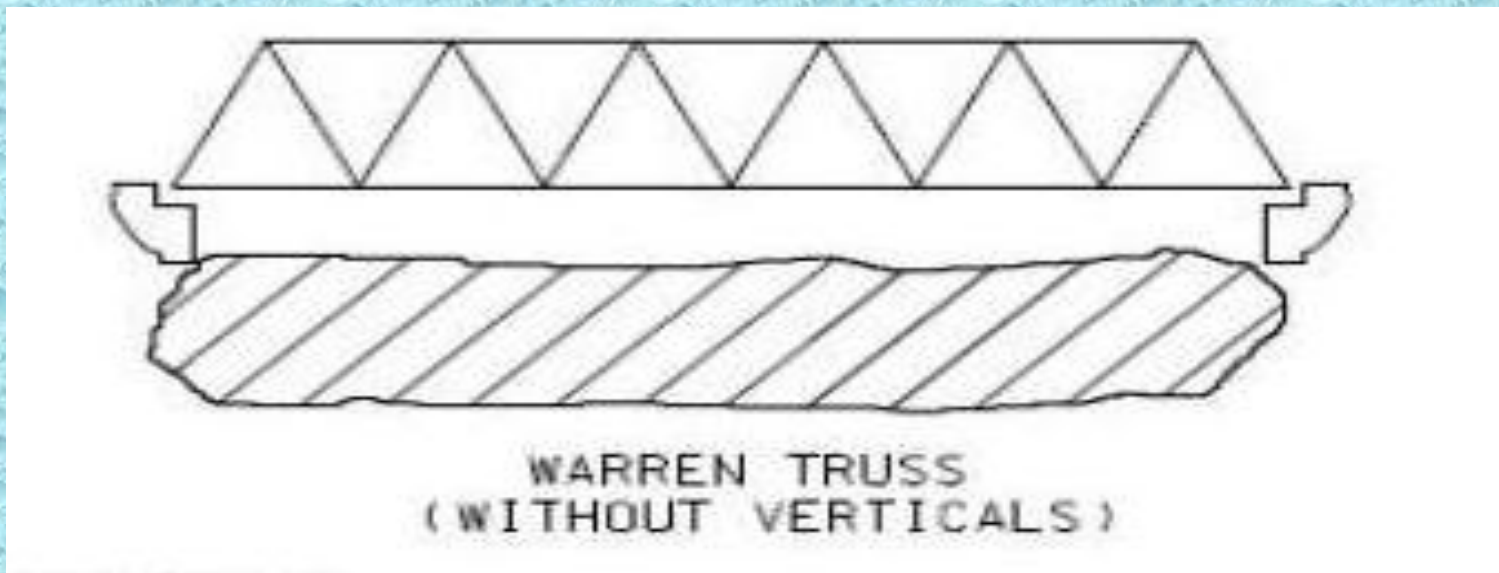
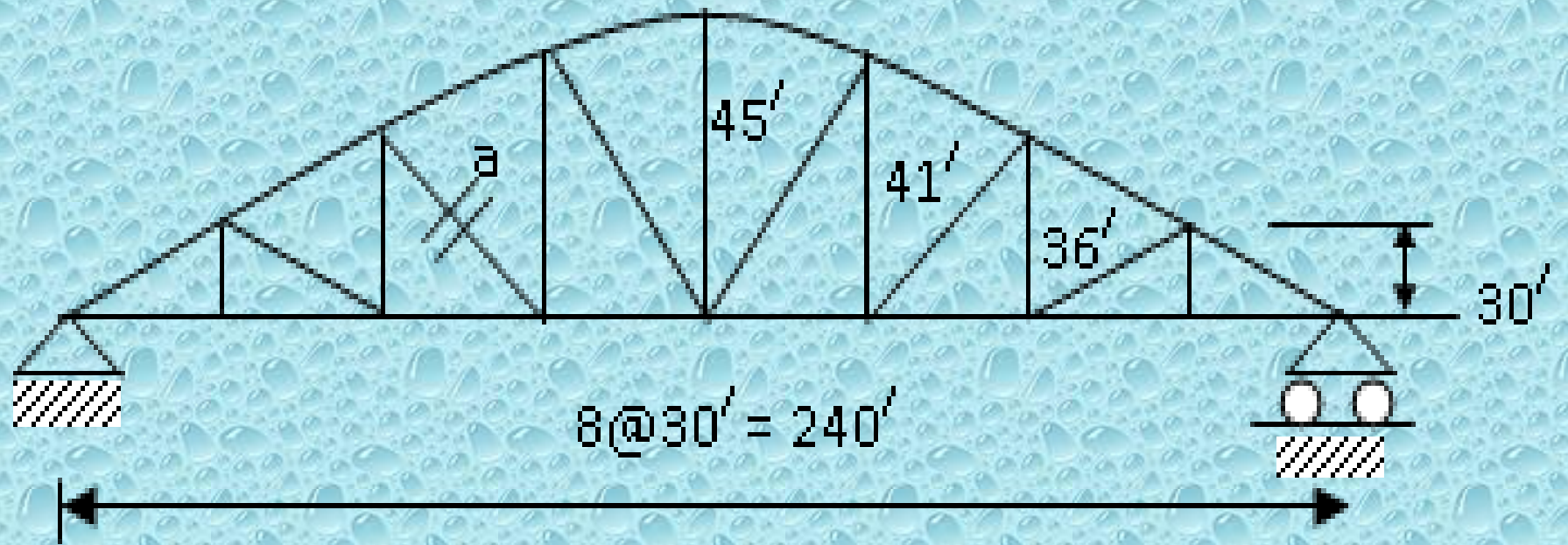
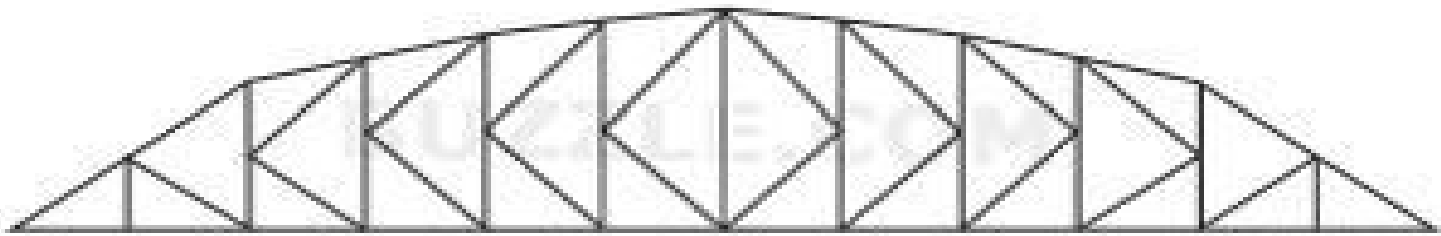


Fig. 1.1(a) simple truss

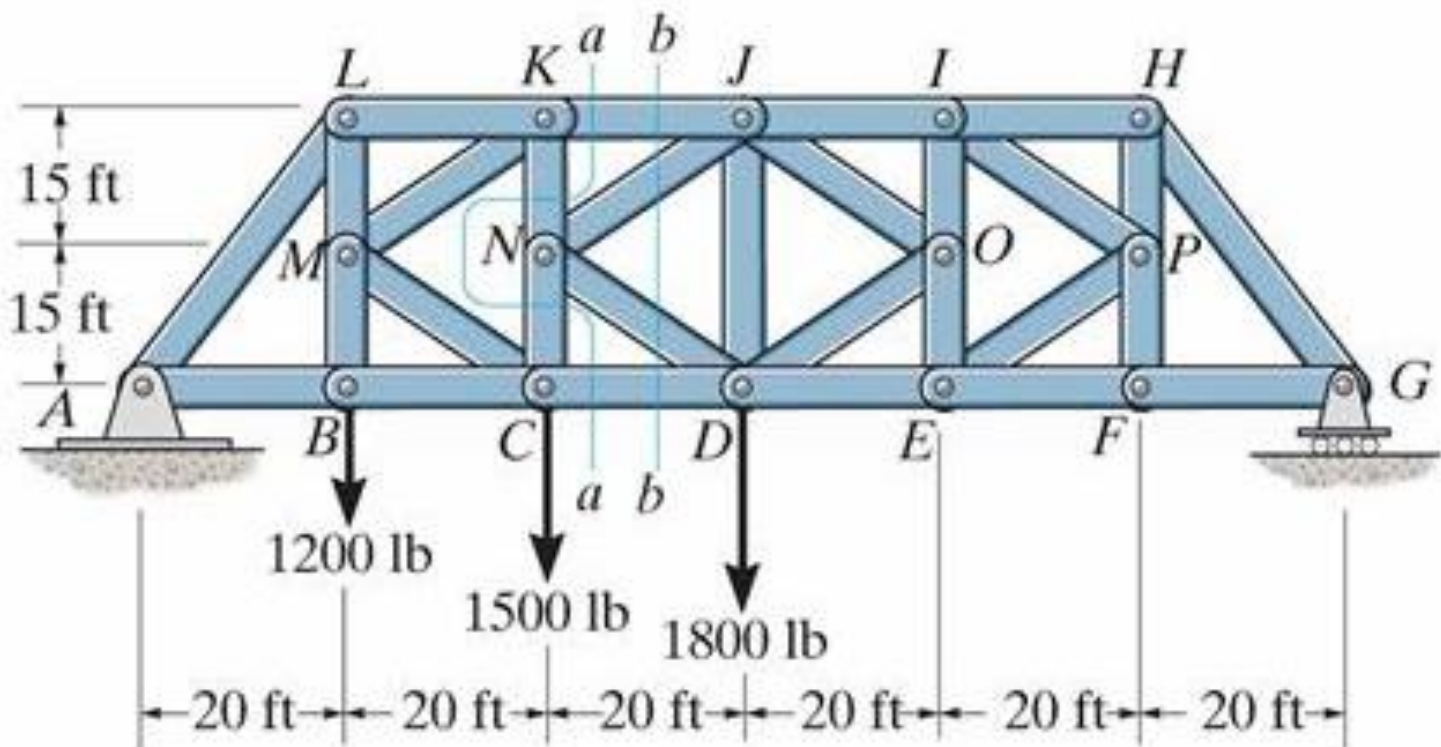
Fig. 1.1(b) Compound truss

Fig. 1.1(c) Complex truss





K truss



Lane Loading:

Concentrated load

1. 18000 for moment
2. 26000 for shear

Uniform load 640 lbs. per linear foot of lane

H₂₀ Loading

Lane Loading:

Concentrated load

1. 13500 for moment
2. 19500 for shear

Uniform load 480 lbs. per linear foot of lane

H₁₅ Loading

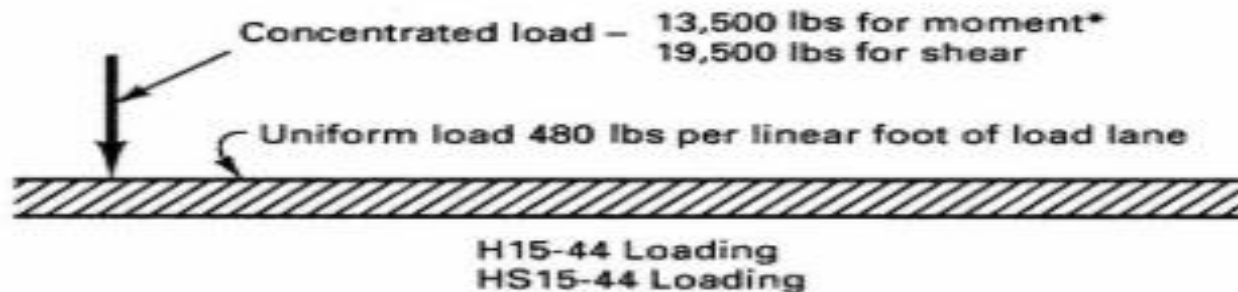
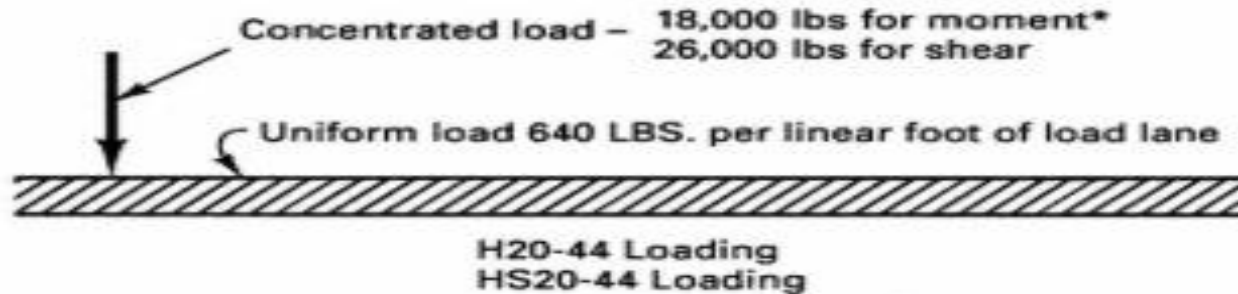
Lane Loading:

Concentrated load

1. 9000 for moment
2. 13000 for shear

Uniform load 320 lbs. per linear foot of lane

H₁₀ Loading



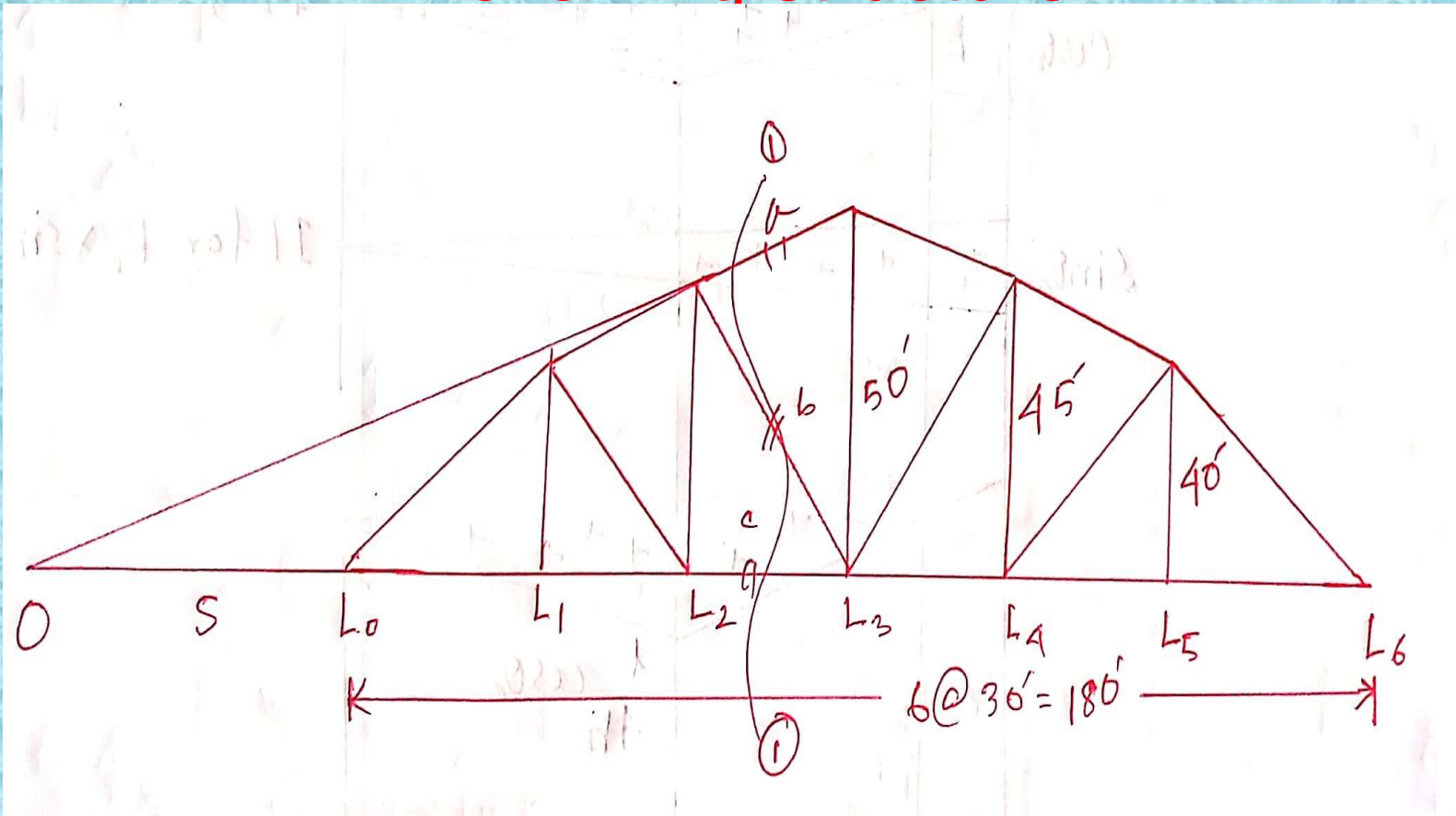
Harding Bridges



Harding Bridges with train loading (Lane Loading:)



Draw the IL diagram for a, b, c, of the following structure.



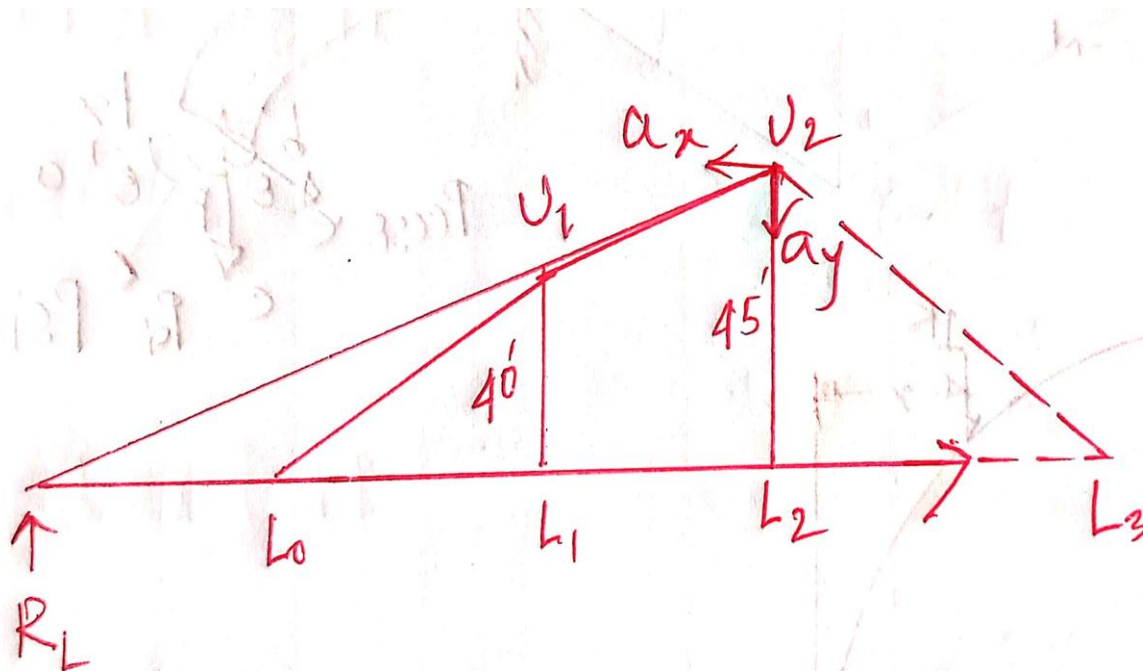
When the unit load at L_0 ,

$$R_L=1, R_R=0, a=b=c=0$$

When the unit load at L_2 ,

$$R_L=4/6 =.67$$

$$R_R=.33$$



$$\sum Mu^2 = 0$$

$$C \times 45 = .67 \times 60$$

$$C = 0.89$$

Similar triangle

$$\frac{50}{90+s} = \frac{45}{60+s}$$

$$S = 210$$

$$\sum Mu_2 = 0$$

$$C \times 45 = .33 \times 120$$

$$C = 0.88$$

$$\sum ML_2 = 0$$

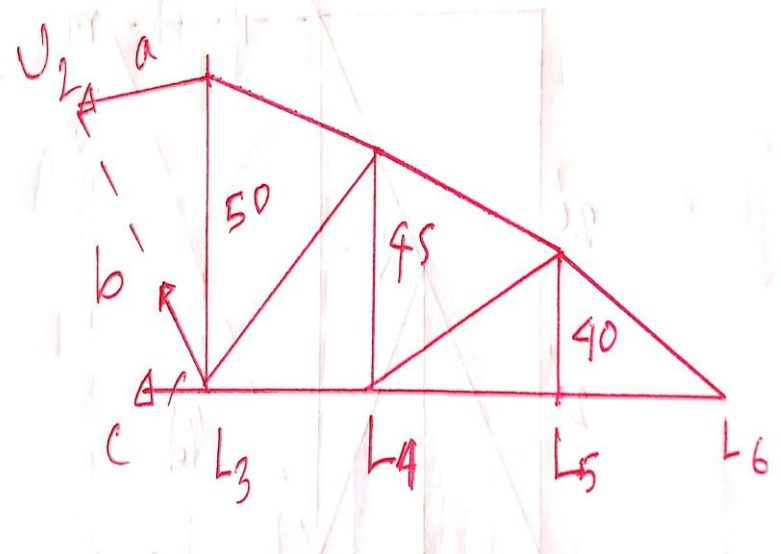
$$ax \times 50 + .33 \times 90 = 0$$

$$50 \times .986 \times a + .33 \times 90 = 0$$

$$A = - .60$$

$$\sum F_x = 0$$

$$-ax - bx - c = 0$$



$$-.986 \times .60 - .55b - .88 = 0$$

$$b = -.52$$

When unit load at L3

$$R_R = .5$$

$$\sum Mu_2 = 0$$

$$C \times 45 + 1 \times 30 = .5 \times 120$$

$$C = .67$$

$$\sum ML_3 = 0$$

$$50 \times .986a + .50 \times 90 = 0$$

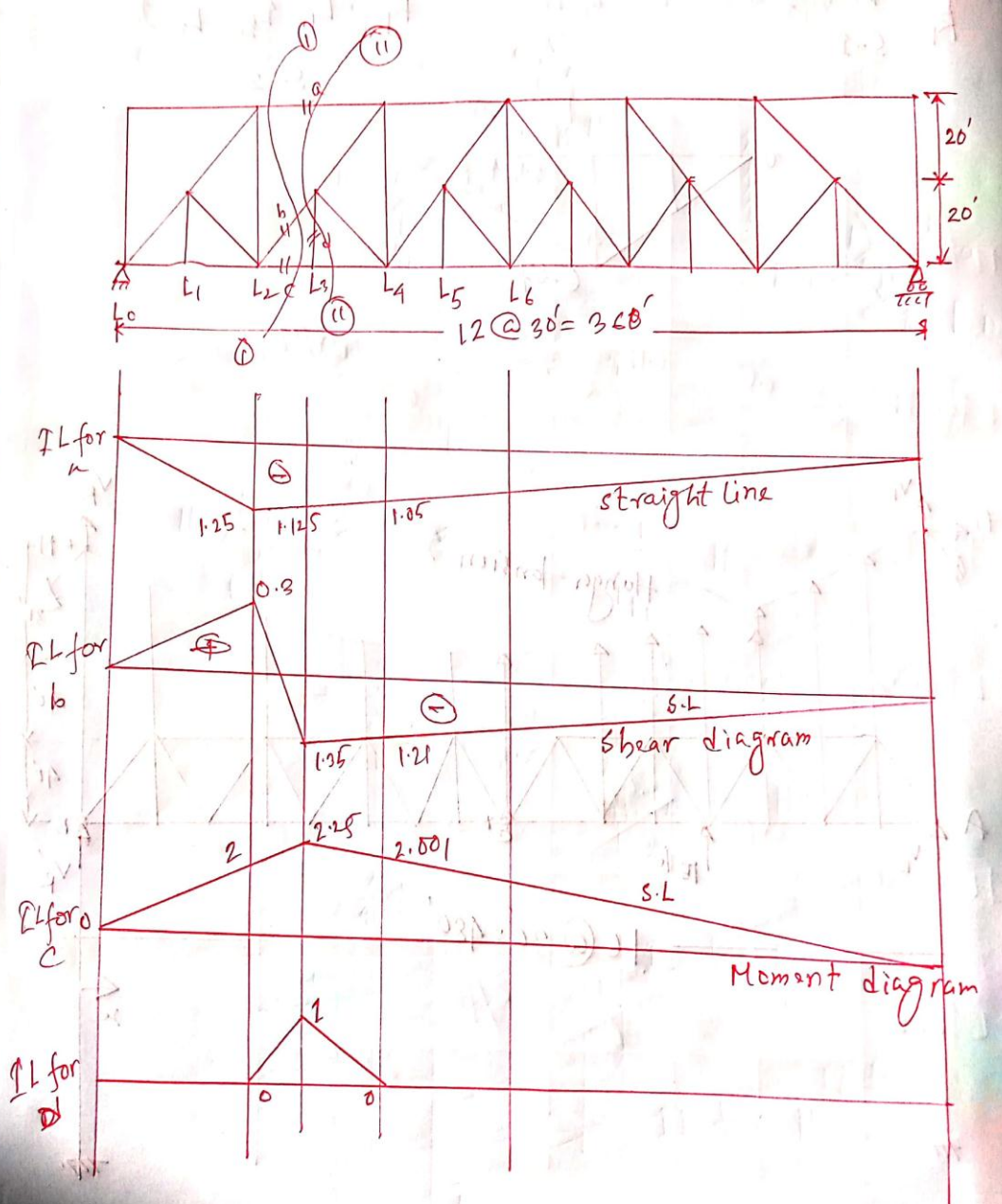
$$a = .91$$

$$\sum F_x = 0$$

$$-.986 \times (-.91) - .555b - .67 = 0$$

$$b = .41$$

Draw IL diagram for member a, b, c, d also calculate minimum stress due to H20 loading of the same structures



Calculation:
 When the unit load at L₀,
 $R=0$, $RR=0$,
 $a=0$, $b=0$, $c=0$, $d=0$
 When the unit load at L₂,

$$RL = 10/12 = .83$$

$$RR = 1/6$$

S-1,

$$\sum ML_2 = 0$$

$$a \cdot 40 + \frac{5}{6} \cdot 60 = 0$$

$$a = -1.25$$

$$\sum Mm = 0$$

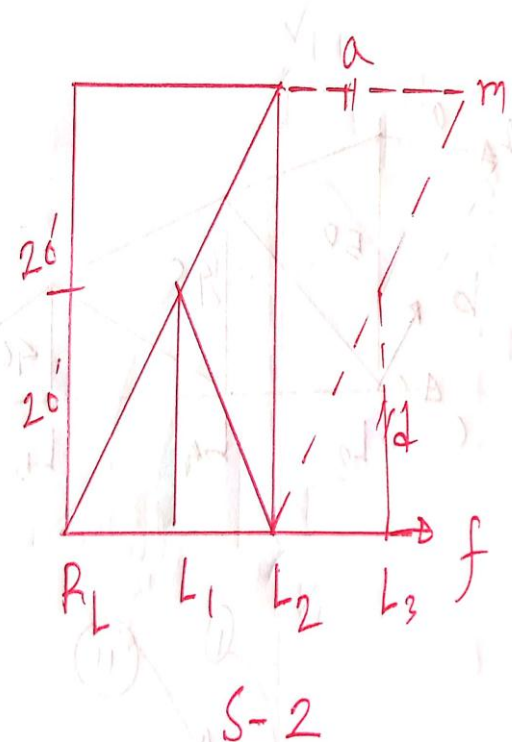
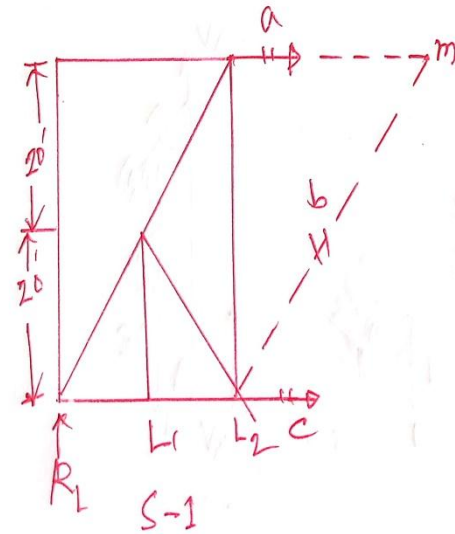
$$1 \cdot 60 + C \cdot 40 = \frac{5}{6} \cdot 120$$

$$C = 1$$

$$\sum Fy = 0$$

$$0.83 + b \sin 33.70 - 1 = 0$$

$$b = .30$$



S-2

$$\sum ML2 = 0$$

$$.83*60-1.25*40= d * 30$$

$$d=0$$

When unit load at L3

$$RL=9/12= .75$$

$$RR=.25$$

S-1

$$\sum ML2 = 0$$

$$a*40+.75*60= 0$$

$$a= -1.125$$

$$\sum Mm = 0$$

$$C*40=.75*120$$

$$C=2.25$$

$$.75 + b \sin 33.70 - 1 = 0$$

$$b = -1.35$$

S-2

$$\sum ML2 = 0$$

$$.75 * 60 + 1 * 30 - 1.125 * 40 = d * 30$$

$$d = 1$$

When unit load at L4,

$$RL = 8/12 = .67$$

$$RR = .33$$

S-1

$$\sum ML2 = 0$$

$$a * 40 + .67 * 60 = 0$$

$$a = -1.005$$

$$\sum Mm = 0$$

$$C * 40 = .67 * 120$$

$$C = 2.01$$

$$\sum Fy = 0$$

$$.67 + b \sin 33.70 - 1 = 0$$

$$b = -1.21$$

S-2

$$\sum ML2 = 0$$

$$.67 * 60 - 1.005 * 40 = d * 30$$

$$d = 0$$

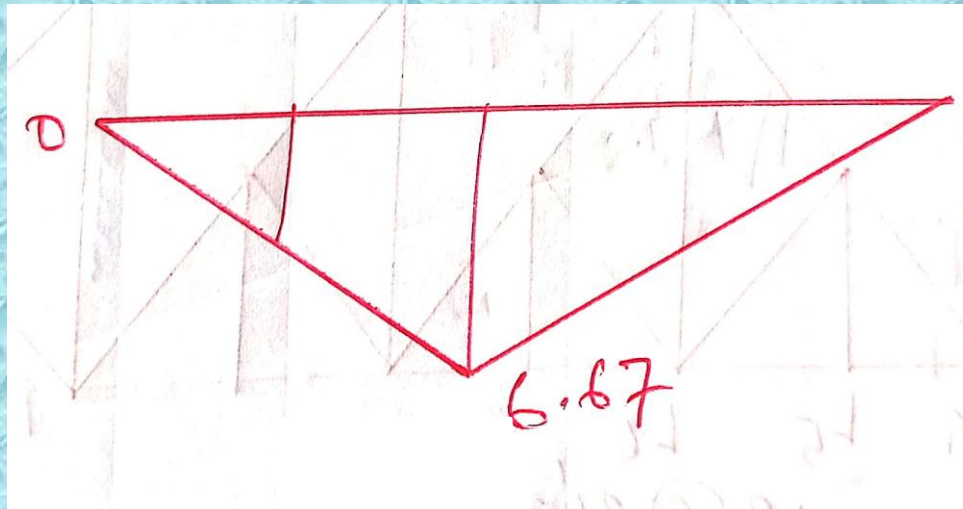
$$\begin{aligned}\text{Maximum stress at member a} &= [.5 \cdot 360 \cdot 1.25] \cdot .64 + 1.25 \cdot 18 \\ &= 310.5 \text{ kips}\end{aligned}$$

$$\begin{aligned}\text{Maximum stress at member b (-ve stress)} &= [.5 \cdot 270 \cdot 1.35 + .5 \cdot 24.54 \cdot 1.35] \cdot .64 + \\ &\quad 1.35 \cdot 26 \\ &= 162.34 \text{ kips}\end{aligned}$$

$$\begin{aligned}\text{Maximum stress at member b (+ve stress)} &= [.5 \cdot 60 \cdot .3 + .5 \cdot 5.45 \cdot .3] \cdot .64 + .3 \cdot 26 \\ &= 14.08 \text{ kips}\end{aligned}$$

$$\begin{aligned}\text{Maximum stress at member c} &= [.5 \cdot 360 \cdot 2.25] \cdot .64 + 2.25 \cdot 18 \\ &= 299.7 \text{ kips}\end{aligned}$$

$$\begin{aligned}\text{Maximum stress at member d} &= [.5 \cdot 60 \cdot 1] \cdot .64 + 1 \cdot 18 \\ &= 37.2 \text{ kips}\end{aligned}$$



$$\begin{aligned} 6.67/10 &= \\ x/2 \\ X &= 1.334 \end{aligned}$$

$$\begin{aligned} 6.67/10 &= \\ x/5 \\ X &= 3.335 \end{aligned}$$



Thanks All